Civic Offices Infirmary Hill, Roosky, Monaghan Town, Co. Monaghan.

Engineering Services Report

Project Ref: 2223

November 2023

Issue No. 3

CORA Consulting Engineers

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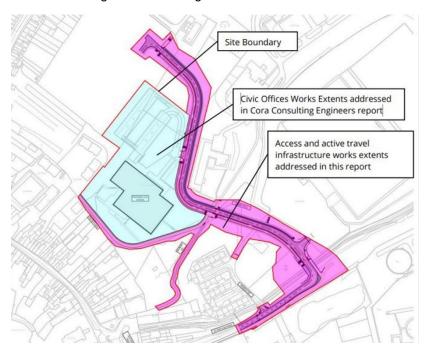
Appendix E – Water Protection Checklist

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1 Introduction

The existing site is located in Roosky Lands, north of the existing Ulster Canal Greenway route and is surrounded by Monaghan Harps GAA Club to the east, residential area to the south with various business on Dublin St. & Glaslough St. backing on to the site to the west & north. The site is currently a sloping greenfield site with no access by road. The River Shambles is to the south of the site.

CORA Consulting Engineers are responsible for the design of the Foul & Surface Water Drainage for the Office and Carpark area of the site while DBFL Consulting Engineers are responsibility for the roads elements. This Report specifically refers to the Office & Carpark areas which are defined in light blue in the figure below.



2 Summary of Design

The following is proposed for the site.

- (i) a new road will be constructed, initially as a construction access road for the site during construction and, upon completion of the site, an access road for the civic offices & any future development in the area;
- (ii) a new 3 storey building for Monaghan County Council Civic Offices with a gross floor area of 5601m² incorporating an entrance foyer, office space, meeting rooms, staff restaurant, council chamber, public counter, reception desk, welfare facilities, internal landscaped courtyards and supporting spaces;
- (iii) provision of 112 no. vehicular parking spaces (including 4 no. mobility parking spaces, and 12 no. electric charging spaces);
- (iv) provision of 80 no. bicycle parking spaces;

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(v) all ancillary works including, landscaping, boundary treatments, internal footpaths, bin storage, services enclosure, foul and surface water drainage, ESB substation and all site services, site infrastructure and associated site development works necessary to facilitate the development.

The civic offices consist of a grey roof with a drainage barrier collected via rainwater pipes and carried to the attenuation tank to the south-east of the site. The outflow from here is limited by a HydroBrake® to 4.3 l/s/ha towards the new surface water pipe under the road. From here is flow to a new attenuation basin at the bottom of Quarry Walk. The attenuation tank also has a high-level overflow to the public sewer. Car parks consist of permeable paving with storage below that infiltrates into the ground. Podiums have storage below that drains to the attenuation tank. All internal drainage devices have overflows to the attenuation tank. The entrance area adjacent to St. Davnet's Row flows into a soakaway which infiltrates into the ground.

3 Design Assumptions

1. SOIL type

The SOIL type was calculated using GDSDS providing a SOIL type 3. This value was used in the surface water calculations. The calculation is included in the surface water calculations in Appendix B.

2. QBAR

QBAR was calculated using the www.uksuds.com website. Using the SOIL type 3, a value of 4.3 l/s/ha was calculated. The total site area is inclusive of any large parkland areas and public open space as per the HR Wallingford website (uksuds.com). The allowable runoff for specific catchment areas is calculated on a catchment area basis. The findings of the soil testing was that the soil was reasonably suitable in certain areas for infiltration with soakaway tests generally passing.

3. Storm Events Analysed

The following Storm events have been considered:

- 1 year return period plus 20% for climate change
- 5 year return period plus 20% for climate change
- 30 year return period plus 20% for climate change
- 100 year return period plus 20% for climate change

4. Runoff Coefficients

The following runoff coefficients have been used:

Grey Roof (Selected Stone Ballast): 1.0

Hardstanding Areas (Paving, etc.): 1.0

Landscaped Areas (Grassed, etc.): 0.3

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4 SuDS Measures Considered

The following is a list of the SuDS measures considered for the site.

Swales/Filter Drains/Infiltration Basin

Swales/Infiltration Basins are not being proposed for the site. There is a Filter Drain running along St. Davnet's Row. This is taking the rainwater from St. Davnet's Row only.

Permeable Paving

Permeable paving is provided for the overground car parks. The car parking spaces will have a permeable surface whereas the circulation areas will have a more traditional hard wearing impermeable surface with storage below. There are overflow pipes from each car park to a surface water pipe system to the attenuation tank.

Surface Water Attenuation

An attenuation tank is provided to the south-east of the site collecting rainwater from the building and overflows from the remainder of the site. There is also attenuation over the podium in the form of 85mm deep drainage board (such as permavoid cells or similar approved).

Rainwater Harvesting

Rainwater harvesting is not being proposed for the site.

Detention Basins, Retention Ponds, Stormwater Wetlands, etc.

Not proposed for this development.

5 Storm Network Details

The civic offices consist of a grey roof made up of selected stone. There is a drainage barrier below the stone removing the rainwater of any debris. The roof areas drain to the edges where they are collected via rainwater pipes and carried to the attenuation tank. The outflow from the attenuation tank is limited by a HydroBrake ®, limiting the flow to 4.3 l/s/ha. This attenuated flow is discharged into the new public sewer under Infirmary Hill/Quarry Walk where it flows into a new attenuation basin and the bottom of Quarry Walk. There is also a high-level overflow pipe from the attenuation tank into the public sewer in the event of a major rainfall event or an unforeseen blockage.

The car parking areas have a permeable surface with storage underneath taking rainwater from the area of the car park & a percentage of the soft landscaping uphill from it. The rainwater is infiltrated into the ground and there is an overflow pipe that drains to the Attenuation Tank in the event of a major rainfall event.

The podium area is an impermeable surface. The rainfall is collected through channels and stored under the podium where it flows to the attenuation tank.

The entrance area adjacent to St. Davnet's Row is drained to a soakaway which infiltrates into the ground.

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The storm network is designed based on the Modified Rational Method where:

Q (I/s) = Cv*Cr*(2.78*I (mm/hr)*A (ha))

Cv = 0.75 and Cr = 1.3 (as recommended by the Wallingford Procedure)

The M5-60 and ratio R values were obtained from Met Eireann data for the site and can be found in Appendix B (16.1mm & 0.271 respectively).

Details of the proposed surface water drainage are detailed on CORA Drawing no. MCC-CORA-ZZ-ZZ-DR-C-0004.

6 Foul Drainage Details

We have calculated the total foul discharge from the site using the Irish Water Codes of Practice for Waste Water based on an office with a canteen (100 l/person/activity/day.)

Post Development Wastewater Loading:

Proposed number of people in the office = 210

For an Office with a canteen:

Wastewater Loading = 100 I/person/day (Irish Water Code of Practice - Appendix D).

Average = $210 \times 100 = 21000 \text{ l/day}$

Average Discharge = 21000/(24x60x60) = 0.24 l/s (Average) DWF

Peak Discharge = 6 DWF = 6 x 0.24 = 1.44 l/s

• Emergency Wastewater Storage:

Emergency storage is based on the Average DWF for 24 hours.

21000 l/day = 21.00m3/day => Storage Required = 21.00m3 min.

Storage Tanks will be provided.

The foul sewer falls by gravity within the site to a final manhole where it then enters the new public sewer under Infirmary Hill/Quarry Walk. From here it flows down to Slí Ógie Uí Dhufaigh where it ties into the existing public sewer.

The connection point to the public sewer has been confirmed as feasible subject to upgades (including new foul sewer under proposed new road) by Uisce Eireann following a review of the pre-connection enquiry process. See Appendix D for copy of correspondence from Irish Water.

Details of the proposed foul drainage are detailed on CORA Drawing no. MCC-CORA-ZZ-ZZ-DR-C-0003.

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7 Water Main

There is an existing watermain running through the site (through the proposed location of the civic offices). This will be diverted along St. Davnet's Row and Infirmary Hill. The proposed new water supply will be connected into this diverted line along Infirmary Hill.

Post Development Wastewater Loading:

It is generally accepted that design loading for foul drainage can be used to evaluate an

approximation of the water demand:

From wastewater discharge (Section 6)

Average Discharge = 0.24 l/s (DWF)

Water main demand is designed with a peak flow of 1.25 x DWF (Irish Water CoP - Cl. 3.7.2).

Peak Water Demand = 1.25 DWF = 1.25 x 0.24 = 0.3 l/s

Fire hydrants are provided throughout the site.

Details of the proposed water main are detailed on CORA Drawing no. MCC-CORA-ZZ-ZZ-DR-C-0005.

8 Flood Risk Assessment

At an early stage of the project it was decided that a detailed Flood Risk Assessment was not required due to the location & topography of the proposed site. A flood map, extracted from https://www.floodinfo.ie/map/floodmaps/, can be found in Appendix F.

9 Maintenance

The required maintenance for each of the SuDS measures on the site is outlined in the various tables below.

9.1 Attenuation Storage Tanks

Operation and maintenance requirements for attenuation storage tanks

Maintenance schedule	Required action	Typical frequency
	Inspect and identify any areas that are not operating correctly. If required, take remedial action	Monthly for 3 months, then annually
	Remove debris from the catchment surface (where it may cause risks to performance)	Monthly

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Regular maintenance	For systems where rainfall infiltrates into the tank from above, check surface of filter for blockage by sediment, algae or other matter; remove and replace surface infiltration medium as necessary.	Annually
	Remove sediment from pre-treatment structures and/or internal forebays	Annually, or as required
Remedial actions	Repair/rehabilitate inlets, outlet, overflows and vents	As required
Monitoring	Inspect/check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed	Annually
	Survey inside of the tank for sediment build-up and remove if necessary	Every 5 years or as required

9.2 Soakaways

Operation and maintenance requirements for soakaways

Maintenance schedule	Required action	Typical frequency
	Inspect for sediment and debris in pre- treatment components and floor of inspection tube or chamber and inside of concrete manhole rings	Annually
Regular maintenance	Cleaning of gutters and any filters on downpipes	Annually (or as required based on inspections)
	Trimming any roots that may be causing blockages	Annually (or as required)
Occasional maintenance	Remove sediment and debris in pre-treatment components and floor of inspection tube or chamber and inside of concrete manhole rings	As required, based on inspections
Remedial actions	Reconstruct soakaway and/or replace or clean void fill, if performance deteriorates or failure occurs	As required
	Replacement of clogged geotextile (will require reconstruction of soakaway)	As required
Monitoring	Inspect silt traps and note rate of sediment accumulation	Monthly in the first year and then annually
	Check soakaway to ensure emptying is occurring	Annually

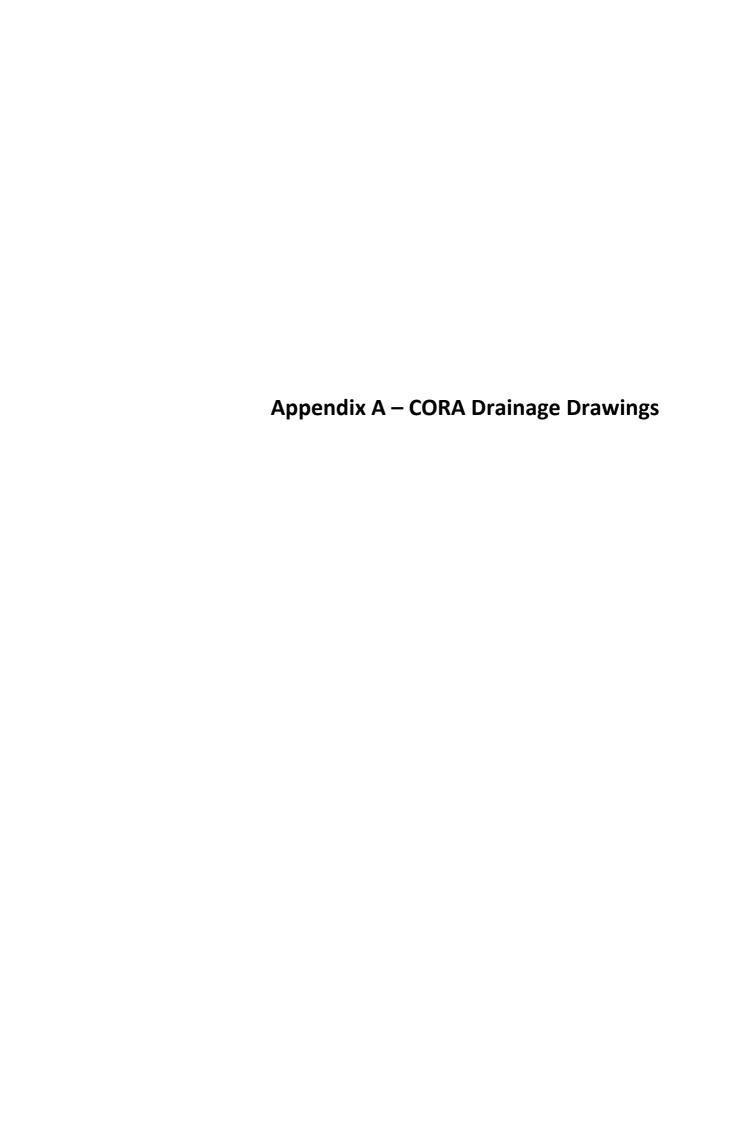
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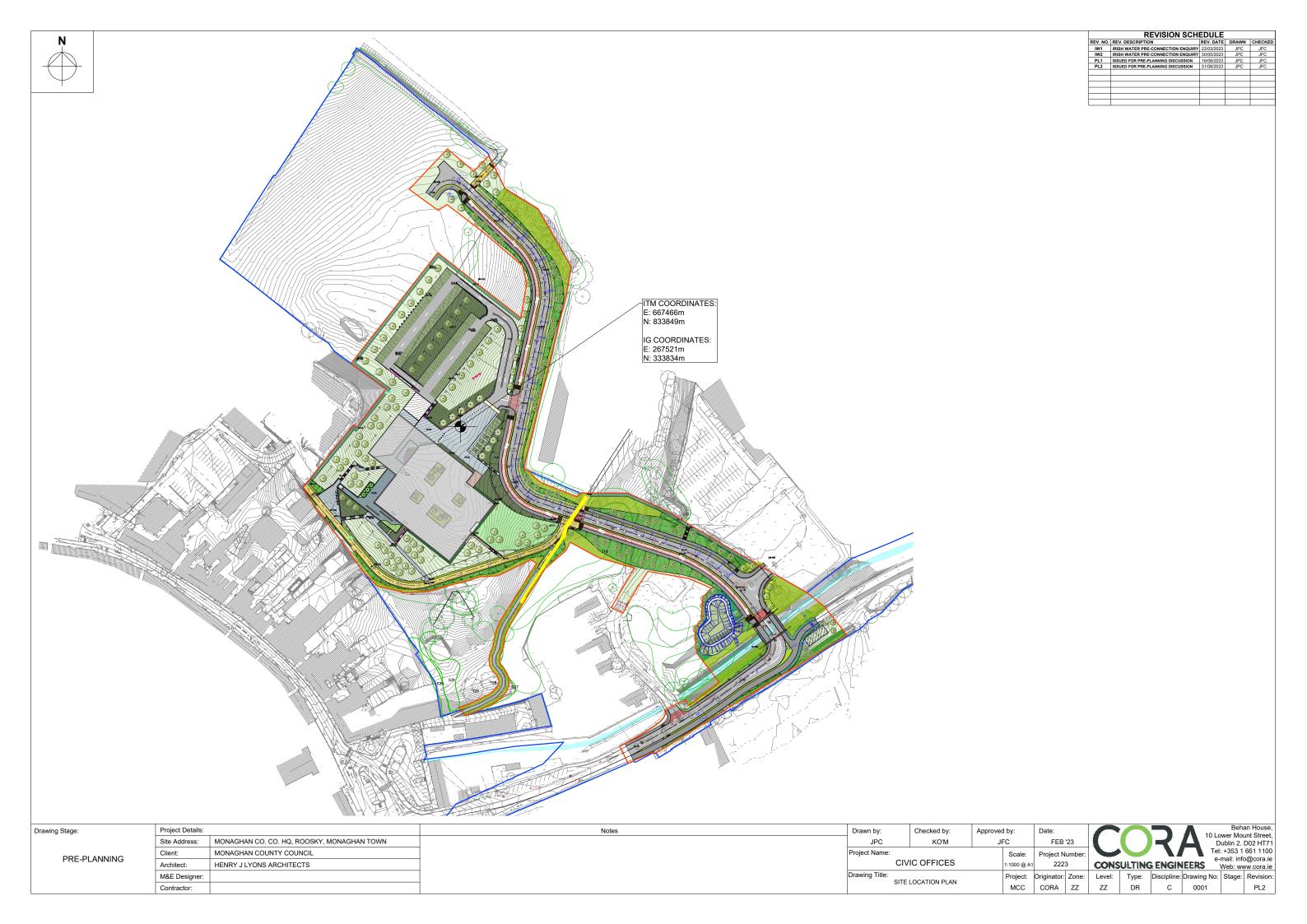
9.3 Permeable Pavements

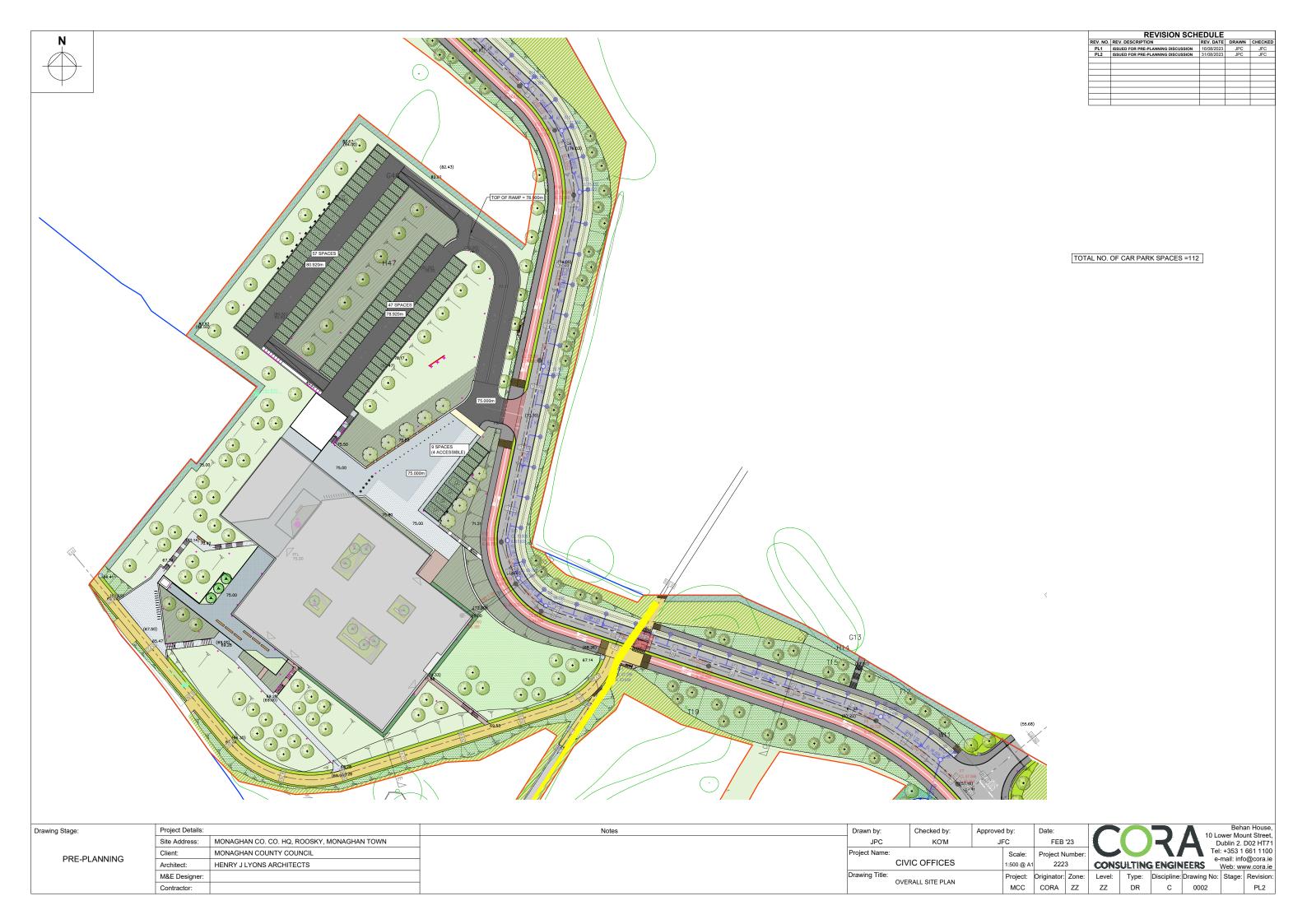
Operation and maintenance requirements for permeable pavements

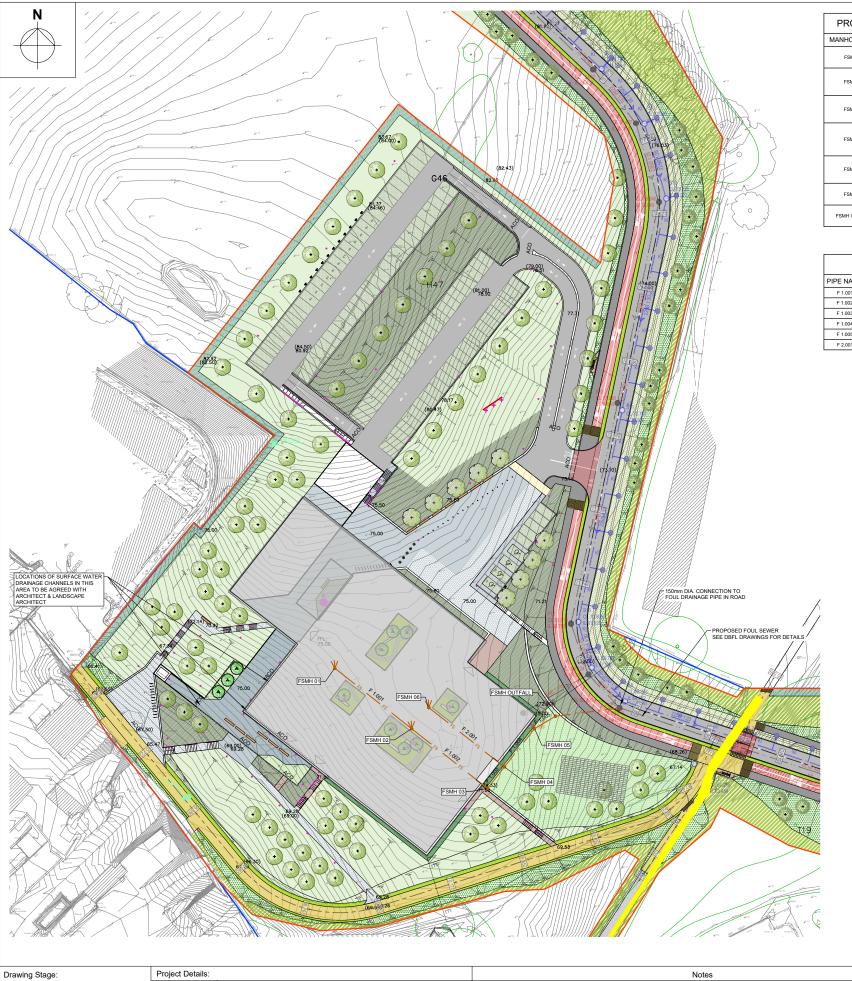
Maintenance schedule	Required action	Typical frequency
Regular maintenance	Brushing and vacuuming (standard cosmetic sweep over whole surface)	Once a year after autumn leaf fall, or reduced frequency as required, based on site-specific observations of clogging or manufacturer's recommendations – pay particular attention to areas where water runs onto permeable surface from adjacent impermeable areas as this area is most likely to collect the most sediment
Occasional maintenance	Stabilise and mow contributing and adjacent areas	As required
	Removal of weeds or management using glyphospate applied directly into the weeds by an applicator rather than spraying	As required – once per year on less frequently used pavements
	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50mm of the level of the paving	As required
Remedial actions	Remedial work to any depressions, rutting and cracked or broken blocks considered detrimental to the structural performance or a hazard to users, and replace lost jointing material	As required
	Rehabilitation of structure and upper substructure by remedial sweeping	Every 10 to 15 years or as required (if infiltration performance is reduced due to significant clogging)
	Initial inspection	Monthly for three months after installation
Monitoring	Inspect for evidence of poor operation and/or weed growth – if required, take remedial action	Three monthly, 48hrs after large storms in first six months
	Inspect silt accumulation rates and establish appropriate brushing frequencies	Annually
	Monitor inspection chambers	Annually

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Site Address: MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN

MONAGHAN COUNTY COUNCIL

HENRY J LYONS ARCHITECTS

PRE-PLANNING

Architect:

Contractor:

M&E Designer:

PROPOSED FOUL SEWER MANHOLE SCHEDULE			
MANHOLE REF.	REF. MANHOLE DETAILS EASTING (m)		NORTHING (m)
FSMH 01	CL = 75.000 IL = 74.220 F 1.001 : IL = 74.220	667434.537	833811.477
FSMH 02	CL = 75.000 IL = 73.897 F 1.001 : IL = 73.897 F 1.002 : IL = 73.897	667454.879	833795.497
FSMH 03	CL = 74.755 IL = 73.578 F 1.002 : IL = 73.578 F 1.003 : IL = 73.578	667474.964	833779.718
FSMH 04	CL = 74.807 IL = 73.488 F 2.001 : IL = 73.920 F 1.003 : IL = 73.488 F 1.004 : IL = 73.488	667479.398	833785.375
FSMH 05	CL = 74.012 IL = 71.839 F 1.004 : IL = 73.327 F 1.005 : IL = 71.839	667487.333	833795.498
FSMH 06	CL = 75.000 IL = 74.170 F 2.001 : IL = 74.170	667459.403	833801.088
FSMH OUTFALL	CL = 72.356 IL = 71.758 F 1.005 : IL = 71.758	667492.925	833798.804

PROPOSED FOUL SEWER PIPE DETAILS							
PIPE NAME SIZE (mm) PLAN LENGTH (m) SLOPE START IL (m) END IL (
F 1.001	110Ø : uPVC	25.868	1:80	74.220	73.897		
F 1.002	110Ø : uPVC	25.542	1:80	73.897	73.578		
F 1.003	150∅ : uPVC	7.188	1:80	73.578	73.488		
F 1.004	150∅ : uPVC	12.862	1:80	73.488	73.327		
F 1.005	150Ø : uPVC	6.497	1:80	71.839	71.758		
F 2.001	110Ø : uPVC	25.431	1:102	74.170	73.920		

REVISION SCHEDULE				
REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKED
IW1	IRISH WATER PRE-CONNECTION ENQUIRY	22/03/2023	JPC	JFC
IW2	IRISH WATER PRE-CONNECTION ENQUIRY	30/05/2023	JPC	JFC
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC

FOUL SEWER DRAINAGE LEGEND

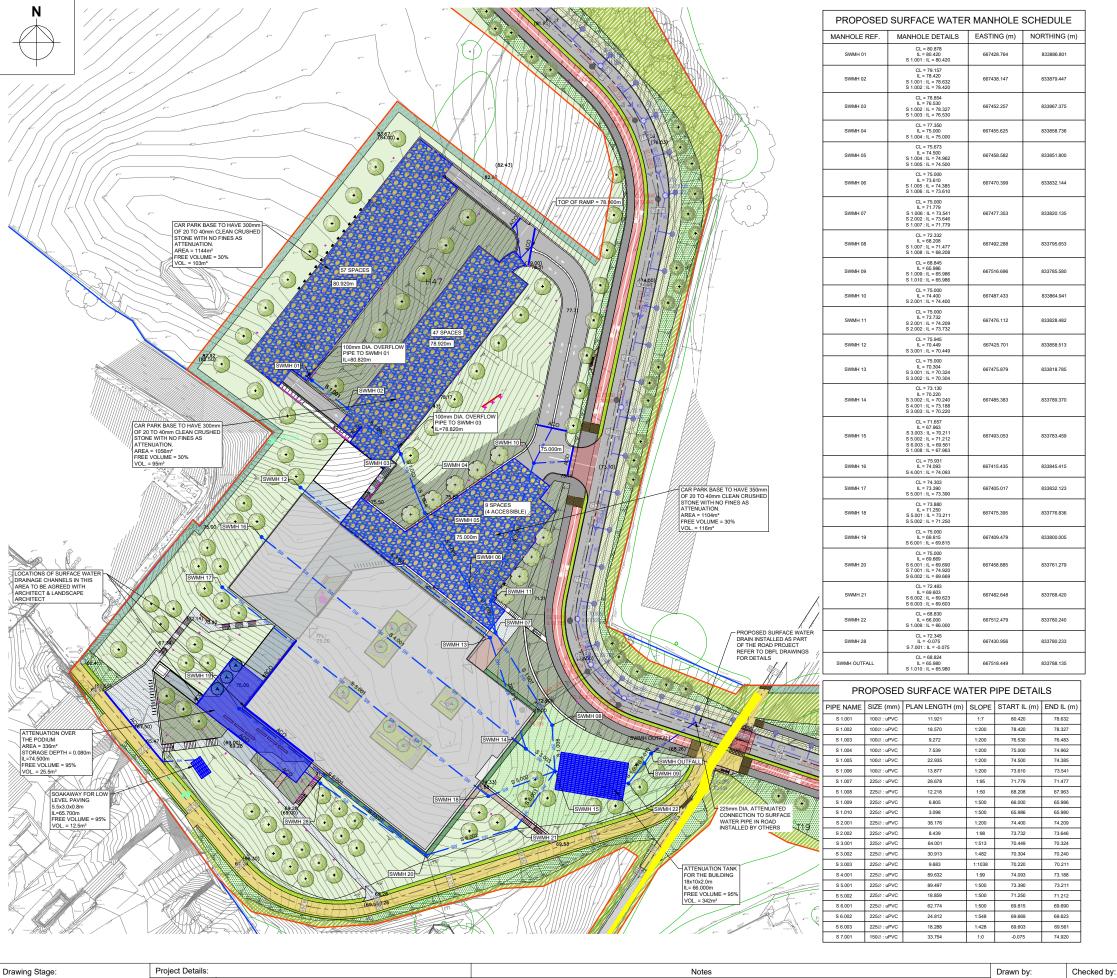
FOUL SEWER (EXISTING)
FOUL SEWER (NEW)
FOUL SEWER (NEW)
FOUL SEWER (SLUNG FROM GROUND FLOOR SLAB)
FOUL SEWER RISING MAIN
EXISTING SEWER TO BE DECOMISSIONED
EX. FSMH (EXISTING FOUL SEWER MANHOLE)
FSMH (NEW FOUL SEWER MANHOLE)

MH (EXISTING MANHOLE TO BE DEMOLISHED)

MH (EAS) ING MANNIOLE TO BE DEMOLISHED)
P.U. • POP UP
SVP • SOIL VENT PIPE
F.S.I.C. • F.S.I.C. (FOUL SEWER INSPECTION CHAMBER)
ACO — ACO DRAINAGE CHANNEL
F.G. ■ FLOOR GULLY

Drawn by: Checked by: Approved by: FEB '23 JPC KO'M JFC Project Number: Scale: CIVIC OFFICES 2223 1:500 @ A1 Drawing Title: OVERALL SITE DRAINAGE PLAN FOUL SEWER Project: Originator: Zone: CORA ZZ MCC

Behan House, 10 Lower Mount Street, Dublin 2. D02 HT71 Tel: +353 1 661 1100 e-mail: info@cora.ie Web: www.cora.ie CONSULTING ENGINEERS Type: Discipline: Drawing No: Stage: Revision: Level: DR ZZ С 0003



	REVISION SCHEDULE				
REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKED	
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC	
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC	
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SURFACE WATER DRAINAGE LEGEND

SURFACE WATER SEWER (EXISTING)

SURFACE WATER SEWER (NEW)

SURFACE WATER (SLUNG FROM GROUND FLOOR SLAB SURFACE WATER RISING MAIN

- EXISTING SEWER TO BE DECOMISSIONED

SWMH (NEW SURFACE WATER MANHOLE)

MH (EXISTING MANHOLE TO BE DEMOLISHED) RAIN WATER DOWN PIPE

S.W.I.C. (SURFACE WATER INSPECTION CHAMBER)
FLOOR GULLY

ACO / SLOT DRAINAGE CHANNEL

Rehan House 10 Lower Mount Street, Dublin 2. D02 HT71 Tel: +353 1 661 1100 e-mail: info@cora.ie **CONSULTING ENGINEERS** Web: www.cora.ie Type: Discipline: Drawing No: Stage: Revision: Level: ZZ DR С 0004 PL2

Approved by:

JFC

Scale:

1:500 @ A1

MCC

KO'M

CIVIC OFFICES

SURFACE WATER

Drawing Title:
OVERALL SITE DRAINAGE PLAN

JPC

Project Name:

Date:

Project: Originator: Zone:

CORA

FEB '23

Project Number

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PRE-PLANNING	

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Site Address:	MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN
Client:	MONAGHAN COUNTY COUNCIL
Architect:	HENRY J LYONS ARCHITECTS
M&E Designer:	
Contractor:	



Architect:

M&E Designer: Contractor:

HENRY J LYONS ARCHITECTS

	REVISION SCHEDULE				
REV. NO.	REV. NO. REV. DESCRIPTION REV. DATE DRAW				
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC	
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC	

 Drawn by:
 Checked by:
 Approved by:
 Date:

 JPC
 KO'M
 JFC
 FEB '23

 Project Name:
 Scale:
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 Project Number:

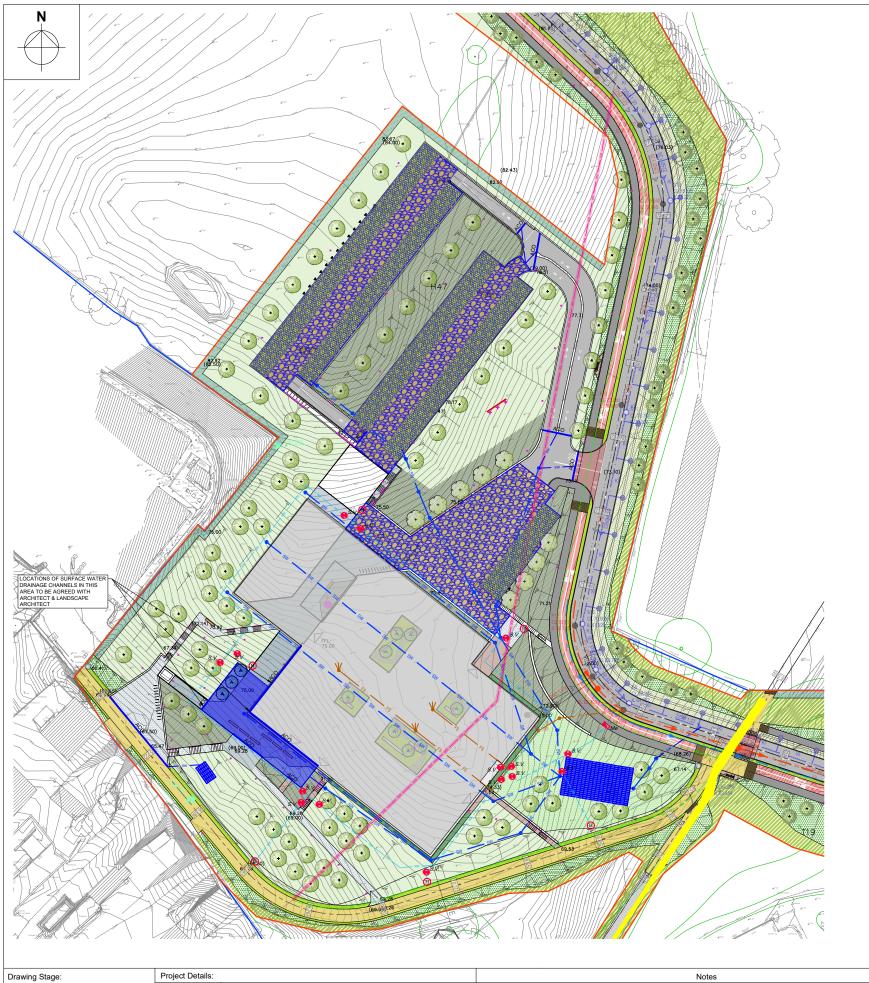
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 2223

 Drawing Title:
 OVERALL SITE WATERMAIN PLAN
 Project:
 Originator:
 Zone:

 MCC
 CORA
 ZZ

Behan House,
10 Lower Mount Street,
Dublin 2. D02 HT71
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e-mail: info@cora.ie
Web: www.cora.ie

Level: Type: Discipline: Drawing No: Stage: Revision:
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REVISION SCHEDULE									
REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKED					
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC					
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC					

●── sw ── EXISTING SURFACE WATER MANHOLE & PIPE ● SEVER MANHOLE & PIPE PROPOSED SURFACE WATER MANHOLE & PIPE PROPOSED FOUL SEWER MANHOLE & PIPE WM --- EXISTING WATER MAIN TO BE RETAINED EXISTING WATER MAIN TO BE DEMOLISHED - ww ---- PROPOSED DIVERTED WATER MAIN (BY DBFL) PROPOSED BUILDING WATER MAIN P.U. O FOUL SEWER POP-UP F.S.A.J. 💠 FOUL SEWER ADJACENT JUNCTION F.G. FLOOR GULLY FOUL SEWER RISING MAIN PROPOSED SURFACE WATER ATTENUATION TANK

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	Architect:	HENRY J LYONS ARCHITECTS			CIVIC OFFICES	1:500 @ A1	2223	•	3
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	Contractor:				COMBINED SERVICES	MCC	CORA	ZZ	

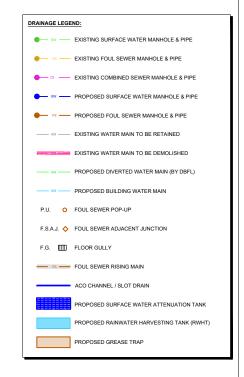
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10 Lower Mount Street,
Dublin 2. D02 HT71
Tel: +353 1 661 1100
e-mail: info@cora.ie
Web: www.cora.ie Type: Discipline: Drawing No: Stage: Revision: DR С

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	REVISION SCHEDULE										
REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKED							
IW1	IRISH WATER PRE-CONNECTION ENQUIRY	29/05/2023	JPC	JFC							
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC							
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC							
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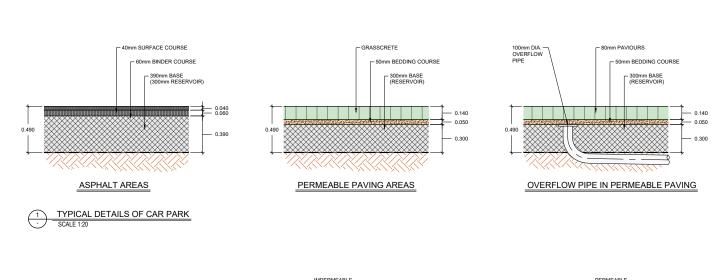


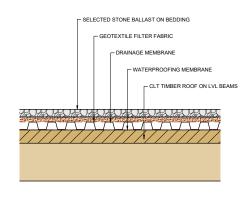
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Client:	MONAGHAN COUNTY COUNCIL		Project Name:			Scale:	Project Number:	
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Contra	ctor:		OVERALL SI	HE FOOL SEWER PLA	AIN	MCC	CORA ZZ	77

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 REVISION SCHEDULE

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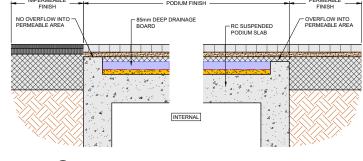
 PL1
 Issued For Pre-Planning Discussion
 16/08/2023
 JPC
 JFC

 PL2
 Issued For Pre-Planning Discussion
 31/08/2023
 JPC
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INTERNAL

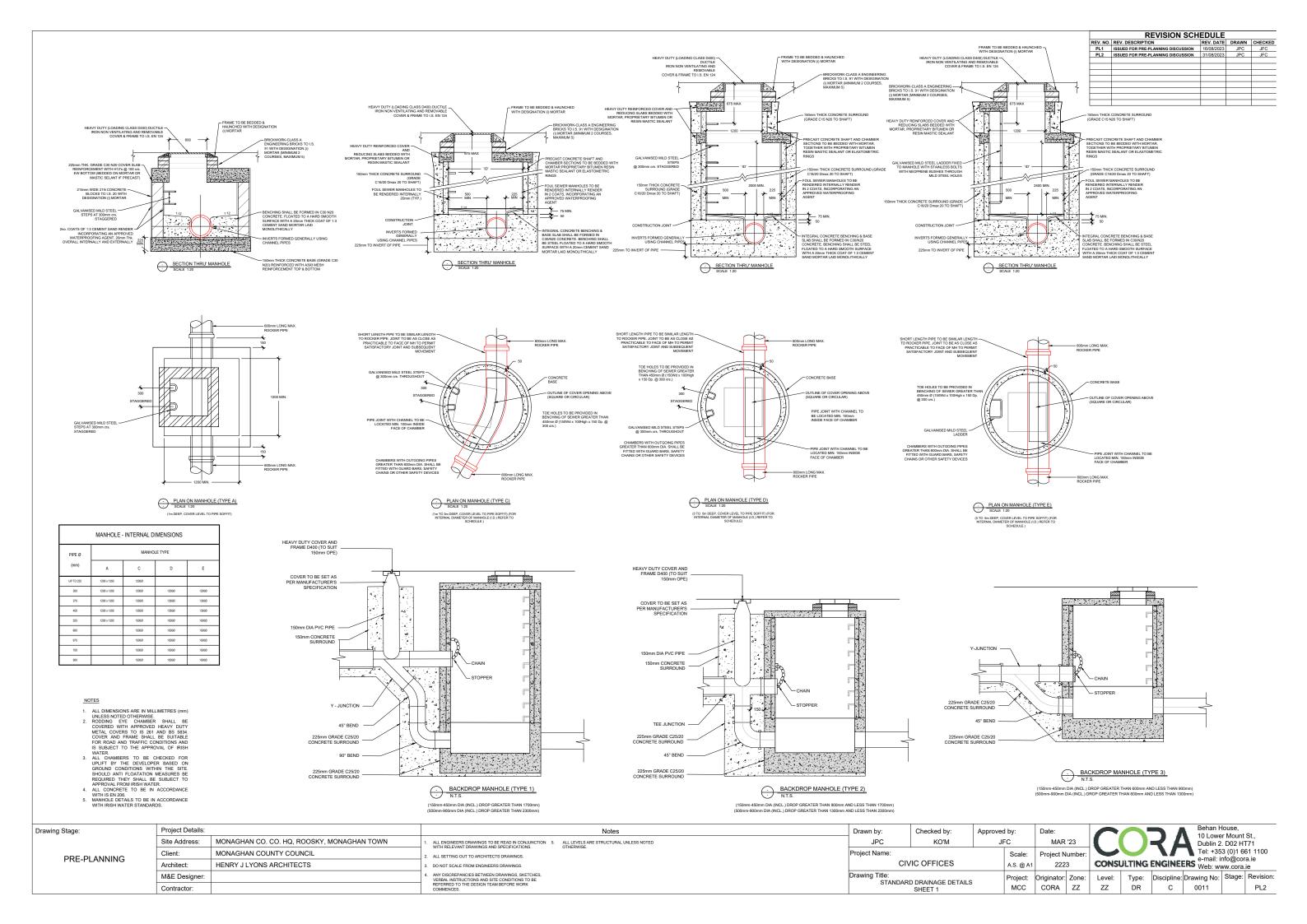
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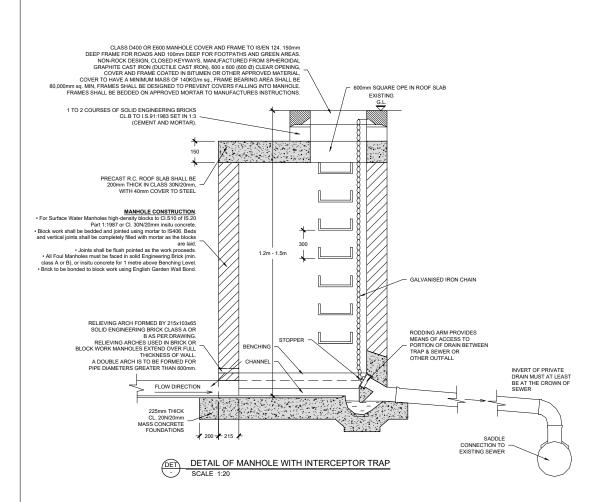
RC SUSPENDED PODIUM SLAB	NO OVERFLOW INTO PERMEABLE AREA	\
INTERNAL		
TYPICAL SECTION OVER PODIUM SCALE 1:20	4	TYPICAL SEC

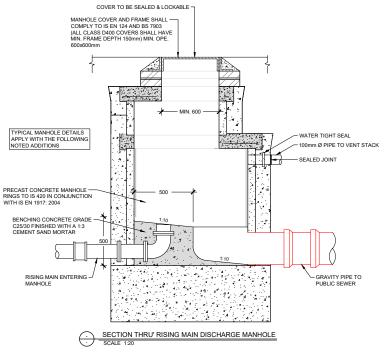


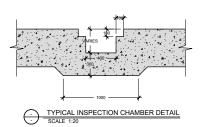
4 TYPICAL SECTION THROUGH EDGE OF PODIUM SCALE 1:20

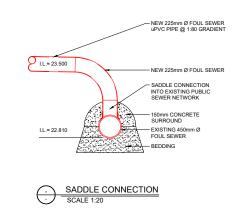
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	Site Address:	MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN		JPC	KO'M	JFC	AUG '23			Dublin 2. D02 HT71
DDE DI ANNUNO	Client:	MONAGHAN COUNTY COUNCIL		Project Name:		Sca	e: Project Numb	er:		Tel: +353 1 661 1100
PRE-PLANNING	Architect:	HENRY J LYONS ARCHITECTS			CIVIC OFFICES	1:20 (D A1 2223	CONSULTI	NG ENGINEER	e-mail: info@cora.ie Web: www.cora.ie
	M&E Designer:			Drawing Title:	E CECTIONS & DETAILS	Proje	ct: Originator: Zor	e: Level: Type	e: Discipline: Draw	ing No: Stage: Revision:
	Contractor:			511	E SECTIONS & DETAILS	мс	c cora zz	ZZ DR		008 PL2

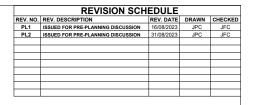


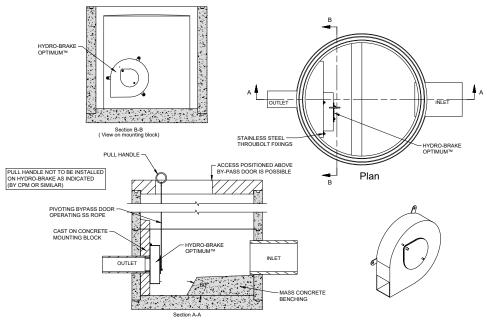












TYPICAL HYDRO-BRAKE DETAILS FOR FINAL SURFACE WATER MANHOLE (FOR HYDRO-BRAKE MANHOLE REFERENCES REFER TO DRG. C001 & C002.)

VIEW OF HYDRO-BRAKE & PENSTOCK VALVE AS SUPPLIED BY CPM SCALE N.T.S.

'	1.2m-1.5m 300 BENCHING CHANNEL 215 LOF MANHOLE WITH INTE	MANHOLE CONSTRUCTION. • For Sulface Water Manholes high-dens Part 1:1987 or CJ. 30N/20mm insitu conce. • Block work shall be bedded and jointed and wertical joints shall be completely filler are laid. • Joints shall be flush pointed as the work - All Foul Manholes must be faced in solid class A or BJ, or insitu concrete for 1 met - Brick to be bonded to block work using I - GALVANISED IRON CHAIN RODDING ARM PROVIDES MEANS OF ACCESS TO - PORTION OF DRAIN BETWEEN TRAP & SEWER OR OTHER OUTFALL SADDLE CONNECTION TO EXISE SEWER (SEE SADDLE CONNECTION TO EXISE SEWER (SEE SADDLE CONNECTION OF DRAIN SEWER OR OTHER OUTFALL	using mortar to IS406. Beds ad with mortar as the blocks of with mortar as the blocks of the proceeds. d Engineering Brick (min. re above Benching Level. English Garden Wall Bond. INVERT OF PRIVATE DRAIN MUST AT LEAST BE AT THE CROWN OF SEWER
SCALE	1:20		

600mm SQUARE OPE IN ROOF SLAB EXISTING G.L.

CLASS D400 OR E600 MANHOLE COVER AND FRAME TO IS/EN 124. 150mm
DEEP FRAME FOR ROADS AND 100mm DEEP FOR FOOTPATHS AND GREEN AREAS.
NON-ROCK DESIGN. CLOSED KEYWAYS, MANUFACTURED FROM SPHEROIDAL
GRAPHITE CAST IRON (DUCTILE CAST IRON), 600 x 600 (600 Ø) CLEAR OPENING,
COVER AND FRAME COATED IN BITUMEN OR OTHER APPROVED MATERIAL,
COVER TO HAVE A MINIMUM MASS OF 140KG/m sq. FRAME BEARING AREA SHALL BE
00mm sq. MIN, FRAMES SHALL BE DESIGNED TO PREVENT COVERS FALLING INTO MANHOLE.
FRAMES SHALL BE BEDDED ON APPROVED MORTAR TO MANUFACTURES INSTRUCTIONS.

Drawing Stage:	Project Details:		Notes	Drawn by:	Checked by:	Approved by:	Date:				Behan House, 10 Lower Mount	nt St
	Site Address:	MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN	L. ALL ENGINEERS DRAWINGS TO BE READ IN CONJUNCTION	JPC	KO'M	JFC	MA	R '23		$A \Delta$	Dublin 2. D02 H	
DDE DI ANNING	Client:	MONAGHAN COUNTY COUNCIL	2. ALL SETTING OUT TO ARCHITECTS DRAWNIGS.	Project Name:		Scal	: Project	Number:			Tel: +353 (0)1 66	
PRE-PLANNING Architect:		HENRY J LYONS ARCHITECTS	DO NOT SCALE FROM ENGINEERS DRAWINGS.	CIVIC OFFICES			1:20 @ A1 2223		CONSULTING ENGINEERS e-mail: info@cora.ie Web: www.cora.ie			a.ie
	M&E Designer:		4. ANY DISCREPANCIES BETWEEN DRAWINGS, SKETCHES, VERBAL INSTRUCTIONS AND SITE CONDITIONS TO BE VERBAL INSTRUCTIONS AND SITE CONDITIONS TO BE	Drawing Title:	ARD DRAINAGE DETAILS	Projec	: Originato	r: Zone:	Level: Type:	Discipline: Dra	awing No: Stage:	: Revision:
	Contractor:		REFERRED TO THE DESIGN TEAM BEFORE WORK COMMENCES.	STANDA	SHEET 2	.S MC	CORA	ZZ	ZZ DR	С	0012	PL2

NOTES

- ALL DIMENSIONS ARE IN MILLIMETRES (mm)
 UNLESS NOTED OTHERWISE.

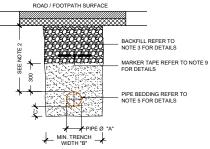
 THE MINIMUM DEPTH OF COVER FROM THE FINISHED SURFACE TO THE CROWN OF GRAVITY PIPES WITHOUT PROTECTION SHOULD BE AS FOLL WITHOUT PROTECTION.
- GRAVITY PIPES WITHOUT PROTECTION SHOULD BE AS FOLLOWS.
 GARDENS AND PATHWAYS WITHOUT ANY POSSIBILITY OF VEHICULAR ACCESS DEPTH NOT LESS THAN 0.5m (THIS WOULD NORMALLY RELATE TO DRAINS IN PRIVATE PROPERTY, SHALLOW PIPES OF THIS NATURE ARE UNDESIRABLE AND CHICAGO PROPERTY. SHOULD BE INSTALLED IN ACCORDANCE WITH THE CURRENT BUILDING REGULATIONS)
- REGULATIONS)

 B) DRIVEWAYS, PARKING AREAS AND YARDS
 WITH HEIGHT RESTRICTIONS TO PREVENT
 ENTRY BY VEHICLES WITH A GROSS
 VEHICLE WEIGHT IN EXCESS OF 7.5
 C) DRIVEWAYS, PARKING AREAS AND
 NARROW STREETS WITHOUT FOOTWAYS
 (E.G. MEWS DEVELOPMENTS) WITH
 LIMITED ACCESS FOR VEHICLES WITH A
 GROSS VEHICLE WEIGHT IN EXCESS OF
 7.5 TONNES DEPTH NOT LESS THAN 0.5m
 D) DEPTHS OF SEWERS IN GATED ESTATES
 SHALL BE SIMILAR TO THAT OUTLINED
 ABOVE

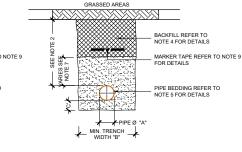
- SHALL BE SIMILAR TO THAT OUTLINED
 ABOVE
 E) AGRICULTURAL LAND AND PUBLIC OPEN
 SPACE DEPTH NOT LESS THAN 09m
 F) OTHER HIGHWAYS AND PARKING AREAS
 WITH UNRESTRICTED ACCESS TO
 VEHICLES WITH A GROSS VEHICLE
 WEIGHT IN EXCESS OF 7.5 TONNES DEPTH NOT LESS THAN 1.2m
 3. CLAUSE BOM MATERIAL IN A CANTHORITY
 SPECIFICATION FOR ROAD WORKS IS TO BE
 LISED AS RACKELI MATERIAL WHERE THE SPECIFICATION FOR ROAD WORKS IS TO BE USED AS BACKFIL MATERIAL WHERE THE SEWER MAIN IS LOCATED IN ROADS. FOOTPATHS OR WHEN THE NARREST PART OF THE TRENCH IS WITHIN 1M OF THE PAVED EDGE OF THE ROADWAY. CLAUSE 808 IS TO BE COMPACTED AS PER CLAUSE 802 OF THE NATIONAL ROADS AUTHORITY SPECIFICATION FOR ROAD WORKS. SELECTED EXCAVATED MATERIAL MAY BE USED IN GREEN-FIELD AREAS ABOVE GRANULAR PIES SURROUND MATERIAL SUBJECT TO THE APPROVAL OF IRISH WATER.
- WATER.
 PIPE BEDDING SHALL COMPLY WITH
 WIS-08-02 AND IGN 4-08-01 GRANULAR
 MATERIAL SHALL BE 14mm TO 5mm GRADED
 AGGREGATE OR 10mm SINGLE SIZED
 AGGREGATE IS EN 13242. CONCRETE BED,
 HAUNCH & SURROUND, WHERE REQUIRED
 SHALL BE TO RISH WATER STANDARDS.
- SHALL BE TO IRISH WATER STANDARDS.
 IN SOFT GROUND CONDITIONS (CBR <5)
 THE MATERIAL SHOULD BE EXCAVATED
 AND DISPOSED OF IN ACCORDANCE WITH AND DISPOSED OF IN ACCORDANCE WIT THE WASTE MANAGEMENT ACT AND CLAUSE 808 MATERIAL IN ACCORDANCE WITH THE NATIONAL ROADS AUTHORITY WITH THE NATIONAL ROADS AUTHORITY SPECIFICATION FOR ROAD WORKS SHALL REPLACE THE EXCAVATED MATERIAL, WRAPPED IN GEO-TEXTILE WRAPPING, ALTERNATIVELY, SPECIAL PIPE SUPPORT ARRANGEMENTS, INCLUDING PILING ETC. MAY BE REQUIRED WHERE THE DEPTH OF SOFT MATERIAL IS EXCESSIVE. SUCH

- SHALL BE EXCAVATED TO A DEPTH OF THE TSOm BELOW THE ACTUAL DEPTH OF THE TRENCH WITH THE VOID FILLED WITH CAUSE 800 MATERIAL IN A COORDANCE WITH THE NATIONAL ROADS AUTHORITY SPECIFICATION FOR ROAD WORKS. THE GRANULAR MATERIAL SHALL BE LAID ABOVE THIS VOID BACKFILL MATERIAL NON DEGRADABLE MARKER TAPE SHOULD BE INSTALLED A TOP OF PIPE BEDDING LAYER. IN THE CASE OF NON METAL PIPE MATERIAL THE TAPE SHOULD INCORPORATE A TRACE WIRE WHICH IS INNED TO FITTINGS AND TERMINATED AT THE WASTE WATER PUMPING STATION AND DISCHARGE MANHOLE.

 1 TRENCH DEPTH, HEALTH A SAFETY & CONSTRUCTION OF THE TRENCH DEPTH, HEALTH A SAFETY & CONSTRUCTION.





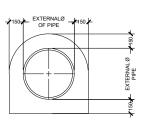


CROSS SECTION IN GRASSED AREAS

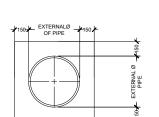
N.T.S



TYPE 'A'
N.T.S.



TYPE 'C'
N.T.S.



TYPE 'D'

TYPE 'B'
N.T.S.

- ALL DIMENSIONS ARE IN MILLIMETRES (mm)
 UNLESS NOTED OTHERWISE.

 CONCRETE PIPE BEDS AND HAUNCHES MAY
 BE REQUIRED TO ADDRESS MINIMUM COVER
 SITUATIONS, AND SHALL BE SUBJECT TO
 SUBMISSION AND ASSESSMENT BY IRISH
 WATER BEFORE ADVANCING WITH THE
 WORKS.
- WORKS.
 CONCRETE PIPE BEDS AND HAUNCHES
 SHALL HAVE A MINIMUM THICKNESS OF
 150mm WITH AN ABSOLUTE MINIMUM DEPTH
 OF COVER ABOVE THE EXTERNAL CROWN
 OF THE PIPE OF 750mm.

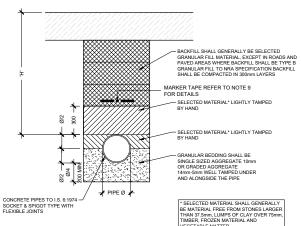
- OF COVER ABOVE THE EXTERNAL CROWN OF THE PIPE OF 750mm.

 4. CONCRETE TO BE IN ACCORDANCE WITH IS EN 206 AND TO BE CLASS C16/20.

 5. THE HAUNCHES AND SURROUNDS TO BE FORMED USING FORM WORK TO PROVIDE A ROUGH CAST FINISH.

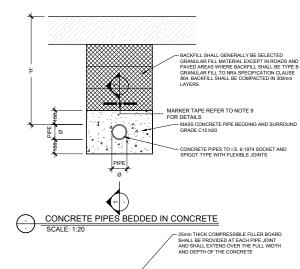
 6. EXPANSION JOINTS IN THE CONCRETE SHALL BE PROVIDED AT ALL PIPE JOINTS TO ALLOW FOR PIPE FLEXIBILITY. COMPRESSIBLE FILLER BOARD TO BE IN ACCORDANCE WITH BS EN 8221 AND BS EN 6224, AN TO BE 18mm THICK.

 7. POLYETHYLENE PIPES SHALL BE WRAPPED IN PLASTIC SHEETING HAVING A COMPOSITION IN ACCORDANCE WITH BS
- IN PLASTIC SHEETING HAVING A COMPOSITION IN ACCORDANCE WITH BS 6076 BEFORE BEING CAST INTO CONCRETE. BITUMINOUS MATERIAL SHALL NOT BE PUT IN CONTACT WITH PE OR PVC PIPES.



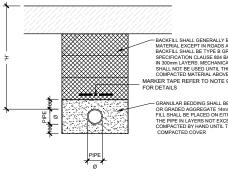
CONCRETE PIPES BEDDED IN GANULAR MATERIAL SCALE: 1:20

NOTE:
DETAIL APPLIES WHERE COVER 'H' IS <u>GREATER THAN</u> 1.2m IN ROADS AND PAVED AREAS, 0.9m IN FIELDS AND FOOTPATHS AND 0.6m IN GARDENS



SECTION 1-1 SCALE: 1:20

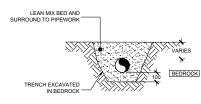
 $\frac{\text{NOTE:}}{\text{DETAIL APPLIES WHERE COVER'H' IS }} \underbrace{\text{LESS THAN}}_{\text{1.2m IN}} 1.2 \text{m IN} \\ \text{ROADS AND PAVED AREAS, 0.9m IN FIELDS AND FOOTPATHS}$ AND 0.6m IN GARDENS.



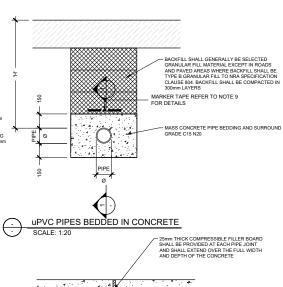
GRANULAR BEDDING SHALL BE SINGLE SIZED AGGREGATE 10mm OR GRADED AGGREGATE 14mm-5mm. FILL SHALL BE PLACED ON EITHER SIDE OF THE PIPE AND OVER THE PIPE IN LAYERS NOT EXCEEDING 100mm, EACH LAYER BEING COMPACTED BY HAND UNIT. THE PIPE HAS A MAXIMUM OF 150mm

uPVC PIPES BEDDED IN GANULAR MATERIAL
SCALE: 1:20

NOTE:
DETAIL APPLIES WHERE COVER 'H' IS GREATER THAN 1.2m IN
ROADS AND PAVED AREAS, 0.9m IN FIELDS AND FOOTPATHS AND 0.6m IN GARDENS

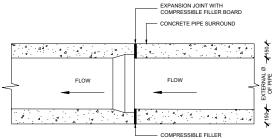


SECTION A-A CROSS SECTION OF DRAINAGE PIPEWORK WITHIN BUILDINGS



SECTION 2-2 SCALE: 1:20

NOTE: DETAIL APPLIES WHERE COVER 'H' IS <u>LESS THAN</u> 1.2m IN ROADS AND PAVED AREAS, 0.9m IN FIELDS AND FOOTPATHS AND 0.6m IN GARDENS.



SPIGOT AND SOCKET JOINT

Drawing Stage:

PRE-PLANNING

Project Details: Site Address: MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN Client: MONAGHAN COUNTY COUNCIL Architect HENRY J LYONS ARCHITECTS M&E Designer Contractor:

ANY DISCREPANCIES BETWEEN DRAWINGS, SKETCHES VERBAL INSTRUCTIONS AND SITE CONDITIONS TO BE REFERRED TO THE DESIGN TEAM BEFORE WORK COMMENCES.

ALL ENGINEERS DRAWINGS TO BE READ IN CONJUNCTION 6. ALL LEVELS ARE STRUCTURAL UNLESS NOTED WITH RELEVANT DRAWINGS AND SPECIFICATIONS. 6. OTHERWISE.

Drawn by Checked by: Approved by: JFC MAR '23 JPC KO'M Project Name Scale: Project Number CIVIC OFFICES A.S. @ A1 2223 Drawing Title: STANDARD DRAINAGE DETAILS Project: Originator: Zone: MCC CORA ZZ

CONSULTING ENGINEERS e-mail: info@cora.ie
Web: www.cora.ie

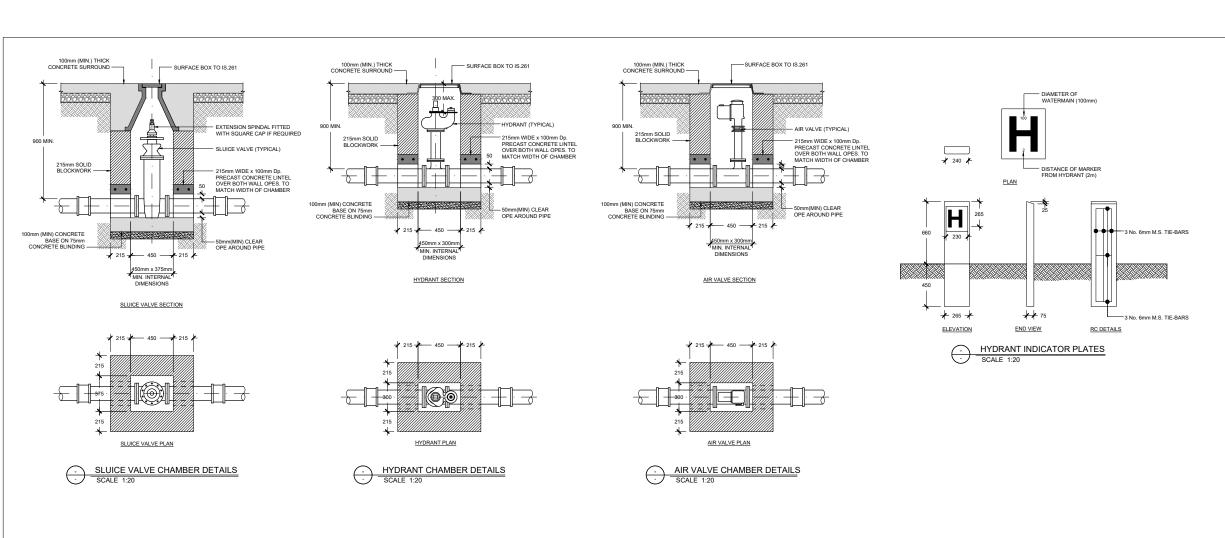
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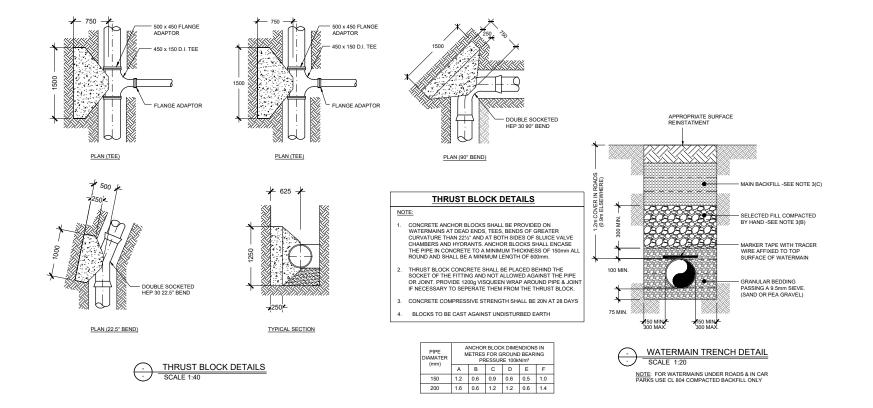
Behan House 10 Lower Mount St. Dublin 2. D02 HT71

Tel: +353 (0)1 661 1100

Discipline: Drawing No: Stage: Revision: Type: DR С 0013

REVISION SCHEDULE





	REVISION SCHEDULE											
REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKED								
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC								
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC								

NOTES:

- THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL ENGINEERS & ARCHITECT'S DRAWINGS FIGURED DIMENSIONS ONLY (NOT SCALING) TO BE USED. WHERE A CONFLICT OF INFORMATION EXISTS OF IF IN ANY DOUBLY 38K.
- 2. CONSULTANTS TO BE INFORMED IMMEDIATELY OF ANY DISCREPANCIES BEFORE WORK
- 3. PIPE BEDDING

A. GRANULAR BEDDING FOR PIPES TO BE EITHER:
(i) FREE DRAINING COARSE SAND OR
(ii) 10mm NOMINAL SINGLE SIZED GRAVEL OR
(iii) 5mm TO 10mm GRADED GRAVEL

GRAVELS IN (ii) OR (iii) CAN BE EITHER ROUNDER OR ANGULAR BUT WITH GOOD SELF COMPACTING PROPERTIES.

B. GRANULAR SURROUND & COVER FOR PIPES TO BE AS FOLLOWS:

TABLE 5	/3 GRANULAR MATERIALS 1	TO BS882
NOMINAL PIPE PIPE DIA. (mm)	AGGREGATE TYPE	NOMINAL MAXIMUM AGGREGATE SIZE (mm)
NOT	SINGLE SIZED	10
EXCEEDING 140	GRADED	-
EXCEEDING 140 BUT NOT	SINGLE SIZED	10, 14 OR 20
EXCEEDING 400	GRADED	20 TO 5 OR 14 TO 5
EXCEEDING 400	SINGLE SIZED	10, 14, 20 OR 40
EACEEDING 400	GRADED	14 TO 5 20 TO 5 OR 40 TO 5

C. BACKFILL

(I) BACKFILL UNDER FOOTPATH / CYCLEPATH / GRASS VERGE TO BE SELECTED BACKFILL TO NRA SPECIFICATION FOR ROAD WORKS CLAUSE 601, TABLE 6/1 CLASS 2C MATERIAL. COMPACTED IN 300mm LAYERS.
(II) BACKFILL UNDER CARRIAGEWAYS TO BE SELECTED GRANULAR MATERIAL TO NRA SPECIFICATION FOR ROADWORKS CLAUSE 601, TABLE 6/1 CLASS 6F1 OR 6F2 MATERIAL. COMPACTED IN 300mm LAYERS.

D. WARNING MESH

ALL PIPEWORK SHALL HAVE A 400mm WIDE WATER WARNING MESH - PLYAGE HR 40D BLUE POLYETHYLENE WARNING MESH OR SIMILAR - WITH TRACER WIRE LAID DIRECTLY OVER CENTRELINE OF THE PIPELINE AND TIED TO VALVES AT A DEPT OF 350mm BELOW THE FINISHED GROUND SURFACE.

4. PIPE TESTING

A PRESSURE TEST

PRESSURE TEST

ALL WATERMAINS SHOULD BE HYDRAULICALLY TESTED AFTER LAYING, FOR A PERIOD OF 24 HOURS, AT A TEST PRESSURE OF 1.5 TIMES THE SPECIFIED CLASS PRESSURE (1.5.3 BAR, THE HOURS, AT A TEST PRESSURE OF 1.5 TIMES THE SPECIFIED CLASS PRESSURE (1.5.3 BAR, THE ACTION OF THE STATE OF THE PRESSURE GAUGE. IS CONNECTED TO THE WATERMAIN AND OPERATED UNTIL THE CAUGE SHOWS THE FOULIED THE STATE OF THE STATE OF THE SUBJECT OF THE STATE O NOMINAL PIPE DIAMETER, PER KILOMETER OF LENGTH, PER 24 HOURS. A REPRESENTATIVE OF DROGHEDA BOROUGH COUNCIL TO BE PRESENT DURING TESTING.

B. CHLORINATION TEST

ALL MAINS SHALL BE SWABBED AND DISINFECTED BEFORE BEING PUT IN TO SUPPLY. THE PIPELINES SHALL BE DISINFECTED WITH WATER HAVING A MINIMUM CONCENTRATION OF 20mg/l OF FREE AVAILABLE CHLORINE.

THE CHLORINATED WATER SHALL BE LEFT IN THE MAIN FOR A PERIOD OF AT LEAST 24 HOURS. CHLORINE RESIDUAL TESTS SHALL BE TAKEN AT THE END OF THE MAIN FURTHERS FROM THE POINT OF INJECTION. THE STREINLISATION PROCESS SHALL BE REPEATED IF THE CHLORINE RESIDUAL IS LESS THAN 10mg/I. THE CHLORINATED WATER SHALL BE DISCHARGED INTO FOUL SEWER AFTER TESTING IS COMPLETED.

C. BACTERIOLOGICAL TEST

THE MAIN SHALL BE REFILLED AND A SAMPLE OF THE WATER SHALL BE TAKEN FOR BACTERIOLOGICAL ANALYSIS. CREAT CARE SHALL BE TAKEN WHEN DOTAINING SAMPLES FOR TESTING AND ONLY STERLIE CONTAINERS SHALL BE USED. THIS SAMPLING SHALL BE CARRIED OUT IN THE PRESENCE OF A REPRESENTATIVE OF DROGHEDA BOROUGH COUNCIL. SAMPLES SHALL BE TESTED WITHIN 6 HOURS OF COLLECTION.

HYDRANTS

HYDRANTS SHALL BE MANUFACTURED IN ACCORDANCE WITH BS 750: 1984 TYPE 2 AND SHALL HYDRAN IS SHALL BE MANUPAL UNLED IN ALCOPRORACE WITH BY 2001 1984 1YPE 2 AND SHALL INCORPORATE A SCREW-DOWN VALVE, UNDERFROUND 'GUIDE IN HEAD' TYPE WITH BAYONET LUG OUTLETS AND FALSE SPINDLE CAP, ALL TO BROGHEDA BOROUGH COUNCILS APPROVAL. THE HYDRANT VALVE SHALL BE ANTI-CLOCKWISE OPENING.

6. SLUICE VALVES

SLUICE VALVES SHALL BE MANUFACTURED IN ACCORDANCE WITH BS 5163:1986. THEY SHALL BE DOUBLE FLANGED: DUCTILE IRON RESILIENT SEAL, GATE VALVES, ALL TO DROCHEDA BOROUGH COUNCILS APPROVAL SLUICE VALVES SHALL BE ANTI-CLOCKWISE CLOSING.

AIR VALVES

AIR VALVES TO BE MANUFACTURED IN ACCORDANCE WITH BS5159 AND TO BE TO DROGHEDA BOROUGH COUNCILS APPROVAL.

8. SURFACE COVERS

HYDRANT, SLUICE VALVE & AIR VALVE CHAMBERS SHOULD BE PROVIDED WITH CAST IRON SURFACE BOXES IN COMPLIANCE WITH THE REQUIREMENTS OF IS. 261. SURFACE BOXES FOR ROADWAYS AND AREAS ACCESSIBLE TO WHEEL TRAFFIC SHOULD BE SUBJECT TO THE APPROVAL OF DROGHEDA BOROUGH COUNCIL.

9. HYDRANT AND VALVE INDICATOR PLATES

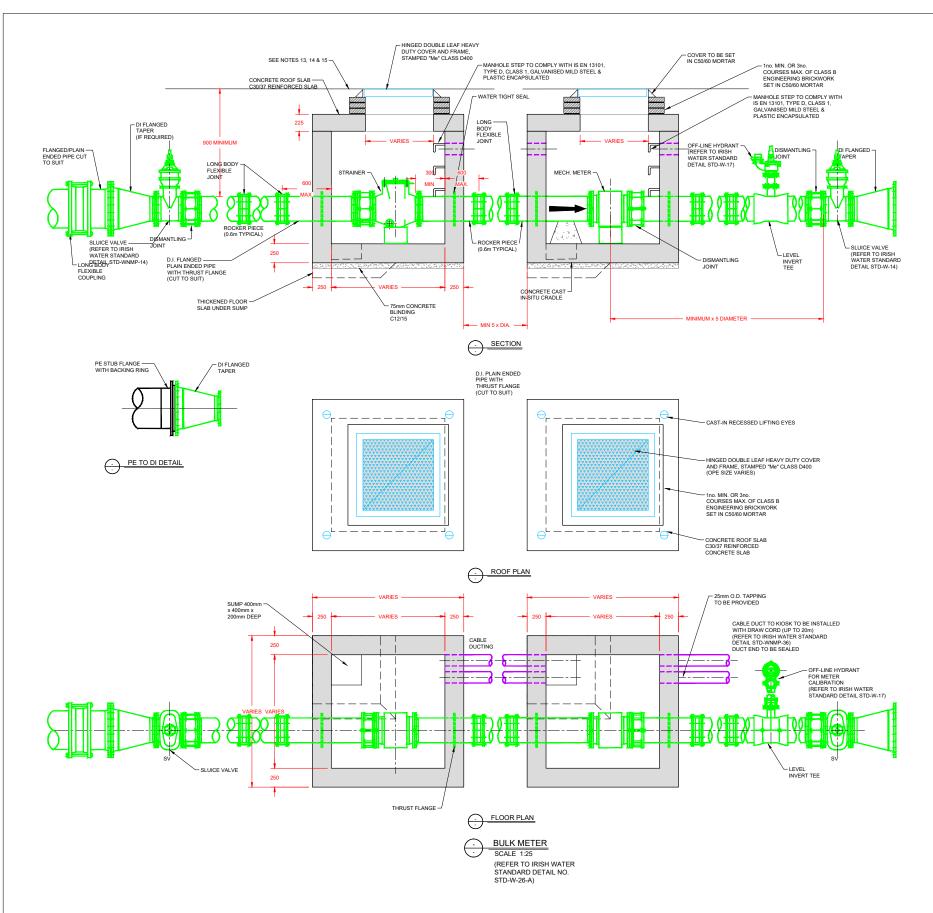
THE LOCATION OF HYDRANTS, AIR VALVES AND SLUICE VALVES SHOULD BE SHOWN BY INDICATOR PLATES POSITIONED TO THE APPROVAL OF THE LOCAL AUTHORITY HYDRANT PLATES SHOULD COMPLY WITH THE REQUIREMENTS OF BS2ST. THEY SHOULD SHOW THE DIAMETER OF THE WATERMAIN IN MILLIMETERS ON THE UPPER PART OF THE PLATE AND THE DISTANCE OF THE MARKER FROM THE HYDRANT ON THE LOWER PART OF THE PLATE AND THE DISTANCE OF THE MARKER FROM THE HYDRANT ON THE LOWER PART OF THE PLATE. ALL CHARACTERS SHOULD BE BLACK AND THE REMAINDER OF THE FRONT FACE SHOULD CONFORM TO COLOUR DEFERENCE NO. 309 (CAMARY VELLOW) OF BS 381C.

SLUICE VALVE. SCOUR VALVE AND AIR VALVE PLATES SHOULD BE CAST IRON MEASURING 200 X 200mm. THEY SHOULD HAVE A BACKGROUND IN BLACK BITUMASTIC PAINT WITH THE LETTERS IN WHITE FINANCE.

WHITE ENNAMEL.

NOICATOR PLATES MAY BE FIXED TO SOLID BOUNDARY WALLS, WHERE MARKER POSTS ARE USED THEY SHOULD BE CONSTRUCTED AS SHOWN ON THE DRAWING.

Drawing Stage:	Project Details:	J. Marie J.		Drawn by: Checked by: Approved		Drawn by: Checked by: Approved		Drawn by: Checked by: Approved		Checked by: Approved I		Date:				Behan H	House, er Mount St
	Site Address:	MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN	1. ALL ENGINEERS DRAWINGS TO BE READ IN CONJUNCTION 6. ALL LEVELS ARE STRUCTURAL UNLESS NOTED WITH RELEVANT DRAWINGS AND SPECIFICATIONS. OTHERWISE.	JPC	KO'M	JFC		MAR	'23		-		2. D02 HT71				
PRE-PLANNING	Client:	MONAGHAN COUNTY COUNCIL	2. ALL SETTING UIT TO ARCHITECTS DRAWNISS.	Project Name:			Scale:	Project Number:									
Architect: HENRY J LYONS ARCHITECTS		HENRY J LYONS ARCHITECTS	3. DO NOT SCALE FROM ENGINEERS DRAWINGS.		CIVIC OFFICES		A.S. @ A1 2223		3 C	CONSULTING ENGINEERS Web: www.cora.ie							
	M&E Designer:		A. ANY DISCREPANCIES RETWEEN DRAWINGS, SKETCHES, VERBAL INSTRUCTIONS AND SITE CONDITIONS TO BE	Drawing Title:	RD DRAINAGE DETAILS	Р	oject: C	Originator:	Zone: L	Level: Type:	Discipline:	Drawing No:	Stage: Revision:				
	Contractor:		REFERRED TO THE DESIGN TEAM BEFORE WORK COMMENCES.	STANDA	SHEET 4	3	мсс	CORA	ZZ	ZZ DR	С	0014	PL2				



ALL ENGINEERS DRAWINGS TO BE READ IN CONJUNCTION WITH RELEVANT DRAWINGS AND SPECIFICATIONS.

ANY DISCREPANCIES BETWEEN DRAWINGS, SKETCHES VERBAL INSTRUCTIONS AND SITE CONDITIONS TO BE REFERRED TO THE DESIGN TEAM BEFORE WORK COMMENCES.

ALL SETTING OUT TO ARCHITECTS DRAWINGS.

Project Details:

Site Address:

M&E Designer: Contractor:

Client:

Architect:

MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN

MONAGHAN COUNTY COUNCIL

HENRY J LYONS ARCHITECTS

Drawing Stage:

PRE-PLANNING

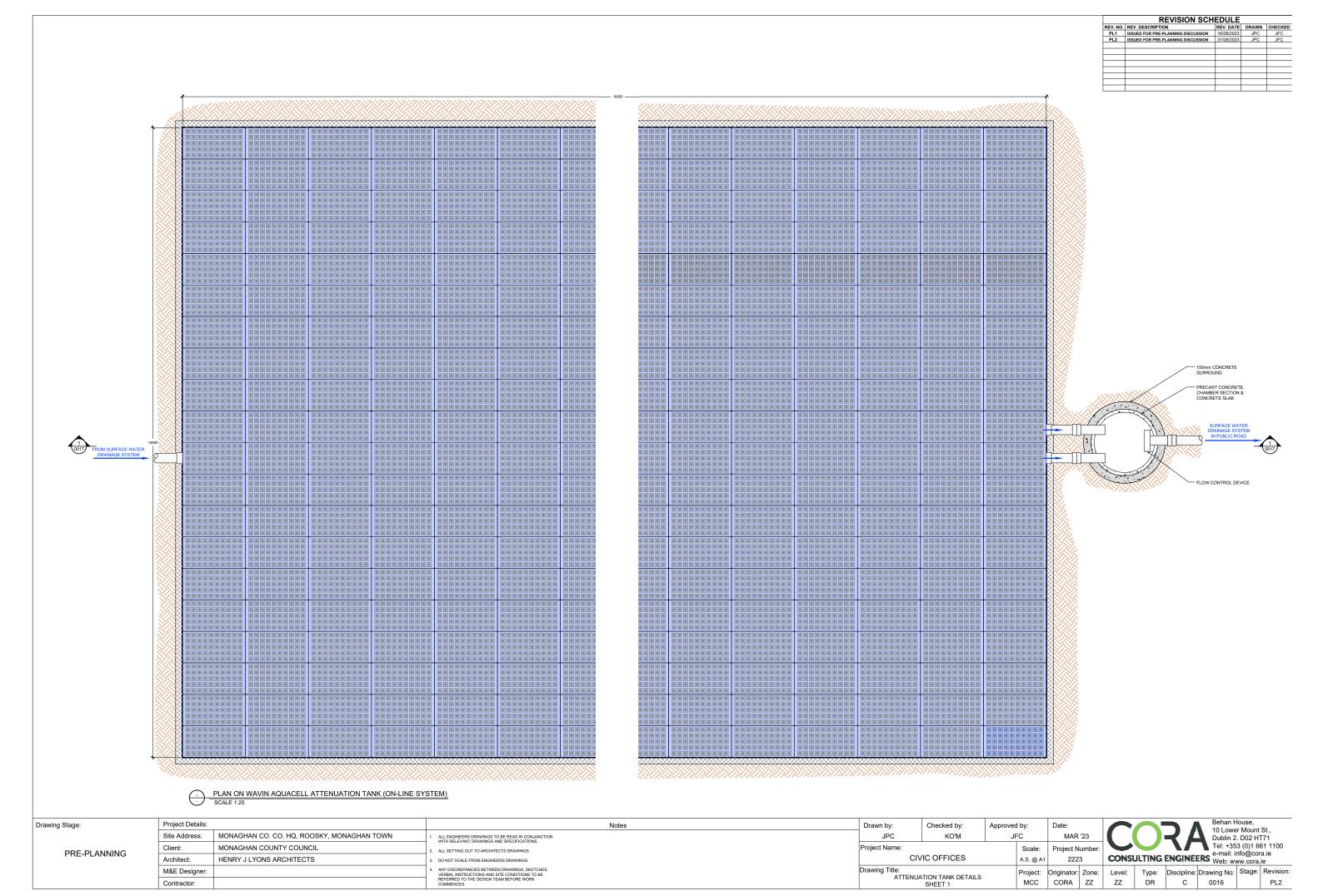
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REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKE
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC

DEVISION SCHEDIII E

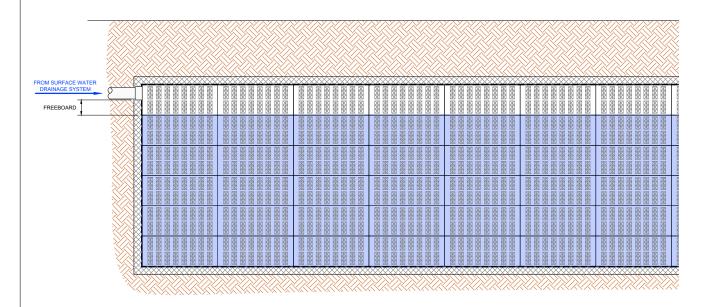
1. ALL DIMENSIONS ARE IN MILLIMETRES MIN) UNLESS NOTED OTHERWISE.
2. STRUCTURAL DESIGN AND REINFORCEMENT DETAIL TO BE PROVIDED BY THE DEVELOPER AND STRUCTURAL DESIGNED TO CARRY ALLIVE LOADS & CONSIST OF A REINFORCEMENT DETAIL TO BE PROVIDED BY THE DEVELOPER AND DEVELOPER. AND THE PROVIDED STRUCTURE TO THE PROVIDED TO CARRY ALLIVE LOADS & CONSIST OF A REINFORCED CONNECTE SLAD OF INSETTU CONCRETE. CRADE COSION, WITH A MINIMUM THICKNESS OF 225MM. AL TERNATIVELY, PRE-CAST CONCRETE ROOFS MAY BE USED, SUBJECT TO IRISH WATER REVIEW, & COMPLIANCE WITH BS 5011, PART 4.
3. CONCRETE FOR CHAMBERS TO BE C30 / 37.
4. PRECAST UNITS COMPLETED WITH RUBBER SEALING GASKET BETWEEN UNITS, COMPLYING WITH THE REQUIREMENTS OF IS EN 1917 AND BS 5911-PART 3, COMPLETE WITH 150MM CONCRETE SURROUND MAY BE USED AS AN ACCEPTABLE ALTERNATIVE. CONCRETE SURROUND TO BE GRADE C1920 IN ACCORDANCE WITH IS EN 206.
5. CHAMBERS SHALL BE COVERED WITH APPROVED HEAVY DUTY METAL COVERS TO IS EN 124 RATING D400.
5. CHAMBERS SHALL BE COVERED WITH APPROVED HEAVY DUTY METAL COVERS TO IS EN 124 RATING D400.
6. COVER AND FRAME SHALL BE SUITABLE FOR ROAD AND TRAFFIC CONDITIONS AND IS SUBJECT TO REVIEW BY IRISH WATER.
6. 20MM ALROON THE STANDED SET PLAYED AND ARE ARE AS A PREAS.
7. CHAMBERS SHALL BE SUITABLE FOR ROAD AND TRAFFIC CONDITIONS AND IS SUBJECT TO REVIEW BY IRISH WATER.
8. DUTTLE BROW PIPES AND FITTINGS TO BE IN ACCORDANCE WITH IS EN 12201-2011.
8. DUTCH LE ROON PIPES AND FITTINGS TO BE IN ACCORDANCE WITH IS EN 12201-2011.
9. ALL CHAMBERS TO BE CHECKED FOR ULFIFT BY THE DEVELOPER BASED ON GROUND CONDITIONS WITHIN THE SITE. SHOULD ANTI FLOATATION MEASURES BE REQUIRED THEY SHALL BE SUBJECT TO REVIEW BY IRISH WATER.

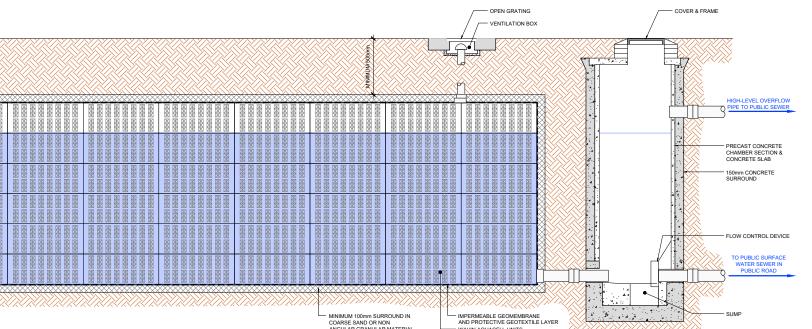
10. PIPEWORK TO BE DOWNISIZED TO ACCOMMODATE THE REQUIRED RANGE OF THE FLOW METER. STRAIGHT PIPE LERGINES UPSTAGE AND ADDITIONS WITHIN THE SITE. SHOULD ANTIFICATION MEASURES BE REQUIRED THEY SHALL BE SHALL BE PROVIDED WITH APPROPRIATE VALVES, FITTINGS AND PIPEWORK.
11. ALL CONCRETE TO BE IN ACCORDANCE WITH IS BE 2001.
12. AS SINGLE METER CAND

Drawn by: JPC	Checked by: KO'M	Approved	d by: C	Date: MAR	'23	($\frac{1}{2}L$	Behan H 10 Lowe Dublin 2	r Mount	
Project Name: CIV	Scale: A.S. @ A1	Project N 222	umber: 3	CONSI	- ` -	0)1 661 1100 @cora.ie cora.ie					
Drawing Title: STANDARD DRAINAGE DETAILS SHEET 5			Project: MCC	Originator: CORA	Zone: ZZ	Level: ZZ	Type: DR	Discipline: C	Drawing No: 0015	Stage:	Revision: PL2



	REVISION SCH	EDULE		
REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKED
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC





SECTION THROUGH WAVIN AQUACELL ATTENUATION TANK (ON-LINE SYSTEM)

Drawing Stage:	Project Details:		Notes	Drawn by:	Checked by:	Approved by:	Date:			<u> </u>		an House, ower Mour	
	Site Address:	MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN	ALL ENGINEERS DRAWINGS TO BE READ IN CONJUNCTION WITH RELEVANT DRAWINGS AND SPECIFICATIONS.	JPC	KO'M	JFC	MAF	R '23		1-7		lin 2. D02 F	
PRE-PLANNING	Client:	MONAGHAN COUNTY COUNCIL	ALL SETTING OUT TO ARCHITECTS DRAWINGS.	Project Name:		Scale:	Project N	Number:				+353 (0)1	
PRE-PLAININING	Architect:	HENRY J LYONS ARCHITECTS	DO NOT SCALE FROM ENGINEERS DRAWINGS.	Cl	VIC OFFICES	A.S. @ A1	222	23	CONSULT	NG ENGI	HEERS Web): www.cor	a.ie
	M&E Designer:		ANY DISCREPANCIES BETWEEN DRAWINGS, SKETCHES, VERBAL INSTRUCTIONS AND SITE CONDITIONS TO BE	Drawing Title:	ATION TANK DETAILS	Project:	Originator	r: Zone:	Level: Ty	/pe: Discipli	ine: Drawing 1	No: Stage	e: Revision:
	Contractor:		REFERRED TO THE DESIGN TEAM BEFORE WORK COMMENCES.	ATTENU	SHEET 2	мсс	CORA	ZZ	ZZ [JR C	0017		PL2

	REVISION SCHEDULE					
REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKED		
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC		
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC		

FROM SURFACE WATER

DRAINAGE SYSTEM

| Manual Residual Re

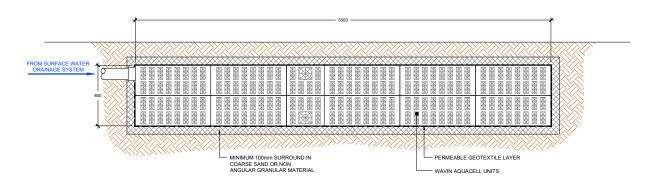
AquaCell Unit 6LB100

100mm diameter pre-formed socket

WAVIN AQUACELL UNIT
NOT TO SCALE

PLAN ON WAVIN AQUACELL SOAKAWAY

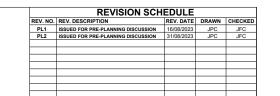
SCALE 1:25



SECTION THROUGH WAVIN AQUACELL SOAKAWAY

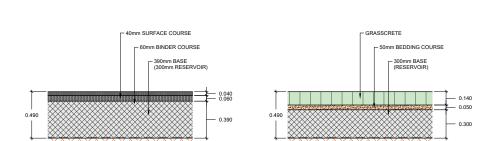
SCALE 1:25

Drawing Stage:	Project Details:		Notes	Drawn by:	Checked by:	Approved by:	Date:				Behan House, 10 Lower Mount St.,
	Site Address:	MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN	LALL ENGINEERS DRAWINGS TO BE READ IN CONJUNCTION WITH RELEVANT DRAWINGS AND SPECIFICATIONS.	JPC	KO'M	JFC	MA	R '23		14/	Dublin 2. D02 HT71
PRE-PLANNING	Client:	MONAGHAN COUNTY COUNCIL	2. ALL SETIMO OUT TO ARCHITECTS DRAWNOS.	Project Name:		Scale	: Project	Number:			Tel: +353 (0)1 661 1100
FRE-FLAINING	Architect:	HENRY J LYONS ARCHITECTS	3. DO NOT SCALE FROM ENGINEERS DRAWINGS.		CIVIC OFFICES	A.S. @	A1 22	23	CONSULTI	NG ENGIN	e-mail: info@cora.ie Web: www.cora.ie
	M&E Designer:		4. ANY DISCREPANCIES BETWEEN DRAWINGS, SKETCHES, VERBAL INSTRUCTIONS AND SITE CONDITIONS TO BE	Drawing Title:	SOAKAWAY DETAILS	Projec	: Originato	r: Zone:	Level: Ty	/pe: Discipline	ne: Drawing No: Stage: Revision:
	Contractor:		REFERRED TO THE DESIGN TEAM BEFORE WORK COMMENCES.		SOARAWAT DETAILS	MCC	CORA	ZZ	ZZ C	JR C	0018 PL2









ASSUMED 300mm OF ATTENUATION STORAGE

ASPHALT AREAS IN CIRCULATION AREAS

ASPHALT AREAS

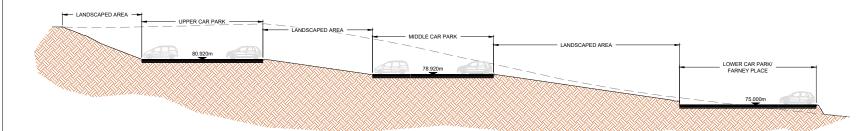
PERMEABLE PAVING AREAS IN SPACES

PERMEABLE PAVING AREAS

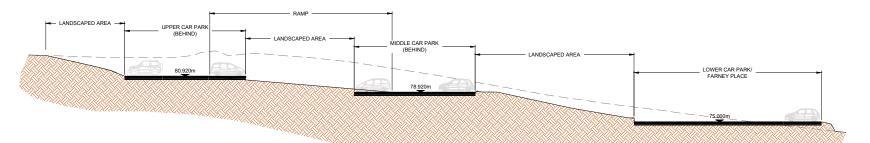
TYPICAL SECTION THRU' CAR PARK CIRCULATION/PARKING SPACES SCALE 120

LANDSCAPED AREA MIDDLE CAR PARK LANDSCAPED AREA RAMP 78.920m	LANDSCAPED AREA	UPPER CAR PARK —	+			
80.920m LANDSCAPED AREA RAMP			LANDSCAPED AREA	†		
80,920m				MIDDLE CAR PARK	1410004050 4054	
		80 920m			LANDSCAPED AREA	RAMP
78.920m - 78.920m		PRINCE CONTRACTOR OF THE PRINCE OF THE PRINC	NAME OF THE OWNER O			
				78.920m		
					THE RESERVE TO THE PERSON OF T	

SITE SECTION 1-1 SCALE 1:250



SITE SECTION 2-2 SCALE 1:250

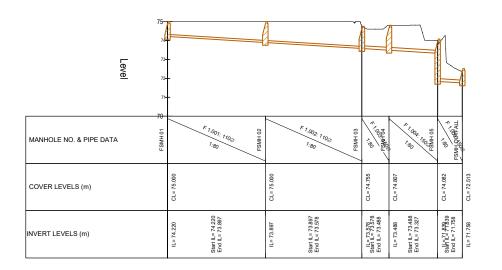


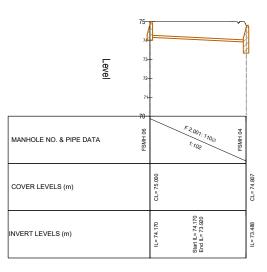
SITE SECTION 3-3 SCALE 1:250

Drawing Stage:	Project Details:		Notes	Drawn by:	Checked by:	Approved by:	Date:			Behan House, 10 Lower Mount Street,
	Site Address:	MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN		JPC	KO'M	JFC	FEB '23		$\prec \Delta$	Dublin 2. D02 HT71
DDE DI ANNUNC	Client:	MONAGHAN COUNTY COUNCIL		Project Name:		Scale:	Project Number:			Tel: +353 1 661 1100
PRE-PLANNING	Architect:	HENRY J LYONS ARCHITECTS			CIVIC OFFICES	A.S. @ A1	2223	CONSULTING	ENGINEERS	e-mail: info@cora.ie Web: www.cora.ie
	M&E Designer:			Drawing Title:	E SECTIONS - SHEET 1	Project:	Originator: Zone:	Level: Type:	Discipline: Drawin	ng No: Stage: Revision:
	Contractor:			311	E SECTIONS - SHEET T	MCC	CORA ZZ	ZZ DR	C 100	01 PL2

FSMH 01 - FSMH OUTFALL - LONGSECTION SCALE: H 1:500 / V 1:100 DATUM: 70.000 FSMH 06 - FSMH 04 - LONGSECTION SCALE: H 1:500 / V 1:100 DATUM: 70.000

	REVISION SCH	EDULE		
REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKED
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC





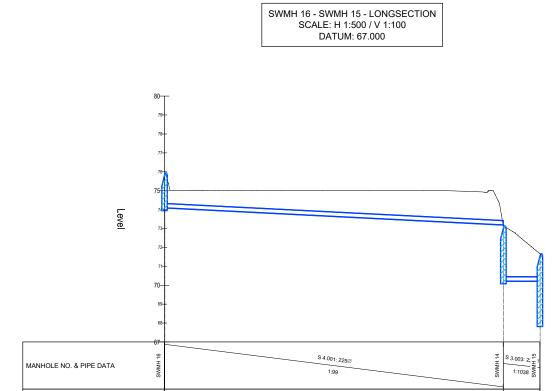
Drawing Stage:	Project Details:		Notes	Drawn by:	Checked by:	Approved by		Date:				Behan House, 10 Lower Mount Street.
	Site Address:	MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN		JPC	KO'M	JFC		FEB '23			$\prec L$	Dublin 2. D02 HT71
DDE DI ANNING	Client:	MONAGHAN COUNTY COUNCIL		Project Name:		S	cale: F	Project Number	er:			Tel: +353 1 661 1100 e-mail: info@cora.ie
PRE-PLANNING	Architect:	HENRY J LYONS ARCHITECTS		CIV	/IC OFFICES	1:50	0 @ A1	2223	CON	SULTING	ENGINEE	Web: www.cora.ie
	M&E Designer:			Drawing Title:	WER LONGSECTIONS	Pro	ject: Or	iginator: Zon	e: Level:	Type:	Discipline: Dra	wing No: Stage: Revision:
	Contractor:			FOUL SE	SHEET 1	N N	icc (CORA ZZ	ZZ	DR	С	2301 PL2

REV. NO. REV. DESCRIPTION REV. DATE DRAWN CHECKED ALIGNMENT - LS - SWMH 01 - SWMH 09 - LONGSECTION SCALE: H 1:500 / V 1:100 DATUM: 70.000 SWMH 10 - SWMH 07 - LONGSECTION SWMH 07 - SWMH XX - LONGSECTION SCALE: H 1:500 / V 1:100 SCALE: H 1:500 / V 1:100 DATUM: 70.000 DATUM: 65.000 Level ATTENUATION TANK Level S 1.007: 225Ø S 2.001: 225Ø S 1.005: 100∅ S 1.006: 100Ø MANHOLE NO. & PIPE DATA MANHOLE NO. & PIPE DATA MANHOLE NO. & PIPE DATA 1:200 1:20(\$ 1:200 1:200 COVER LEVELS (m) COVER LEVELS (m) COVER LEVELS (m) IL= 76.530 Start IL= 76.530 End IL= 76.483 Start IL= 68.208 End IL= 67.963 Start IL= 80.420 End IL= 78.632 INVERT LEVELS (m) INVERT LEVELS (m) INVERT LEVELS (m) SWMH 17 - SWMH 15 - LONGSECTION SCALE: H 1:500 / V 1:100 DATUM: 67.000 SWMH 19 - SWMH 15 - LONGSECTION SCALE: H 1:500 / V 1:100 DATUM: 67.000 Level S 6.001: 225Ø S 5.001: 225Ø MANHOLE NO. & PIPE DATA MANHOLE NO. & PIPE DATA COVER LEVELS (m) COVER LEVELS (m) Start IL= 69.815 End IL= 69.690 Start IL= 69.603 End IL= 69.561 Start IL= 73.390 End IL= 73.211 Start IL= 71.250 End IL= 71.212 Start IL= 69.669 End IL= 69.623 INVERT LEVELS (m) INVERT LEVELS (m) Project Details: Drawing Stage: Notes Drawn by: Checked by: Approved by: 10 Lower Mount Street, Dublin 2. D02 HT71 Tel: +353 1 661 1100 Site Address: MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN FEB '23 JPC KO'M JFC Client: MONAGHAN COUNTY COUNCIL Project Number Scale: e-mail: info@cora.ie Web: www.cora.ie PRE-PLANNING CIVIC OFFICES 2223 CONSULTING ENGINEERS HENRY J LYONS ARCHITECTS 1:500 @ A1 Architect: M&E Designer: Drawing Title: SURFACE WATER LONGSECTIONS Originator: Zone: Type: Discipline: Drawing No: Stage: Revision: Project: Level: Contractor: MCC CORA ZZ ZZ DR С 2401

SWMH 12 - SWMH 15 - LONGSECTION SCALE: H 1:500 / V 1:100 DATUM: 65.000

	REVISION SCH	EDULE		
REV. NO.	REV. DESCRIPTION	REV. DATE	DRAWN	CHECKE
PL1	ISSUED FOR PRE-PLANNING DISCUSSION	16/08/2023	JPC	JFC
PL2	ISSUED FOR PRE-PLANNING DISCUSSION	31/08/2023	JPC	JFC
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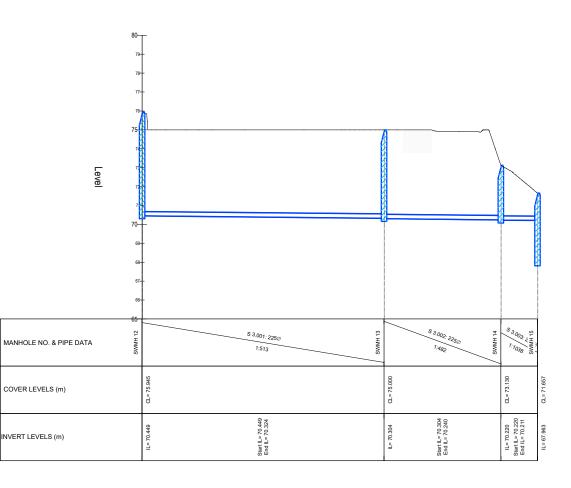
Behan House,
10 Lower Mount Street,
Dublin 2. D02 HT71
Tel: +353 1 661 1100
e-mail: info@cora.ie
Web: www.cora.ie
Level: Type: Discipline: Drawing No: Stage: Revision:
ZZ DR C 2402 PL2



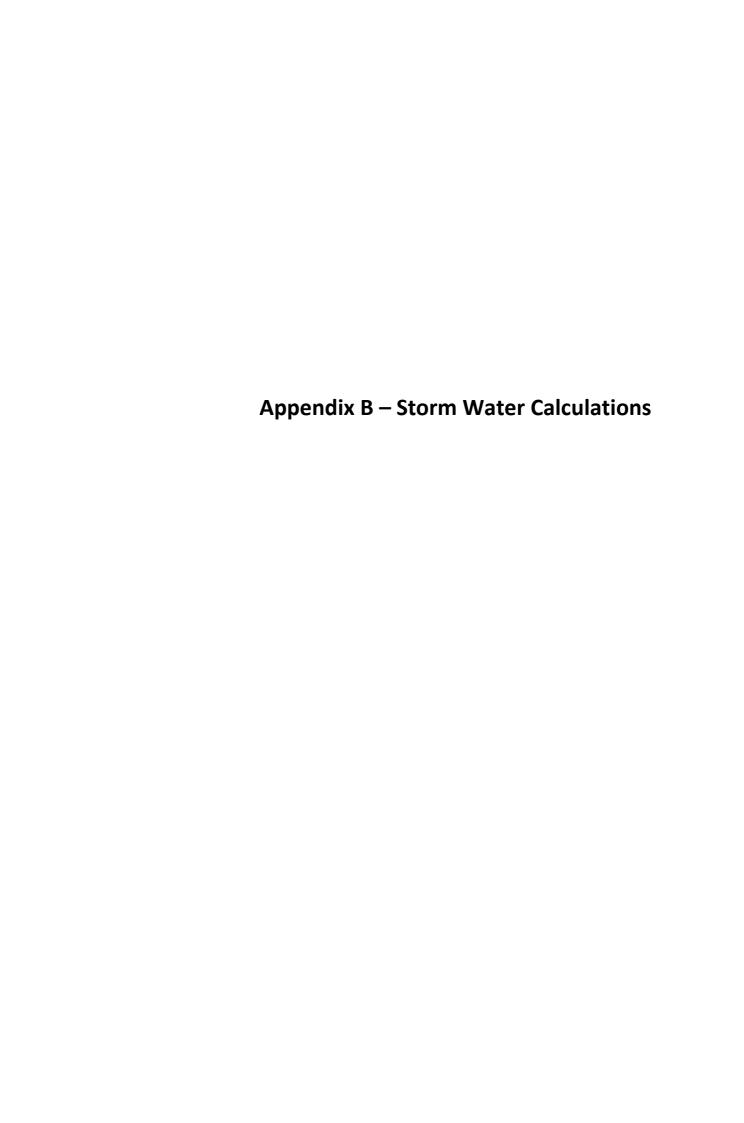
Start IL= 74.093 End IL= 73.188 IL= 70.220 Start IL= 70.220 End IL= 70.211

COVER LEVELS (m)

INVERT LEVELS (m)



Drawing Stage:	Project Details:		Notes	Drawn by:	Checked by:	Approved by:	Date:	
	Site Address:	MONAGHAN CO. CO. HQ, ROOSKY, MONAGHAN TOWN		JPC	KO'M	JFC	FEB '	'23
DDE DI ANNING	Client:	MONAGHAN COUNTY COUNCIL		Project Name:		Scale:	Project N	umber:
PRE-PLANNING	Architect:	HENRY J LYONS ARCHITECTS		CI	/IC OFFICES	1:500 @ A1	1 2223	3 4
	M&E Designer:			Drawing Title:	WATER LONGSECTION	Project:	Originator:	Zone:
	Contractor:			SURFACE	SHEET 2	MCC	CORA	ZZ





Drainage Input Data

Site Coordinates:

	IG	ITM
E:	267521	667466
N:	333834	833850

Standard Average Annual Rainfall, SAAR = 988 mm (From Met Éireann Historical Data)

SOIL Type = 3

SOIL = 0.37

QBAR = **4.3 l/s/ha**

Overall Site Area, ha = 1.84 ha (From Site Plan)

QBAR = 7.92 l/s

Soil Infiltration Rate, f = 0.0000427 m/s (From Soils Tests)

0.15372 m/hr

Growth Curve Factors:

 1 Year
 0.85

 30 Year
 2.13

 100 Year
 2.61

 200 Year
 2.86

Met Eireann
Return Period Rainfall Depths for sliding Durations
Irish Grid: Easting: 267521, Northing: 333834,

	Interval					1	Years								
DURATION	6months, 1year,	2,	3,	4,	5,	10,	20,	30,	50,	75,	100,	150,	200,	250,	500,
5 mins	2.5, 3.6,	4.1,	5.0,	5.6,	6.0,	7.5,	9.2,	10.4,	12.0,	13.4,	14.5,	16.2,	17.6,	18.7,	N/A,
10 mins	3.5, 4.9,	5.7,	6.9,	7.8,	8.4,	10.5,	12.9,	14.4,	16.7,	18.7,	20.2,	22.6,	24.5,	26.0,	N/A,
15 mins	4.1, 5.8,	6.8,	8.2,	9.1,	9.9,	12.3,	15.1,	17.0,	19.6,	22.0,	23.8,	26.6,	28.8,	30.6,	N/A,
30 mins	5.4, 7.6,	8.8,	10.5,	11.7,	12.6,	15.6,	19.0,	21.3,	24.4,	27.2,	29.4,	32.7,	35.3,	37.5,	N/A,
1 hours	7.1, 9.9,	11.4,	13.5,	15.0,	16.1,	19.8,	23.9,	26.6,	30.4,	33.8,	36.4,	40.3,	43.4,	45.9,	N/A,
2 hours	9.4, 12.9,	14.7,	17.4,	19.2,	20.6,	25.1,	30.1,	33.4,	37.9,	41.9,	44.9,	49.6,	53.2,	56.2,	N/A ,
3 hours	11.1, 15.1,	17.2,	20.2,	22.3,	23.8,	28.9,	34.4,	38.1,	43.1,	47.5,	50.9,	56.0,	60.0,	63.3,	N/A ,
4 hours	12.5, 16.8,	19.1,	22.5,	24.7,	26.4,	31.8,	37.9,	41.8,	47.2,	51.9,	55.6,	61.1,	65.3,	68.8,	N/A ,
6 hours	14.7, 19.7,	22.3,	26.0,	28.5,	30.4,	36.6,	43.3,	47.7,	53.7,	58.9,	62.9,	69.0,	73.7,	77.5,	N/A ,
9 hours	17.3, 23.0,	25.9,	30.2,	33.0,	35.1,	42.0,	49.5,	54.4,	61.0,	66.8,	71.2,	77.9,	83.0,	87.2,	N/A ,
12 hours	19.4, 25.7,	28.9,	33.5,	36.6,	38.9,	46.4,	54.5,	59.7,	66.8,	73.0,	77.8,	84.9,	90.4,	94.9,	N/A ,
18 hours	22.9, 30.0,	33.6,	38.9,	42.3,	44.9,	53.3,	62.3,	68.1,	76.0,	82.8,	88.0,	95.9,	101.9,	106.8,	N/A ,
24 hours	25.7, 33.5,	37.4,	43.1,	46.9,	49.7,	58.8,	68.5,	74.7,	83.2,	90.6,	96.2,	104.6,	111.0,	116.2,	134.0,
2 days	33.2, 41.9,	46.3,	52.5,	56.5,	59.5,	68.9,	78.9,	85.2,	93.7,	101.0,	106.4,	114.6,	120.8,	125.8,	142.7,
3 days	39.6, 49.1,	53.7,	60.3,	64.6,	67.7 ,	77.6,	87.9,	94.3,	103.0,	110.3,	115.8,	124.0,	130.1,	135.1,	151.8,
4 days	45.3, 55.5,	60.4,	67.4,	71.8,	75.1,	85.3,	95.9,	102.5,	111.3,	118.8,	124.3,	132.6,	138.7,	143.7,	160.3,
6 days	55.7, 67.0,	72.4,	79.9,	84.7,	88.2,	99.1,	110.3,	117.1,	126.2,	133.9,	139.6,	148.0,	154.2,	159.3,	175.9,
8 days	65.2, 77.4,	83.2,	91.2,	96.3,	100.0,	111.4,	123.1,	130.2,	139.6,	147.4,	153.2,	161.8,	168.1,	173.2,	190.0,
10 days	74.2, 87.1,	93.3,	101.7,	107.0,	111.0,	122.8,	134.9,	142.2,	151.8,	159.9,	165.8,	174.5,	180.9,	186.1,	203.0,
12 days	82.7, 96.4,	102.9,	111.7,	117.2,	121.3,	133.5,	145.9,	153.4,	163.3,	171.5,	177.5,	186.4,	192.9,	198.1,	215.2,
16 days	99.0, 113.9,	120.9,	130.4,	136.2,	140.6,	153.5,	166.5,	174.4,	184.6,	193.1,	199.3,	208.4,	215.0,	220.3,	237.6,
20 days	114.6, 130.5,	137.9,	147.9,	154.1,	158.6,	172.2,	185.7,	193.8,	204.3,	213.0,	219.4,	228.6,	235.4,	240.8,	258.3,
25 days	133.4, 150.4,	158.2,	168.8,	175.3,	180.0,	194.2,	208.2,	216.6,	227.4,	236.3,	242.8,	252.3,	259.2,	264.6,	282.4,
NOTES:						J									

N/A Data not available

These values are derived from a Depth Duration Frequency (DDF) Model

For details refer to:

M5-60m = 16.1

Ratio R = M5-60m/M5-2d

M5-2d = 59.5

R = 16.1/59.5

R = 0.271

SAAR = 988mm (from Historical Data)

^{&#}x27;Fitzgerald D. L. (2007), Estimates of Point Rainfall Frequencies, Technical Note No. 61, Met Eireann, Dublin', Available for download at www.met.ie/climate/dataproducts/Estimation-of-Point-Rainfall-Frequencies_TN61.pdf



Greenfield runoff rate estimation for sites

www.uksuds.com | Greenfield runoff tool

Calculated by:	John Callanan
Site name:	2223 - MCC
Site location:	Roosky, Monaghan

Site Details

Latitude: 54.24982° N

Longitude: 6.96516° W

This is an estimation of the greenfield runoff rates that are used to meet normal best practice Reference: criteria in line with Environment Agency guidance "Rainfall runoff management for developments", SC030219 (2013), the SuDS Manual C753 (Ciria, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

2827752064

Aug 17 2023 11:19

Runoff estimation approach

IH124

Site characteristics

Total site area (ha): 1

Methodology

QBAR estimation method:

V_{BAR} estimation method:

SPR estimation method:

Calculate from SPR and SAAR

Calculate from SOIL type

Notes

(1) Is $Q_{BAR} < 2.0 \text{ l/s/ha}$?

When Q_{BAR} is < 2.0 l/s/ha then limiting discharge rates are set at 2.0 l/s/ha.

Soil characteristics

Default

Edited

HOST class:

SOIL type:

SPR/SPRHOST:

4	3
N/A	N/A
0.47	0.37

(2) Are flow rates < 5.0 l/s?

Where flow rates are less than 5.0 l/s consent for discharge is usually set at 5.0 l/s if blockage from vegetation and other materials is possible. Lower consent flow rates may be set where the blockage risk is addressed by using appropriate drainage elements.

Hydrological characteristics

SAAR (mm):

Hydrological region:

Growth curve factor 1 year.

Growth curve factor 30 years:

Growth curve factor 100 years:

Growth curve factor 200 years:

Default	Edited
1016	988
13	13
0.85	0.85
1.65	1.65

1.95

2.15

(3) Is SPR/SPRHOST ≤ 0.3?

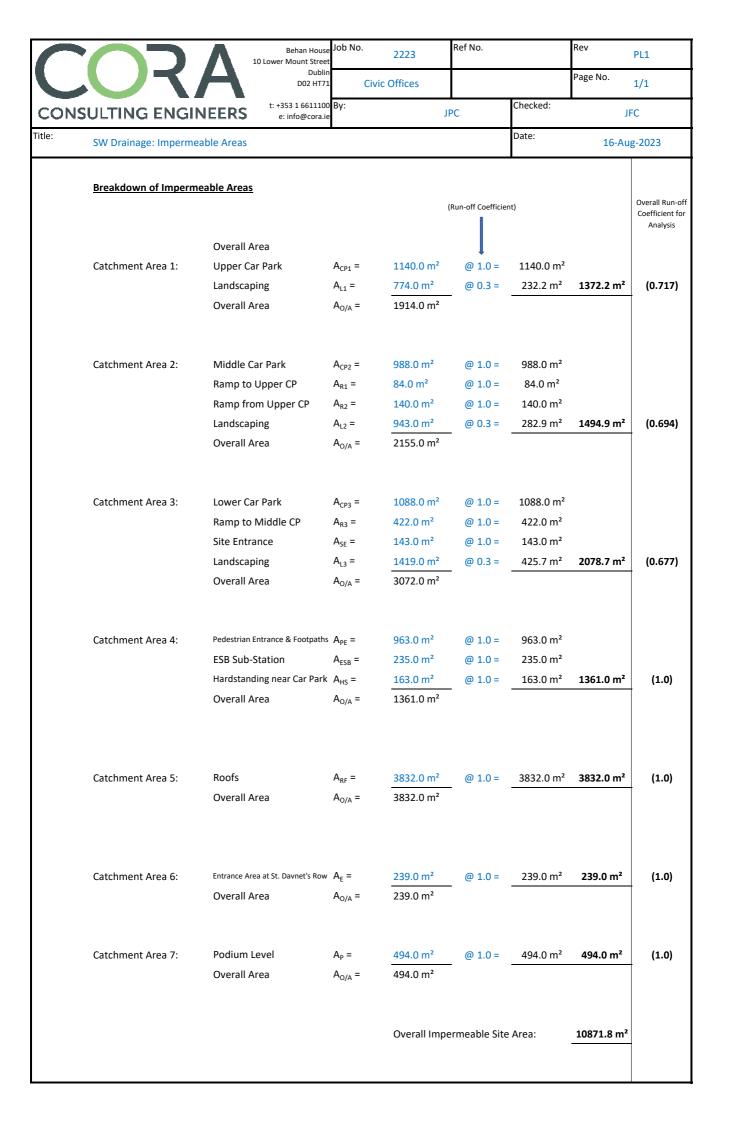
Where groundwater levels are low enough the use of soakaways to avoid discharge offsite would normally be preferred for disposal of surface water runoff.

1.95

2.15

Q _{BAR} (I/s):	7.47	4.3
1 in 1 year (l/s):	6.35	3.65
1 in 30 years (l/s):	12.32	7.09
1 in 100 year (l/s):	14.56	8.38
1 in 200 years (l/s):	16.05	9.24

This report was produced using the greenfield runoff tool developed by HR Wallingford and available at www.uksuds.com. The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at www.uksuds.com/terms-and-conditions.htm. The outputs from this tool are estimates of greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, CEH, Hydrosolutions or any other organisation for the use of this data in the design or operational characteristics of any drainage scheme.



		Behan House 10 Lower Mount Street	2223	Ref No.		Rev PL1
	ノてA	Dublin D02 HT71	Civic Offices			Page No. 1/1
CONSULT	TING ENGINEERS	t: +353 1 6611100 e: info@cora.ie	,	JPC	Checked:	JFC
Title: SW I	Orainage: Storage Volume & I	nfiltration under Ca	ar Park (80.920)		Date:	16-Aug-2023

Length, I = 71.5 m

Width, w = 16 m

Depth, d = 300 mmFree Volume, V_{free} = 30%M5-60min from Met Éireann Data = 16.1 mmM5-60m/M5-2d from Met Éireann Data, r = 0.271

Return Period, years = 100 year + 20%

 $\label{eq:main_equation} \begin{array}{ll} \mbox{Impermeable Area, A =} & \mbox{1372.2 m}^2 \\ \mbox{Soil Infiltration Rate, f =} & \mbox{0.0000427 m/s} \\ \mbox{Surface Area to 50% storage depth, A}_{\rm s50} = & \mbox{26.25 m}^2 \\ \end{array}$

Outflow Factor, AF = $0.001120875 \text{ m}^3/\text{s}$

Max. Inflow (from Podium Permavoid) = 0.0 l/s = $0 \text{ m}^3/\text{s}$

Duration	M5 Rainfalls	100 year Rainfall	100 year Rainfall	Inflow	Additional Inflow	Outflow	Storage Required
			+ 20%	(m³)	(m³)	(m³)	(m³)
5 mins	6.0 mm	14.5 mm	17.4 mm	23.9	0	0.3	23.5
10 mins	8.4 mm	20.2 mm	24.2 mm	33.3	0	0.7	32.6
15 mins	9.9 mm	23.8 mm	28.6 mm	39.2	0	1.0	38.2
30 mins	12.6 mm	29.4 mm	35.3 mm	48.4	0	2.0	46.4
1 hour	16.1 mm	36.4 mm	43.7 mm	59.9	0	4.0	55.9
2 hours	20.6 mm	44.9 mm	53.9 mm	73.9	0	8.1	65.9
3 hours	23.8 mm	50.9 mm	61.1 mm	83.8	0	12.1	71.7
4 hours	26.4 mm	55.6 mm	66.7 mm	91.6	0	16.1	75.4
6 hours	30.4 mm	62.9 mm	75.5 mm	103.6	0	24.2	79.4
9 hours	35.1 mm	71.2 mm	85.4 mm	117.2	0	36.3	80.9
12 hours	38.9 mm	77.8 mm	93.4 mm	128.1	0	48.4	79.7
18 hours	44.9 mm	88.0 mm	105.6 mm	144.9	0	72.6	72.3
24 hours	49.7 mm	96.2 mm	115.4 mm	158.4	0	96.8	61.6

Rainfall Values are taken from Met Éirean Data for the site (see separate data sheet)

Required Storage, S_{reqd} = **80.9 m³**

Actual Storage, $S_{act} = 102.96 \text{ m}^3$ (0.79)

Storage Volume is OK

Area: 1144.0 m²

Time for emptying half volume:

 $T_{s50} =$ 00 days 10 hr 01 min 39 s

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Title:	SW Drainage: Storage Volume & II	nfiltration under Ca	ar Park (78.920)		Date:	16-Au	g-2023

Length, I = 66 m

Width, w = 16 m

Depth, d = 300 mmFree Volume, V_{free} = 30%M5-60min from Met Éireann Data = 16.1 mmM5-60m/M5-2d from Met Éireann Data, r = 0.271

Return Period, years = 100 year + 20%

Impermeable Area, A = 1494.9 m²
Soil Infiltration Rate, f = 0.0000427 m/s

Surface Area to 50% storage depth, $A_{s50} = 24.6 \text{ m}^2$

Outflow Factor, AF = 0.00105042 m³/s

Max. Inflow (from Podium Permavoid) = 0.0 l/s = 0 m³/s

Duration	M5 Rainfalls	100 year Rainfall	100 year Rainfall	Inflow	Additional Inflow	Outflow	Storage Required
			+ 20%	(m³)	(m³)	(m³)	(m³)
5 mins	6.0 mm	14.5 mm	17.4 mm	26.0	0	0.3	25.7
10 mins	8.4 mm	20.2 mm	24.2 mm	36.2	0	0.6	35.6
15 mins	9.9 mm	23.8 mm	28.6 mm	42.7	0	0.9	41.7
30 mins	12.6 mm	29.4 mm	35.3 mm	52.7	0	1.9	50.8
1 hour	16.1 mm	36.4 mm	43.7 mm	65.3	0	3.8	61.5
2 hours	20.6 mm	44.9 mm	53.9 mm	80.5	0	7.6	73.0
3 hours	23.8 mm	50.9 mm	61.1 mm	91.3	0	11.3	80.0
4 hours	26.4 mm	55.6 mm	66.7 mm	99.7	0	15.1	84.6
6 hours	30.4 mm	62.9 mm	75.5 mm	112.8	0	22.7	90.1
9 hours	35.1 mm	71.2 mm	85.4 mm	127.7	0	34.0	93.7
12 hours	38.9 mm	77.8 mm	93.4 mm	139.6	0	45.4	94.2
18 hours	44.9 mm	88.0 mm	105.6 mm	157.9	0	68.1	89.8
24 hours	49.7 mm	96.2 mm	115.4 mm	172.6	0	90.8	81.8

Rainfall Values are taken from Met Éirean Data for the site (see separate data sheet)

Required Storage, S_{reqd} = 94.2 m³

Actual Storage, $S_{act} = 95.04 \text{ m}^3$ (0.99)

Storage Volume is OK

Area: 1056.0 m²

Time for emptying half volume:

 $T_{s50} =$ 00 days 12 hr 27 min 12 s

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Title: SW Drainage: Storage Volume &	Infiltration under Ca	ar Park (75.000)		Date:	16-Aug-2023

Length, I = 69 m Width, w =

16 m

Depth, d = 350 mm Free Volume, V_{free} = 30% M5-60min from Met Éireann Data = 16.1 mm M5-60m/M5-2d from Met Éireann Data, r = 0.271

Return Period, years = 100 year + 20%

Impermeable Area, A = 2078.7 m² Soil Infiltration Rate, f = 0.0000427 m/s Surface Area to 50% storage depth, A_{s50} = 29.75 m²

0.001270325 m³/s Outflow Factor, AF =

Max. Inflow (from Podium Permavoid) = 0.0 l/s $0 \text{ m}^3/\text{s}$

Duration	M5 Rainfalls	100 year Rainfall	100 year Rainfall	Inflow	Additional Inflow	Outflow	Storage Required
			+ 20%	(m³)	(m³)	(m³)	(m³)
5 mins	6.0 mm	14.5 mm	17.4 mm	36.2	0	0.4	35.8
10 mins	8.4 mm	20.2 mm	24.2 mm	50.4	0	0.8	49.6
15 mins	9.9 mm	23.8 mm	28.6 mm	59.4	0	1.1	58.2
30 mins	12.6 mm	29.4 mm	35.3 mm	73.3	0	2.3	71.0
1 hour	16.1 mm	36.4 mm	43.7 mm	90.8	0	4.6	86.2
2 hours	20.6 mm	44.9 mm	53.9 mm	112.0	0	9.1	102.9
3 hours	23.8 mm	50.9 mm	61.1 mm	127.0	0	13.7	113.2
4 hours	26.4 mm	55.6 mm	66.7 mm	138.7	0	18.3	120.4
6 hours	30.4 mm	62.9 mm	75.5 mm	156.9	0	27.4	129.5
9 hours	35.1 mm	71.2 mm	85.4 mm	177.6	0	41.2	136.4
12 hours	38.9 mm	77.8 mm	93.4 mm	194.1	0	54.9	139.2
18 hours	44.9 mm	88.0 mm	105.6 mm	219.5	0	82.3	137.2
24 hours	49.7 mm	96.2 mm	115.4 mm	240.0	0	109.8	130.2

Rainfall Values are taken from Met Éirean Data for the site (see separate data sheet)

Required Storage, S_{reqd} = 139.2 m³

Actual Storage, $S_{act} =$ 115.92 m³ (1.20)

Storage Volume is NOT OK

Area: 1104.0 m²

NOTE: Anything above the following will be directed to the overflow pipe

100yr + 0% Climate Change is 0.92 50yr + 20% Climate Change is 0.96

Time for emptying half volume:

 $T_{s50} =$

00 days 15 hr 13 min 05 s

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Titl	e: SW Drainage: Attenuation Storage	e Volume under Po	dium			Date:	16-Au	ig-2023

Calculation of Attenuation Volume

Length, I = 33.6 m
Width, w = 10 m
Area: 336.0 m²

Depth, d = 0.08 mFree Volume, V_{free} = 95%M5-60min from Met Éireann Data = 16.1 mmM5-60m/M5-2d from Met Éireann Data, r = 0.271

Return Period, years = 100 year + 20%

Contributory Area, A = 494 m²

Outflow Factor, AF = $\frac{2.0 \text{ l/s}}{}$ (0.002 m³/s)

Max. Inflow (from RWHT) = 0.0 l/s = $0 \text{ m}^3/\text{s}$

Duration	M5 Rainfalls	100 year Rainfall	100 year Rainfall	Inflow	Additional Inflow	Outflow	Storage Required
			+ 20%	(m³)	(m³)	(m³)	(m³)
5 mins	6.0 mm	14.5 mm	17.4 mm	8.6	0	0.6	8.0
10 mins	8.4 mm	20.2 mm	24.2 mm	12.0	0	1.2	10.8
15 mins	9.9 mm	23.8 mm	28.6 mm	14.1	0	1.8	12.3
30 mins	12.6 mm	29.4 mm	35.3 mm	17.4	0	3.6	13.8
1 hour	16.1 mm	36.4 mm	43.7 mm	21.6	0	7.2	14.4
2 hours	20.6 mm	44.9 mm	53.9 mm	26.6	0	14.4	12.2
3 hours	23.8 mm	50.9 mm	61.1 mm	30.2	0	21.6	8.6
4 hours	26.4 mm	55.6 mm	66.7 mm	33.0	0	28.8	4.2
6 hours	30.4 mm	62.9 mm	75.5 mm	37.3	0	43.2	-5.9
9 hours	35.1 mm	71.2 mm	85.4 mm	42.2	0	64.8	-22.6
12 hours	38.9 mm	77.8 mm	93.4 mm	46.1	0	86.4	-40.3
18 hours	44.9 mm	88.0 mm	105.6 mm	52.2	0	129.6	-77.4
24 hours	49.7 mm	96.2 mm	115.4 mm	57.0	0	172.8	-115.8

Rainfall Values are taken from Met Éirean Data for the site (see separate data sheet)

Required Storage, S_{reqd} = 14.4 m³

Actual Storage, $S_{act} = 25.54 \text{ m}^3$ (0.56)

Storage Volume is OK

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Title:	SW Drainage: Attenuation Storage	e Volume			Date:	16-Au	g-2023

Calculation of Attenuation Volume

Length, I = 18 m
Width, w = 10 m
Area: 180.0 m²

Depth, d = 2 mFree Volume, V_{free} = 95%M5-60min from Met Éireann Data = 16.1 mmM5-60m/M5-2d from Met Éireann Data, r = 0.271

Return Period, years = 100 year + 20%

Contributory Area, A = 5193 m²

Outflow Factor, AF = $\frac{7.92 \text{ J/s}}{(0.00792 \text{ m}^3/\text{s})}$

Max. Inflow (from RWHT) = 0.0 l/s = $0 \text{ m}^3/\text{s}$

Duration	M5 Rainfalls	100 year Rainfall	100 year Rainfall	Inflow	Additional Inflow	Outflow	Storage Required
			+ 20%	(m³)	(m³)	(m³)	(m³)
5 mins	6.0 mm	14.5 mm	17.4 mm	90.4	0	2.4	88.0
10 mins	8.4 mm	20.2 mm	24.2 mm	125.9	0	4.8	121.1
15 mins	9.9 mm	23.8 mm	28.6 mm	148.3	0	7.1	141.2
30 mins	12.6 mm	29.4 mm	35.3 mm	183.2	0	14.3	169.0
1 hour	16.1 mm	36.4 mm	43.7 mm	226.8	0	28.5	198.3
2 hours	20.6 mm	44.9 mm	53.9 mm	279.8	0	57.0	222.8
3 hours	23.8 mm	50.9 mm	61.1 mm	317.2	0	85.5	231.7
4 hours	26.4 mm	55.6 mm	66.7 mm	346.5	0	114.0	232.4
6 hours	30.4 mm	62.9 mm	75.5 mm	392.0	0	171.1	220.9
9 hours	35.1 mm	71.2 mm	85.4 mm	443.7	0	256.6	187.1
12 hours	38.9 mm	77.8 mm	93.4 mm	484.8	0	342.1	142.7
18 hours	44.9 mm	88.0 mm	105.6 mm	548.4	0	513.2	35.2
24 hours	49.7 mm	96.2 mm	115.4 mm	599.5	0	684.3	-84.8

Rainfall Values are taken from Met Éirean Data for the site (see separate data sheet)

Required Storage, S_{reqd} = 232.4 m³

Actual Storage, $S_{act} = 342.0 \text{ m}^3$ (0.68)

Storage Volume is OK

ADDITIONAL STORAGE FOR OVERFLOWS FROM OTHER AREAS

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CONS	SULTING ENGINEERS	t: +353 1 6611100 e: info@cora.ie	I '	PC	Checked:	JFC
Title:	SW Drainage: Soakaway - Storage	& Infiltration Calcu	ulation		Date:	16-Aug-2023

Length, I = 5.5 m Width, w = 3 m

Area: 16.5 m²

Depth, d = 800 mm Free Volume, V_{free} = 95% M5-60min from Met Éireann Data = 16.1 mm M5-60m/M5-2d from Met Éireann Data, r = 0.271

Return Period, years = 100 year + 20%

Impermeable Area, A = 239 m²

Soil Infiltration Rate, f = 0.0000427 m/s

Surface Area to 50% storage depth, A_{s50} = 6.8 m²

Outflow Factor, AF = 0.00029036 m³/s

Max. Inflow = 0.0 l/s $0 \text{ m}^3/\text{s}$

Duration	M5 Rainfalls	100 year Rainfall	100 year Rainfall	Inflow	Additional Inflow	Outflow	Storage Required
			+ 20%	(m³)	(m³)	(m³)	(m³)
5 mins	6.0 mm	14.5 mm	17.4 mm	4.2	0	0.1	4.1
10 mins	8.4 mm	20.2 mm	24.2 mm	5.8	0	0.2	5.6
15 mins	9.9 mm	23.8 mm	28.6 mm	6.8	0	0.3	6.6
30 mins	12.6 mm	29.4 mm	35.3 mm	8.4	0	0.5	7.9
1 hour	16.1 mm	36.4 mm	43.7 mm	10.4	0	1.0	9.4
2 hours	20.6 mm	44.9 mm	53.9 mm	12.9	0	2.1	10.8
3 hours	23.8 mm	50.9 mm	61.1 mm	14.6	0	3.1	11.5
4 hours	26.4 mm	55.6 mm	66.7 mm	15.9	0	4.2	11.8
6 hours	30.4 mm	62.9 mm	75.5 mm	18.0	0	6.3	11.8
9 hours	35.1 mm	71.2 mm	85.4 mm	20.4	0	9.4	11.0
12 hours	38.9 mm	77.8 mm	93.4 mm	22.3	0	12.5	9.8
18 hours	44.9 mm	88.0 mm	105.6 mm	25.2	0	18.8	6.4
24 hours	49.7 mm	96.2 mm	115.4 mm	27.6	0	25.1	2.5

Rainfall Values are taken from Met Éirean Data for the site (see separate data sheet)

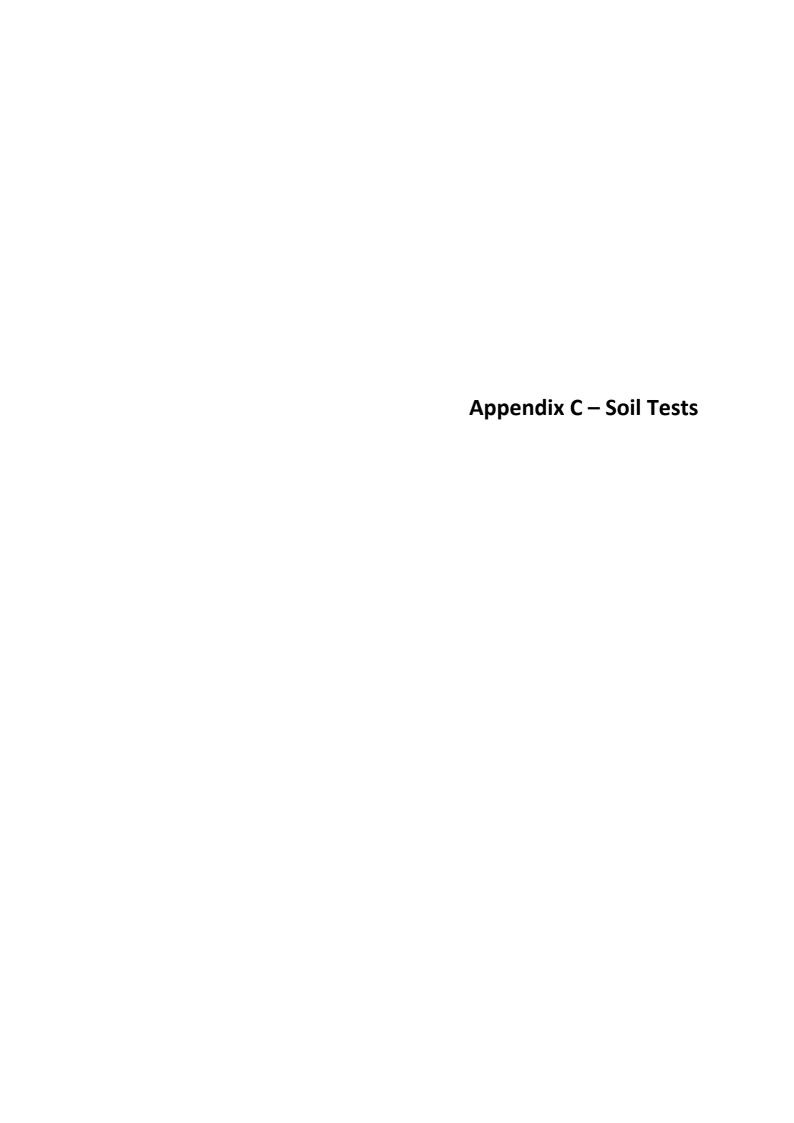
Required Storage, S_{reqd} = 11.8 m³

Actual Storage, $S_{act} =$ 12.54 m³ (0.94)

Storage Volume is OK

Time for emptying half volume:

00 days 05 hr 37 min 44 s $T_{s50} =$



PROPOSED DEVELOPMENT NEW CIVIC CENTRE MONAGHAN MONAGHAN CO. COUNCIL

CORA

CONSULTING ENGINEERS

CONTENTS

I	INTRODUCTION
П	FIELDWORK
Ш	TESTING
Ш	DISCUSSION / SUMMARY

APPENDICES

I	BORING RECORDS
II	ROTARY CORE LOGS
Ш	TRIAL PIT RECORDS
IV	BRE DIGEST 365 DATA
\mathbf{V}	LABORATORY
	a. Geotechnical Soil and Rock Data
	b. Chemical and Environmental Data
VI	SITE PLAN

FOREWORD

The following Conditions and Notes on Site Investigation Procedures should be read in conjunction with this report.

General.

Recommendations made, and opinions expressed in the report are based on the strata observed in the exploratory holes, together with the results of in-situ and laboratory tests. No responsibility can be held for conditions which have not been revealed by exploratory work, or which occur between exploratory hole locations. Whilst the report may suggest the likely configuration of strata, both between exploratory hole locations, or below the maximum depth of the investigation, this is only indicative, and liability cannot be accepted for its accuracy.

Unless specifically stated, no account has been taken of possible subsidence due to mineral extraction below or close to the site.

Standards

The ground investigation works for this project have been carried out by IGSL in accordance with Eurocode 7 - Part 2: Ground Investigation & Testing (EN 1997-2:2007). This has been used together with complementary documents such as BS 5930 (1999), BS 1377 (Parts 1 to 9) and Engineers Ireland Specification & Related Documents for Ground Investigation in Ireland (2006). The following Irish (IS) and European Standards or Norms are referenced:

- O IS EN 1997-2 Eurocode 7: 2007 Geotechnical Design Part 2: Ground Investigation & Testing
- O IS EN ISO 22475-1:2006 Geotechnical Investigation and Sampling Sampling Methods & Groundwater Measurements
- IS EN ISO 14688-1:2002 Geotechnical Investigation and Testing Identification and Classification of Soil, Part 1: Identification and Description
- IS EN ISO 14688-2:2004 Geotechnical Investigation and Testing Identification and Classification of Soil, Part 2: Classification Principles

Routine Sampling.

Undisturbed samples of soils, predominantly cohesive in nature are obtained unless otherwise stated by a 104mm diameter open-drive tube sampler or Piston Sampler. In granular soils, and where undisturbed sampling is inappropriate, disturbed samples are collected. Smaller disturbed samples are also recovered at intervals to allow a visual examination of the full strata section.

In-Situ Testing.

Standard penetration tests were conducted strictly in accordance with Section 4.6 of IS EN 1997-2:2007. The SPT equipment (hammer energy test) has been calibrated in accordance with EN ISO 22476-3:2005 to obtain the Energy Ratio (E_r) of each hammer. A calibration certificate is available upon request. The E_r is defined as the ratio of the actual energy E_{meas} (measured energy during calibration) delivered to the drive weight assembly into the drive rod below the anvil, to the theoretical energy (E_{theor}) as calculated from the drive weight assembly. The recorded number of blows (N) reported on the engineering logs are uncorrected. In sands, the energy losses due to rod length and the effect of the overburden pressure should be taken into account (see IS EN ISO 22476-3:2005).

Groundwater

The depth of entry of any influx of groundwater is recorded during the course of boring operations. However, the normal rate of boring does not usually permit the recording of an equilibrium level for any one water strike. Where possible drilling is suspended for a period of twenty minutes to monitor the subsequent rise in water level. Groundwater conditions observed in the borings or pits are those appertaining to the period of investigation. It should be noted however, that groundwater levels are subject to diurnal, seasonal and climatic variations and can also be affected by drainage conditions, tidal variations etc.

Engineering Logging

Soil and rock identification has been based on the examination of the samples recovered and conforms with IS EN ISO 14688-1:2002 and IS EN ISO 14689-1:2004.

Where peat has been encountered during site works, samples have been logged in accordance with the Von Post Classification (ref. Von Post, L. 1992. Sveriges Gologiska Undersoknings torvinventering och nogra av dess hittils vunna resultat (SGU peat inventory and some preliminary results) Svenska Mosskulturforeningens Tidskrift, Jonkoping, Swedden, 36, 1-37 & Hobbs N. B. Mire morphology and the properties of some British and foreign peats. QJEG, Vol. 19, 1986).

Retention of Samples.

After satisfactory completion of all the scheduled laboratory tests on any sample, the remaining material is discarded unless a period of retention of samples is agreed, it is our normal practice to discard all soil samples one month after submission of our final report.

Reporting

Recommendations made and opinions expressed in this report are based on the strata observed in the exploratory holes, together with the results of in-situ and laboratory tests. No responsibility can be held by IGSL Ltd for ground conditions between exploratory hole locations.

The engineering logs provide ground profiles and configuration of strata relevant to the investigation depths achieved and caution should be taken when extrapolating between exploratory points. No liability is accepted for ground conditions extraneous to the investigation points. Unless specifically stated, no account has been taken of possible subsidence due to mineral extraction, mining works or karstification below or close to the site.

This report has been prepared for the project client and the information should not be used without prior written permission. Any recommendations developed in this report specifically relate to the proposed development. IGSL Ltd accepts no responsibility or liability for this document being used other than for the purposes for which it was intended.

REPORT ON A SITE INVESTIGATION

NEW CIVIC OFFICES FOR MONAGHAN COUNTY COUNCIL

CORA CONSULTING ENGINEERS

Report No. 24665

July 2023

I Introduction

A major new development is proposed for a site in Monaghan where new Civic Offices are to be located.

An investigation of sub soil conditions in the area of the new development has been carried out by IGSL for CORA, Consulting Engineers, on behalf of Monaghan County Council.

The scheduled site investigation included the following elements.

*	Cable Percussion Boreholes	8 nr.
•	Rotary Core Holes	3 nr.
•	Standpipe Installations	1 nr.
•	Trial Pits	14 nr.
•	BRE Digest 365 Infiltration Tests	4 nr.
•	Geotechnical Laboratory Tests	
*	Chemical and Environmental Tests	

This report includes all factual data from field and laboratory operations and discusses these findings relative to foundation and infrastructural design for the proposed new development.

II Fieldwork

This development is to take place on an undulating greenfield site in Monaghan Town.

The exploratory locations are noted on the drawing enclosed in Appendix VI and were marked out by IGSL on site. All locations have been referenced to national grid and ground levels established.

The various elements of the investigation are detailed in the following paragraphs. All field works were supervised by an experienced geotechnical engineer who carefully recorded stratification, took photographs as necessary, recovered samples and prepared detailed records.

Close liaison was maintained throughout with CORA Consulting Engineers and Monaghan County Council personnel.

All appropriate documentation was submitted and approved prior to site commencement. Each location was scanned electronically (CAT) to ensure that existing services were not damaged. A shallow trial pit was also opened by hand at borehole / corehole locations to confirm this.

Drawings from the various utilities were also examined to ensure that major services were avoided.

Statutory HSE safety precautions relating to general safety and COVID 19 were strictly observed, with working areas restricted to IGSL personnel only, to ensure safety of the general public.

Boreholes

Boreholes were 200mm diameter and were constructed using conventional cable percussion equipment. Holes were referenced BH01 to BH08. A trial pit was opened at each borehole location to 1.00 metre deep to ensure that underground services were not damaged.

Shallow refusal was recorded on boulder obstructions at two locations (BH04 and BH06). Following a period of abortive chiselling, the equipment was moved by about 3 metres and re-bores were taken. These are referenced BH04A and BH06A.

Detailed geotechnical records are contained in Appendix I to this report - the records give details of stratification, sampling, in-situ testing and groundwater. Note is also taken of any obstructions to normal boring requiring the use of the heavy chisel for advancement. It was not possible to recover undisturbed samples because of the hard and granular nature of the strata encountered.

The findings are fairly consistent, with topsoil generally overlying a1.50 stratum of soft to firm brown sandy SILT/CLAY.

Stiff brown sandy gravelly CLAY, typically containing cobble and boulder material, is encountered at shallow depth (generally 0.50 to 1.00 metres). This stratum continues to about 2.50 metres where very stiff to hard dark grey gravelly CLAY is noted. This stratum also contains significant cobbles and boulder.

Boreholes were terminated on boulder obstructions in all locations at varying depths. Chiselling techniques were used in all locations in an attempt to advance borehole depths without success.

The stiff brown and grey gravelly CLAY encountered on this site is a GLACIAL TILL or BOULDER CLAY with the high percentage of coarse material typical of the stratum.

The increasing strength with depth pattern particularly in the base grey boulder clay is also noted. The final refusal depths are **NOT** indicative of rock horizon.

The borehole findings are summarised in the following **TABLE A:**

TABLE A

Ref	Sandy Clay	Stiff brown BC	Stiff grey BC	Refusal Depth
BH01	0.30 – 1.20	1.20 – 3.00		3.00
BH02	0.30 - 1.20 $0.30 - 0.70$	0.70 - 2.50	2.50 - 4.50	4.50
BH03	0.30 - 1.50	1.50 - 3.70	2.00	3.70
BH04	0.30 - 1.20			1.20
BH04A	0.30 - 0.50	0.50 - 2.50	2.50 - 4.50	4.50
BH05	0.30 - 1.50	1.50 - 2.00	2.00 - 4.50	4.50
BH06	0.30 - 0.50	0.50 - 1.00		1.00
BH06A	0.30 - 0.50	0.50 - 1.00		1.00
BH07	0.20 - 0.50	0.50 - 1.00		1.00
BH08	0.30 - 1.00	1.00 - 1.80	1.80 - 3.40	3.40

Ground water ingress was note in two locations, at 3.00 metre BGL in BH01 and at 4.50 metres BGL in BH05. The remaining boreholes were DRY.

Rotary Core Drilling

Rotary core drilling was employed at three of the borehole locations to advance investigation depth, establish bedrock horizon and recover representative rock core if practical.

A BT-44 drilling rig was used to drill in each location using triple tube core drilling technique and an air-mist coolant. Symmetrix open hole drilling (100mm diameter) was used through the overburden deposits.

Detailed drilling records are presented in Appendix II with accompanying core photographs. The records note Total and Solid Core Recovery (TCR / SCR) and provide a detailed geological description of the rock.

Drilling continued in each location to depths between 10.50 and 15.00 metres, penetrating very stiff to hard GLACIAL TILL consisting of brown or grey gravelly CLAY with extensive boulder presence.

Some core was recovered in the hard base till. The enclosed core photographs clearly indicate the significant boulder presence.

The strength of the boulder clay was established by standard penetration tests taken at 1.50 metre intervals during the drilling operation. Results are noted in the right hand column of the records. SPT values typically exceed N=40 with numerous test refusals recorded.

A slotted PVC standpipe was installed in RC02 to facilitate on-going monitoring of ground water level. The installation was sealed at surface and protected by a steel cover.

The rotary core findings are summarised in the following table.

TABLE B

Hole No.	Overburden	Core Recovered	Standpipe	
RC02	0 – 10.50		0 – 10.50	
RC03	0 - 10.50	8.10 - 10.50		
RC06	0 - 15.00	13.5 - 15.00		

Trial Pits

Trial Pits were scheduled in fourteen specified locations and referenced TP01 to TP14. A tracked excavator was used under engineering supervision. Detailed records for each location are presented in Appendix III. These records note the soil stratification and record sampling and ground water details.

Topsoil surface was noted in each location generally overlying a zone of soft SILT/CLAY. Firm to stiff brown or grey gravelly CLAY was then encountered, this stratum typically containing cobbles and boulders.

The findings are consistent with the stratification noted in the boreholes.

Several trial pits were terminated on large boulders. The findings are summarised and presented as follows:

TABLE C

Ref No.	Topsoil	Soft SILT- CLAY	Stiff gravelly CLAY	Water
TP01	0 - 0.25	0.25 - 0.90	0.90 - 1.80	Dry
TP02	0 - 0.25	0.25 - 0.55	0.55 - 1.40	Dry
TP03	0 - 0.20	0.20 - 0.80	0.80 - 1.70	Dry
TP04	0 - 0.10	0.10 - 0.60	0.60 - 1.80	Dry
TP05	0 - 0.20	0.20 - 0.50	0.50 - 2.10	Dry
TP06	0 - 0.25	0.25 - 0.80	0.80 - 1.50	1.00
TP07	0 - 0.25	0.25 - 1.50	1.50 - 1.90	Dry
TP08	0 - 0.25	0.25 - 0.50	0.50 - 2.20	Dry
TP09	0 - 0.25	0.25 - 0.50	0.50 - 1.50	Dry
TP10	0 - 0.30	0.30 - 0.50	0.50 - 2.50	Dry
TP11	0 - 0.20	0.20 - 1.10	1.10 - 2.30	Dry
TP12	0 - 0.30	0.30 - 1.00	1.00 - 1.80	Dry
TP13	0 - 0.20	0.20 - 0.50	0.50 - 1.40	Dry
TP14	0 - 0.20	0.20 - 0.50	0.50 - 2.10	Dry

Trial Pits were backfilled with the excavated spoil, compacted in layers, the disturbed areas were levelled and coarse material was removed.

BRE Digest 365 Test

Infiltration testing was performed at four locations as specified in accordance with BRE Digest 365 'Soakaway Design'. Tests are referenced SA01 to SA04. Detailed data is presented in Appendix IV.

To obtain a measure of the infiltration rate of the sub-soils, water is poured into the test pit, and records taken of the fall in water level against time. The test is carried out over two cycles following initial soakage.

The infiltration rate is the volume of water dispersed per unit exposed area per unit of time, and is generally expressed as metres/minute. In these calculations the exposed area is the sum of the base area and the average internal area of the permeable stratum over the test duration. Design is based on the slowest infiltration rate, which has been calculated from the final cycle.

The stratification in the test area comprised Topsoil over gravelly sandy SILT/CLAY.

Results are summarised as follows:

TABLE D

SA02 SA03	Depth	Soil Type	Infiltration Rate (1 (Metres/ Minute)		
SA 01	1.30	Gravelly CLAY	0.00173		
SA02	1.60	Gravelly CLAY	0.00023		
SA03	1.60	Gravelly CLAY	5.3E-05		
SA04	1.30	Gravelly CLAY	0.0000		

The results confirm low to very low permeability for the cohesive gravelly clay soils present on the site.

III. Testing

In Situ

Standard penetration tests were carried out at approximate 1.00 metre intervals in the geotechnical boreholes and at 1.50 metres in the Rotary Core Holes to measure relative in-situ soil strength. N values are noted in the right hand column of the individual records, representing the blow count required to drive the standard sampler 300mm into the soil, following initial seating blows. Where full test penetration was not achieved the blow count for a specific penetration is recorded, or refusal is indicated where appropriate. The results of the tests are summarised as follows:

STRATUM	N VALUE RANGE	COMMENT
Gravelly CLAY (Box	ılder Clay)	
1.00 m BGL	6 to 13	Soft to Firm
2.00 m BGL	10 to 29	Firm to Stiff
3.00 m BGL	26 to 50	Stiff to Hard
4.00 m BGL	> 50	Hard
4.00 to 15.00 m BGL		
(Rotary Holes)	40 to >50	Hard
		A 457A 37

Limited penetration SPT tests with refusal were recorded on numerous occasions, reflecting a high concentration of cobble / boulder material in the glacial till

Laboratory

A programme of laboratory testing was scheduled following completion of site operations. Geotechnical testing was carried out by IGSL in it's INAB-Accredited laboratory. Chemical and environmental testing was carried out in the UK by EUROFINS / CHEMTEST Ltd. The test programme included the following elements:

Liquid and Plastic Limits / Moisture Content IGS	SL
PSD Grading by Wet Sieve and Hydrometer IGS	SL
MCV IGS	SL
CBR IGS	SL
Compaction	SL
Organic Content EU	ROFINS
± 1	ROFINS
RILTA Suite Environmental EU	ROFINS

All laboratory data is presented in Appendices Va and Vb and individual tests are discussed briefly as follows:

Index Properties / Natural Moisture Content

Classification tests have been carried out on samples of the cohesive soils from borehole and trial pit locations.

The glacial tills plot generally in the CI/CL zone of the standard Classification chart indicative of low plasticity gravelly CLAY matrix material. Natural Moisture Content ranges from 14 to 19 %.

Grading

Wet sieve and hydrometer analysis has been carried out on samples of the cohesive soils from both boreholes and trial pits. The graphs are typically straight line, grading from the fine clay to coarse gravel fraction. The pattern is very typical of glacial till or boulder clay deposition.

Organic Content

Six samples of the soils from the site had organic contents established. Samples were generally taken from shallow depths below the topsoil. Values of 1.0 to 2.5% were determined indicative of very low to negligible organic content.

MCV/CBR/Compaction

Six large composite samples were selected from Trial Pits 01 / 03 / 05 / 09 / 12 and 14 and a series of tests were scheduled to establish the soil characteristics relative to possible re-use during the new development.

The tests carried out included MCV (Moisture Condition Value), Natural Moisture Content, CBR (California Bearing Ratio), Dry Density / Moisture Content relationship.

The results are summarised as follows:

Ref No.		TP03	TP05	TP09	TP12	TP14
Depth	0.70	0.6	0.7	0.7	0.8	1.5
Natural MC (%)	15 ,	13	13	13	10	14
MCV	6.6	7.3	6.8	6.8	6.7	7.8
CBR (%)	5.6	4.6	4.1	2.0	7.7	3.0
Max.Dry Density (mg/cu.m.) 1.90	1.86	1.86	1.88	1.89	1.85
Optimum Moisture (%)	11	12	12	12	12	14

Chemical Suite (Sulphate Chloride pH)

Six samples were sent for analysis to BRE Chemical Suite parameters.

Sulphate concentrations (SO4 2:1 extract) of <0.010 to 0.240 g/l were established with pH values ranging from 7.8 to 8.6. Chloride concentrations (<0.010 to 0.24 g/l) were also determined.

The results indicate a design class of DS-1 (ACEC Classification for Concrete) for sulphate concentrations below 0.5 g/l. No special precautions are necessary to protect below ground foundation concrete.

RILTA Environmental Suite

Six samples of the sub soils were sent to specialist environmental laboratory EUROFINS and testing was carried out in accordance with RILTA requirements to establish Landfill Waste Acceptance Criteria (WAC).

Detailed results are presented in Appendix V o. All samples tested fall into the INERT category with no elevated contaminant levels recorded.

Material excavated from this site can be safely disposed of either within the site boundary or off site to a suitably licensed Landfill Facility

Asbestos screening was carried out on all RILTA samples with no traces of Asbestos noted.

A comprehensive Waste Characterisation Assessment may be required by landfill operators. This can be prepared by specialist environmental consultants using the factual data from field and laboratory as presented in this report.

IV. Discussion:

A major development is being undertaken at this site in Monaghan. A new CIVIC CENTRE is to be constructed for Monaghan County Council.

A detailed investigation of subsoil and bedrock has been carried out under the direction of CORA Consulting Engineers in the area of development.

The exploratory locations are detailed on the site plan in Appendix VI.

The factual data from the field and laboratory is presented in Sections 1 to III of this report.

The site is grassed with some significant variation in ground level.

SUMMARY STRATIFICATION

TOPSOIL overlies soft to firm sandy SILT/CLAY. This upper material extends to depth between 0.50 and 1.50 metres as shown in TABLE A and TABLE B.

Stiff brown gravelly CLAY (brown BOULDER CLAY) is then encountered and continues to about 2.00 metres where it changes to very stiff to hard grey gravelly CLAY (grey black BOULDER CLAY). Proof core drilling confirmed that the GLACIAL TILL continues to at least 15 metres BGL. Bedrock was not established.

UPPER SILT/CLAY

The soils extending from surface to depths up to 1.50 metre are described as soft to firm sandy slightly gravelly SILT/CLAY. SPT values of N=6 to N=10 have been recorded at a depth of 1.00 metre.

BOULDER CLAY

GLACIAL TILL or BOULDER CLAY has been confirmed below approximately 1.50 metres, the stratum continues to at least 15.00 metres BGL. Visual inspection of trial pit excavations and results of in-situ Standard Penetration Tests are indicative of stiff to hard consistency.

The characteristics of the regional boulder clay or glacial till are very well documented and the findings from this detailed investigation are consistent with extensive published data.

ALLOWABLE BEARING PRESSURES

The soil strength has been assessed visually in the trial pits and confirmed by Standard Penetration Tests in boreholes and core holes. The allowable bearing pressures indicated by the field data are summarised as follows:

Depth	Average N Value	Allowable Bearing Pressure				
1.00	7	75 kPa	(Upper Silt/Clay)			
2.00	20	200 kPa	(Boulder Clay)			
3.00	35	300 kPa	` ',			
4.00	>50	400 kPa				

FOUNDATION RECOMMENDATIONS

The use of traditional reinforced foundations for the new Civic Offices development is proposed. Foundations to be placed on the stiff brown or grey boulder CLAY using the allowable bearing pressures indicated above.

We strongly recommend visual inspection of foundation excavations by experienced personnel to ensure uniformity and suitability of the founding medium. Any soft or suspect material should be removed and where necessary replaced with low-grade concrete. The glacial till soils are sensitive to moisture variation and should be protected by blinding following excavation.

The presence of extensive boulders should also be noted with possible over-break in excavation occurring.

The majority of boreholes and trial pits were dry with only occasional water seepages recorded. This may indicate isolated water bearing gravelly zones, typical of the heterogeneous nature of the regional Glacial Till.

SETTLEMENT

Settlement of the order of 5 to 10mm can be expected under the foundation loadings indicated above. Settlement should be quite uniform and differential movement is not anticipated.

EXCAVATION

Given the variations in site levels it is likely that significant cut and fill operations will be required. No major issues will arise with excavation, other than the presence of boulder obstructions and possibly water ingress if gravel zones are encountered.

A detailed programme of laboratory testing has been carried out to establish soil parameters relative to the suitability of excavated material for re-use as engineered fill.

The results reflect a high degree of consistency in the boulder clay over the site area and will allow the appointed contractor to design a suitable programme for earthworks on this site.

BRE DIGEST 365 TESTS

The test results reflect very low permeability characteristics in the gravelly CLAY soils. This is very typical of the cohesive material. Clay matrix material is generally unsuited to dispersion of storm or surface water and consideration should be given to the use of the Local Authority Drainage System for this development.

FOUNDATION CONCRETE

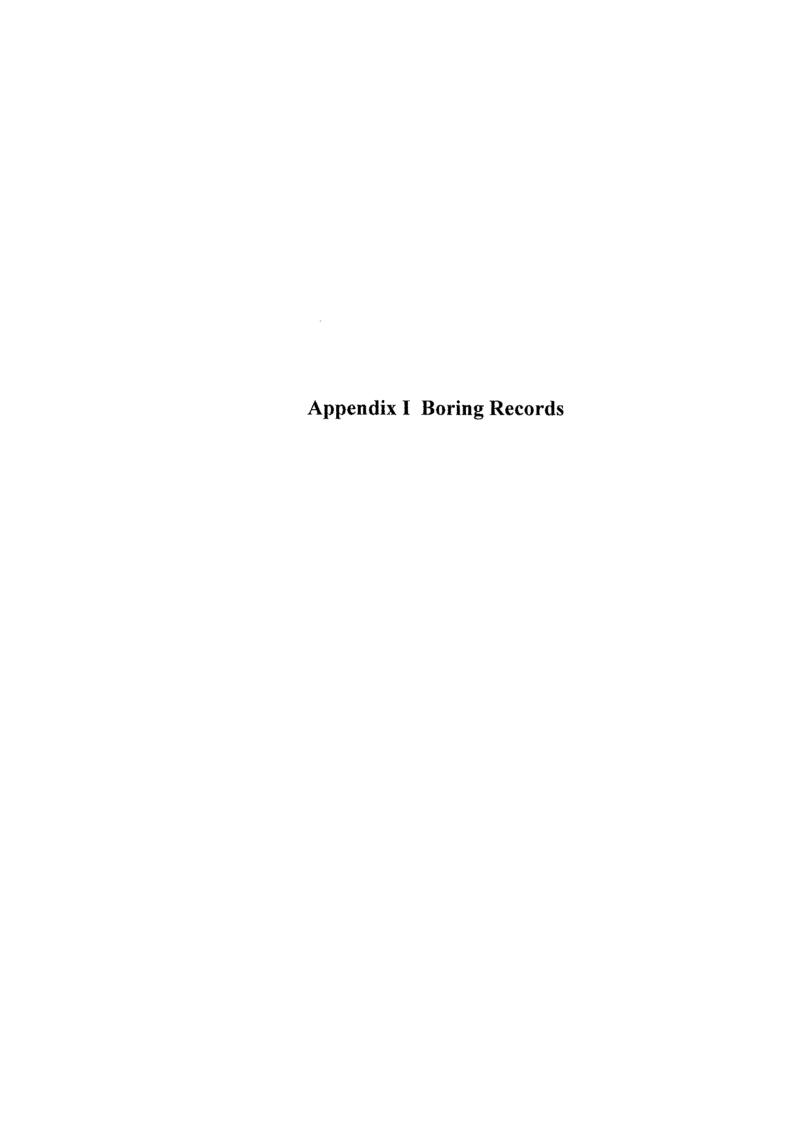
No special precautions are necessary for protection of below ground concrete.

ENVIRONMENTAL

Six samples have been tested to RILTA Suite Parameters and the results confirm an INERT classification for the soils. Excavated material can be safely used on the site or can be disposed of to a suitably licensed Landfill.

A waste Characterisation Assessment (WCA) may be necessary and should be carried out by environmental specialists. This WCA should be submitted to the relevant waste management facility, to confirm suitability for acceptance.

IGSL/JC July 2023





REPORT NUMBER

CONTR	RACT M	onaghan	Active T	ravel -	- Main Sit	е					11	BOREHO SHEET	OLE NO	D. BH01 Sheet 1 of 1	
	DINATES	m AOD)			12.0		YPE Dando 2000 HOLE DIAMETER (mm) 200 HOLE DEPTH (m) 3.00				000	DATE COMMENCED 13/05/2023 DATE COMPLETED 13/05/2023			
CLIENT		onaghan BFL	Co.Co.			SPT HA	MMER REF	NO.				BORED PROCES		P.Allan BY F.C	
		-				LIVEINO	T TOTTO	•,		1		nples	JOED L	1.0	
Depth (m)		C	Description	on			Legend	Elevation	Depth (m)	Ref. Number	Sample Type		Recovery	Field Test Results	Standpipe
0 TC	PSOIL						<u> </u>						1		0,
Fir	m brown sa	andy SILT	T/CLAY	with o	ccasional	gravel	<u>xo</u>		0.30	AA192931	В	0.50			
1 Fir	m grey san	dy SII T/	CLAV				<u> </u>		1.00	AA192932	В	1.00		N = 13	
	own sandy			th occ	asional co	obbles	<u>0</u>		1.20					(2, 3, 2, 3, 4, 4)	
2							8-0-1 0-0-1		2.70	AA192933	В	2.00		N = 17 (2, 2, 2, 4, 5, 6)	
3 Ob	own sandy		-	th som	ne cobble		<u> </u>		3.00					N = 50/75 mm (25, 25, 50)	
5	STRATA BO	DRING		NG											
om (m	2 62 9 2	Time	Comm				Water			Sealed	Rise	e Ti	T	VATER STRIKE DETA Comments	AILS
2.8	3	(h) 1.5	Comm	5,113			Strike 3.00		epth .00	At No	<u>To</u>		nin) 20	Moderate	
											1 =		GF	ROUNDWATER PRO	GRE
	LATION DE						Date		Hole Depth	Casing Depth	Dej	oth to ater	Comme	ents	
Date	Tip De	oth RZT	op RZ	Base	Тур	e	11-05-2		3.00	Nil			nd of BH		
EMAR	KS CAT so	anned lo	ocation a	ind ha	nd dug in	spection	pit was car	ried	I B - Bulk D	le Legeno Disturbed (tub) disturbed Bulk Disturbed fronmental Sam			Samı P - U	Undisturbed 100mm Diameter ple Judisturbed Piston Sample Water Sample	



REPORT NUMBER

CONTRACT		ian ACIIV	re iravel	- Main Site	RIG TYF)E		i.	Derdi 61		BOREH SHEET	OLE NO	D. BH02 Sheet 1 of 1	
CO-ORDINAT		D)			BOREH	OLE DIAME		nm) :	Dando 20 200 4.50	1	DATE C DATE C		NCED 13/05/2023 ETED 14/05/2023	
CLIENT ENGINEER	Monagh DBFL	an Co.C	Co.	1		MMER REF					BORED PROCES		P.Allan	
	DBIL			-3//	ENERG	Y RATIO (%)	09	T	-	nples	33ED E	BY F.C	
Depth (m)		Descr	iption			Legend	Elevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	Recovery	Field Test Results	Standpipe
0 TOPSOI						31/2 31/2 1		0.30				+-		
	wn sandy S				W 48-01.7-01	X0		0.70	AA197801	В	0.50			
Stiff brov	wn sandy S	ILT/CLA	Y with so	me gravel		<u>x</u>			AA197802	В	1.00		N = 6 (1, 0, 1, 1, 2, 2)	
2								2.50	AA197803	В	2.00		N = 26 (2, 3, 6, 8, 5, 7)	
Stiff to ve occasion	ery stiff grey aal cobbles	y sandy	gravelly (CLAY with					AA197804	В	3.00		N = 50/225 mm (4, 5, 9, 15, 26)	
4 Obstruct								4.50	AA197805	В	4.00		N = 50/150 mm (6, 10, 20, 30)	
5														
3														ý
HARD STRA	TA BORING	G/CHISE	LLING									W	VATER STRIKE DET	All S
rom (m) To	Ti	e Cor	mments			Water Strike	Ca	sing S	Sealed At	Rise		ima a	Comments	
0.7 4.3 4.3	.9 1		4110			Julke		pui	O.	10	(1)	11111)	No water strike	
									1			GF	ROUNDWATER PRO	GRES
NSTALLATIO						Date		Hole Depth	Casing Depth	Der W	oth to ater	Comme	ents	
Date Ti	p Depth R	Z Top F	RZ Base	Тур	е									
REMARKS C	AT scanned ut .	d location	on and ha	nd dug ins	spection	pit was cari	ried	B - Bulk D	le Legeno Disturbed (tub) Disturbed e Bulk Disturbed ironmental Sam	d		Sam P - U	Undisturbed 100mm Diarneter ple Indisturbed Piston Sample Water Sample	



REPORT NUMBER

		tive Travel - Mair							BOREHO SHEET	DLE NO.	BH03 Sheet 1 of 1	
CO-ORDINATES GROUND LEVEL				PE OLE DIAME OLE DEPTI		nm) :	Dando 20 200 3.70	-0.00.000	DATE CO		CED 12/05/2023	
	Monaghan Co DBFL	o.Co.	200 20002000	MMER REF Y RATIO (%				- 1	BORED I		P.Allan F.C	
								San	nples			
Depth (m)	Des	cription		Legend	Elevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	Recovery	Field Test Results	Standpipe Details
0 TOPSOIL				34 34 1		0.30						
Soft to firm gravel	brown sandy	SILT/CLAY with o	occasional	-XO		0.00	AA192934	В	0.50			1
1				×			AA192935	В	1.00		N = 7 (1, 2, 1, 2, 2, 2)	
2		QLAV. ::				2.30	AA192936	В	2.00		N = 10 (2, 2, 2, 3, 2, 3)	
cobbles	sandy gravell	y CLAY with occa	asionai				AA192937	В	3,00		N = 50 (6, 6, 10, 10, 20, 10)	
Obstruction				<u>~ 0 °</u>		3.70					N = 50/75 mm	
6 6 7 7 8 8 HARD STRATA	BORING/CHIS	SELLING								WA	TER STRIKE DETA	AILS
rom (m) To (m)	Time (h)	comments		Water Strike		sing S	Sealed At	Ris To		me in) C	omments	
2.7 3.5 2.9 3.7	1 1.5								ν		No water strike	
NOTAL LATION	L L					Hole	Casing	De	nth to		OUNDWATER PRO	GRESS
Date Tip D	PETAILS PEPTAILS	RZ Base	Туре	Date		Depth	Depth	\(\tilde{\ti}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}}	pth to later	Commen	ıts	-
REMARKS CAT out .	scanned loca	i I ition and hand du	g inspection	pit was car	ried	B - Bulk D LB - Large	le Legene Disturbed (tub) Disturbed Bulk Disturber Fronmental Sam	d	+ Vial + Tub)	Sample P - Und	idisturbed 100mm Diameter isturbed Piston Sample ter Sample	



REPORT NUMBER

CONTRACT Monaghan Active Travel - Main CO-ORDINATES GROUND LEVEL (m AOD) CLIENT Monaghan Co.Co. ENGINEER DBFL	RIG TYP BOREHO	OLE DIAME		ım) i	Dando 20 200	100	BOREH SHEET DATE C		Sheet 1 of 1 ICED 12/05/2023	
GROUND LEVEL (m AOD) CLIENT Monaghan Co.Co. ENGINEER DBFL	BOREHO BOREHO SPT HAI	OLE DIAME		ım) i		nn t		OMMEN		
CLIENT Monaghan Co.Co. NGINEER DBFL	SPT HAI	DLE DEPTH			200	- 1				
ENGINEER DBFL	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	MMED DEE	(12)		1.20		DATE C			
	LIVERG					I	BORED PROCES		P.Allan Y F.C	
Ē		I TATIO (%	'		1		nples	SSED B	T F.C	
Description		Legend	Elevation	Depth (m)	Ref. Number	Sample Type		Recovery	Field Test Results	Standpipe Details
O TOPSOIL		<u> </u>		0.30						
Grey SILT/CLAY with some gravel and occa cobbles Obstruction	asional	8 - x C			AA192938	В	0.50		N = 50/75 mm	
Obstruction End of Borehole at 1.20 m				1.20					(25, 50)	
2										
3										
										1
4										
5										
3										
,										
										7
ARD STRATA BORING/CHISELLING									ATER STRIKE DETA	AILS
om (m) To (m) Time (h) Comments		Water Strike			Sealed At	Ris To		ime nin)	Comments	
1.1 1.2 1									No water strike	
								GR	OUNDWATER PRO	GRESS
ISTALLATION DETAILS		Date		Hole Depth	Casing Depth	De W	pth to ater	Comme	nts	
Date Tip Depth RZ Top RZ Base 1	Гуре								31 22	
EMARKS CAT scanned location and hand dug out . Obstruction encountered . Move rebore .	inspection ed to BH04A	pit was carr and attem	ried pted	Samp D - Small B - Bulk D LB - Large	Legeno Disturbed (tub) Disturbed Bulk Disturbed ironmental Sam	d d		Samp P - Ur	Undisturbed 100mm Diameter ble disturbed Piston Sample Vater Sample	



REPORT NUMBER

CONTRACT Monaghan Active Travel - N	Main Site						BOREH	OLE NO	D. BH04A	
CO-ORDINATES	RIG TYP	E		Г	ando 20	000	SHEET		Sheet 1 of 1	
GROUND LEVEL (m AOD)	BOREHO	DLE DIAMET		m) 2	00 .50		DATE C		NCED 13/05/2023 TED 13/05/2023	
CLIENT Monaghan Co.Co. ENGINEER DBFL	SHEET OF THE STREET	MER REF. RATIO (%)					BORED PROCE:		P.Allan SY F.C	
	ENERGY	NATIO (%)					ples	OSED B	F.C	bon-
Description		Legend	Elevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	Recovery	Field Test Results	Standpipe Details
OUT TOPSOIL		44 47 7		0.30						
Stiff bown sandy SILT/CLAY with some	e gravel	X0			AA192939	В	1.00			
Very stiff grey sandy gravelly CLAY with	h some			2.50	AA192940	В	2.00		N = 21 (2, 2, 3, 6, 8, 4)	
cobbles 3					AA192941	В	3.00		N = 50 (4, 4, 5, 10, 20, 15)	
Obstruction	~~			4.50	AA192942	В	4.00		N = 40/150 mm (6, 10, 19, 21) N = 50/75 mm	
End of Borehole at 4.50 m 6 7 8 HARD STRATA BORING/CHISELLING								W	JATER STRIKE DETA	AILS
rom (m) To (m) Time Comments		Water Strike	Cas		ealed At	Rise		ima	Comments	TILO
1.1 1.3 1 4.4 4.5 1.5		Ottike	De	5411	AL				No water strike	
NSTALLATION DETAILS		Date		lole	Casing	Dei	oth to	GR Comme	ROUNDWATER PRO	GRESS
Date Tip Depth RZ Top RZ Base	Туре	Date		epth	Depth	W	ater	Comme	ents	
REMARKS CAT scanned location and hand out .	dug inspection p	pit was carri	ed	B - Bulk Dis LB - Large	e Legeno Disturbed (tub) Sturbed Bulk Disturbed Onmental Sam	d	Vial + Tub)	Samp P - U	Undisturbed 100mm Diarneter ple indisturbed Piston Sample Water Sample	



REPORT NUMBER

			A A A A	Dan Maria Basin	e goteo so war-						- 1				
	NTRA	200	onaghan	Active Travel	- Main Sit		NE					BOREHO SHEET	DLE NO.	BH05 Sheet 1 of 1	
		NATES LEVEL ((m AOD)				OLE DIAMI		nm)	Dando 20 200 4.50		DATE CO		DED 15/05/2023 ED 15/05/2023	
CLII	ENT SINEE		onaghan BFL	Co.Co.		Personal partition	MMER REF Y RATIO (%					BORED E		P.Allan F.C	
											San	ples			
Depth (m)			D	escription			Legend	Elevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	Recovery	Field Test Results	Standpipe Details
0	TOP	SOIL					71/2/1/2		0.30						
1	Soft grav		own sand	ly SILT/CLAY	with occa	sional	X0 x o		0.00	AA192946 AA192947	1857	0.50		N = 6	
							————X		2.00	, 0 1102347		1.00		(2, 6, 1, 1, 2, 2)	
2	occa	sion cob to stiff b	bles	andy gravelly dy gravelly CL			0 0		2.00	AA192948	В	2.00		N = 19 (2, 2, 3, 4, 5, 7)	
3	Stiff occa	to very st sion cob to stiff bi		andy gravelly dy gravelly CL			0 0 0 0 0			AA192949	В	3.00		N = 26 (2, 3, 4, 6, 8, 8)	
4		ruction							4.50	AA192950	В	4.00		N = 50/150 mm (6, 8, 20, 30) N = 50/75 mm	
5 6 8								V							
HAI	RD S	TRATA B		HISELLING										TER STRIKE DET	AILS
rom	(m)	To (m)	Time (h)	Comments			Wate Strike		sing :	Sealed At	Rise To			omments	
3. 4.		3.9 4.5	1 1.5				4.50		.50	No	3.50		1 0	Moderate	
								- 1	Hole	Casina	D-	ath t = 1		OUNDWATER PRO	GRESS
	ALL/ ate	Tip De	everans—ere	op RZ Base	Тур	е	Date		Depth_	Casing Depth	W	oth to ater	Commen	ts	
REM	IARK	S CAT so	canned lo	cation and ha	and dug in	spection	pit was car	rried	B - Bulk I LB - Larg	le Legene Disturbed (lub) Disturbed e Bulk Disturbed erronmental San	d	· Vial + Tub)	Sample P - Und	disturbed 100mm Diameter isturbed Piston Sample ter Sample	



REPORT NUMBER

1000	25/															
CONTR	ACT M	lonagha	an Activ	e Travel	- Main Site	9						BORE		10.	BH06 Sheet 1 of 1	
	DINATES ID LEVEL	(m AOE	O)				E DLE DIAM DLE DEPT		mm)	Dando 2 200 1.00	000		COMME		ED 16/05/2023	
CLIENT		onagha BFL	an Co.C	Co.			MMER REI					BORE!		BY	P.Allan F.C	
=									=		San	nples				d)
Depth (m)			Descr	iption			Legend	Flevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	(111)	recovery	Field Test Results	Standpipe Details
- 0 TO	PSOIL						31 31 3		0.00		-	-	-			
Ve	ry stiff brow	wn sand	dy SILT bles	CLAY w	ith some g	ravel	® -		1.00	AA197914	В	0.80				
	struction d of Boreh	ole at 1	1.00 m												N = 50/75 mm (25, 50)	
- 6 - 7																
9																
HARD	STRATA B			LLING										WA	TER STRIKE DETA	AILS
From (m)	8 (8)	Time (h)	e Cor	mments			Wate Strike		asing epth	Sealed At	Ris To		Time (min)	Co	omments	
0.9	1	1												N	lo water strike	
													G	RO	UNDWATER PRO	GRES
Security of many filterior	LATION DI						Date	е	Hole Depth	Casing Depth	De V	pth to later	Comn	nent	s	
Date	Tip De	pth RZ	Z Top F	RZ Base	Тур	е	-									
REMARI	KS CAT s out . C rebore)bstruct	d location tion end	on and ha countered	and dug ins I . Moved t	spection to BH06A	pit was ca and atten	rried npted	B - Bulk	ple Leger Il Disturbed (tub Disturbed ge Bulk Disturbed svironmental Sa	ed	+ Vial + Tuh	Sa	mple - Undis	disturbed 100mm Diameter sturbed Piston Sample er Sample	



REPORT NUMBER

U	ರತ	<u> </u>			-					•					24665	
co	NTRA	СТ М	onag	ghan Ac	tive Travel	- Main Site							BOREH	OLE N		
CO	-ORDI	NATES				F	RIG TYP	·F			Dando 20	ากก 🖯	SHEET		Sheet 1 of 1	
		LEVEL	(m A	OD)		E	BOREH	OLE DIAME		nm)	1.00		DATE C			
	ENT			ghan Co	.Co.			VMER REF					BORED	BY	P.Allan	
ENG	GINEE		BFL.		····	E	NERG	RATIO (%	6)	· · · · · · · · · · · · · · · · · · ·	·		PROCE	SSED E		T
Ê									Ē	Έ			nples			9
Depth (m)				Des	cription			Legend	Elevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	Recovery	Field Test Results	Standpipe Details
									<u> </u>	a	άź	s _	దిక్	ě		S a
- 0		SOIL	wn s	andy SII	T/CLAY wi	th some gra	avel	<u>~</u>		0.30	4					
	and	occasion	al co	obbles		in some gre	1461	®			AA171709	8	0.80			
1		truction						6-6		1.00	171708		0.80		N = 50/75 mm (25, 50)	
	End	of Boreh	ole a	at 1.00 n	n					ļ						
_																
2																

3															7	
															7	
<u> </u>																
4																
5																
6																
7																
8																
9																
9																
HA	RD S	TRATA B	ORI	NG/CHIS	BELLING					<u> </u>	<u> </u>	L	I		 WATER STRIKE DET.	 AILS
ron	n (m)	To (m)		ime (h)	omments			Wate Strike		ising S epth	Sealed At	Ris To		ime nin)	Comments	
0	.9	1	1	1											No water strike	
														Gl	ROUNDWATER PRO	GRESS
INS	TALL	ATION DI						Date	•	Hole Depth	Casing Depth	De V	pth to /ater	Comm		
[Date	Tip D∈	pth	RZ Top	RZ Base	Туре	!									
REN	/ARK	S CAT s	canı	ned loca	tion and ha	ind dug insp	ection	pit was car	ried	Samo	e Legen	d				
	v	out.			unsu ila	a aug mis	-00001	p. 1140 001		D - Small 8 - Bulk D	le Legen Disturbed (tub sturbed Bulk Disturbe) ed		San	- Undisturbed 100mm Diameter nple Undisturbed Piston Sample	
										Env - Env	ronmental Sa	nple (Jar -	+ Vial + Tub)	w	Water Sample	



REPORT NUMBER

1																		
COI	NTRAC	T Mo	naghan .	Active	e Travel	- Main S	Site							BOREH SHEET	OLE	NO.	BH07 Sheet 1 of 1	
	-ORDIN OUND I	ATES LEVEL (r	n AOD)					PE IOLE DIAN IOLE DEPT			m) 2	Dando 20 200 1.00	000	DATE C			ED 16/05/2023	
	ENT SINEER		naghan	Co.C	0.			MMER RE		0.				BORED PROCE		BY	P.Allan F.C	
	JIII						LIVEINO	1	1					nples	OOLD		7.0	
Depth (m)			D	escri	ption			Legend		Elevation	Depth (m)	Ref. Number	Sample Type			Recovery	Field Test Results	Standpipe Details
0	and o	stiff brow ccasiona uction	n sandy	s	CLAY w	ith some	e gravel	₩			1.00	AA171710	В	0.80		-	N = 50/75 mm (25, 50)	
3 6 6 7	End o	f Boreho	ele at 1.00	0 m														
			DRING/CI Time	HISE	LING			Wate	or I	Cas	ing G	Sealed	Ris	. T	ime		TER STRIKE DETA	AILS
From 0.	23 23	Го (m) 1	(h) 1	Con	nments			Strik		De		At	To		min)		omments No water strike	
								-								GPO	UNDWATER PRO	GRESS
INS.	TALI A	TION DE	TAILS					Da	te		Hole	Casing	De	pth to ater	Com			UKESS
	Date		oth RZ T	op F	RZ Base	Т	уре			<u> </u>	Depth	Depth	W	/ater	50111	en		
REN	MARKS	CAT so out .	anned lo) ocatio	n and ha	and dug	inspection	l pit was ca	arried	d d	Samp D - Small B - Bulk D LB - Larg Env - Env	DIE Legen Disturbed (tub Disturbed e Bulk Disturbe rironmental Sar	nd ed mple (Jar	+ Vial + Tub)	5	Sample - Undis	disturbed 100mm Diameter sturbed Piston Sample er Sample	



REPORT NUMBER

CO	NTRAC	CT M	onaohan	Active	Travel.	- Main Sit	te.						BOREHO	OLE N	0.	BH08	
		NATES	- i agrian	,	- Huver	mani Oli	RIG TYP	oF			Dando 20	_	SHEET		.35	Sheet 1 of 1	
		LEVEL (m AOD)				BOREH	OLE DIAMI		nm)	200 3.40		DATE C			14/05/2023 14/05/2023	
	ENT		onaghan BFL	Co.Co	٥.		ALC: CIVEDNIC	MMER REF					BORED		DV	P.Allan	
	SINEER	C DE	orL				ENERG	Y RATIO (9	%) 				PROCES	וטשפט	ы	F.C	
Depth (m)			C	Descrip	otion			Legend	Elevation	Depth (m)	Ref. Number	Sample Type	Depth (m)	Recovery	(laconal)	Field Test Results	Standpipe Details
- 0	TOPS							71 71V. 7		0.30							
	Firm	brown sa	andy SIL	T/CLA	Y with o	ccasional	gravel				AA192945	В	0.50				
1								×		1.80	AA192946	В	1.00			N = 12 (1, 2, 2, 2, 3, 5)	
2	Stiff to	o very st sional co	iff grey s bbles	andy g	gravelly (CLAY with	n			1.00	AA192947	В	2.00			N = 29 (2, 3, 3, 10, 10, 6)	
3 - 3	Obate	uction						2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		3.40						N = 50/150 mm (10, 15, 25, 25) N = 50/75 mm (34, 25, 50)	
5 6 8	RD ST	RATA B	ORING/O	CHISEI	LING										WATE	R STRIKE DET	All S
-		To (m)	Time (h)		nments			Wate Strike		sing	Sealed At	Rise		ime nin)		ments	,
2	.6	2.8 3.4	0.75 1.5					Sunk		spul	Υſ	То	(r)	No	water strike	
										Hole	Casing	I Do	nth to T			NDWATER PRO	GRESS
	TALLA Date	TION DE	pth RZ	Top F	RZ Base	Ту	pe	Dat	e	Depth	Depth	Jew M	pth to later	Comm	nents	***************************************	
INS	MARK	S CAT so	canned I	locatio	n and ha	and dug ir	nspection	pit was ca	rried	LB - Larg	DIE Legen I Disturbed (tub Disturbed ge Bulk Disturbe vironmental Sa	ed	+ Vial + Tub)	Sa P -	mple	urbed 100mm Diameter bed Piston Sample ample	

Appendix II Rotary Core Logs Photographs



REPORT NUMBER

***) ගි		4			GEOT	ECF	INIC	JAL CO	RE LO	3 RECC	ORD				4	2466	35
CC	ONTR	ACT	. 1	/lona	ghan Act	ive Travel	- Mair	n Site						RILLHOL	E NO	RC	02 et 1 of	2
			TES EVEL	(mC	וטו				RIG TYPE	i		Beretta T	DA	TE DRIL		25/0	05/202 05/202	3
CL	IENT	•	N		ghan Co.	Co.			FLUSH		,	Air/Mist -90	DF	RILLED E	Y	10	3SL	JK
	Т	EK	T		Ì				CORE DIA	AMETER (m	ım)	78	ILC	GGED E	IY 	T).O'She	ea T
Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	Spa L (n	cture acing .og nm) 50 50d	Non-intact Zone	Legend			Descrip				Depth (m)	Elevation	Standpipe Details	SPT (N Value)
1	1.50	0	0	0		er ing de			SYMMET returns of	RIX DRILLI Fsoft CLAY.	NG: No rec	covery, obse	rved by d	riller as				
2	3.00	0	0	0											ANNUAL TO THE TAXABLE			- Constitution of the Cons
~ 4	4.50	0	0	0	· · · · · · · · · · · · · · · · · · ·	144-11111111111111111111111111111111111									4.50			N = 24 (3, 2, 4, 7, 7, 6)
5	6.00	0	0	O				- 0 - 0 - 0	SYMMET returns of	RIX DRILLI gravelly CL	NG: No rec AY with occ	overy, obse casional cob	rved by d bles	riller as				
7	7.50	0	0	0	7-7-7-8-8-8-8-8											1		N = 43 (4, 6, 10, 10, 10, 13)
8	9.00	0	0	0	Application of the state of the	4		76							- Transmire	· interest		N = 51 (17, 9, 11, 13, 13, 14)
REI Hole	VAR	0	0	0				<u> </u>										
Hole			om 0.	.00-1	0.50m					Water Strike	Casing Depth	Sealed	Rise	Time		mment		DETAILS
										Julke	Берип	At	То	(min)	-			recorded
INS	TALI	.ATI	ON D	ETA	ILS				······································	Date	Hole	Casing	Depth Wate	to Con	GR0 nments		VATE	RDETAILS
	Date		Tip De	pth	RZ Top	RZ Base		Тур			Depth	Depth	Wate	r CON	n nen itt			
25-	05-2	3	10.5	U	1.00	10.50	5	0mm	SP				İ					



GEOTECHNICAL CORF LOG RECORD

REPORT NUMBER

	ලිපි					GEOI	EC.	11/11/	JAL CO	KE LOG	KECU	KU				2	2466	35
	NTR			1ona	ghan Act	ive Travel	- Mai	n Site	T				DRILL SHEE	HOLE I	NO	RC She	02 et 2 of	2
	-ORE		VEL	(mC	DD)				RIG TYPE	E		Beretta T4 Air/Mist	ıa	DRILLE LOGGE			5/2023 5/2023	
	ENT GINE			fona OR/	ghan Co.	Co.		r	INCLINAT	ION (deg) AMETER (m	rn)	-90 78		ED BY			SL - J .O'She	
ವ Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	(1	octure acing .og nm) 50 500	Non-intact Zone	Legend			Descript				Depth (m)	Elevation	Standpipe Details	SPT (N Value)
11	10.50								returns of	RIX DRILLII gravelly CL of Borehole	AY with occ	asional cobl	ved by drille bles <i>(contini</i>		0.50			N = 48 (6, 8, 11, 12, 12, 13)
13	T T TOTAL PROPERTY OF THE PROP				· Proposition de la company									P. P. L. S. C.				
15	***************************************				Organistic state of													
16 17					TO THE PROPERTY OF THE PROPERT	The state of the s								The second section is the second seco				
18 REM Hole														Mary Line (A) a comment			THE PROPERTY AND ADDRESS OF THE PROPERTY ADDRESS OF THE PROPER	
REN	IARH	(S								<u> </u>	····				WAT	ER ST	RIKE	DETAILS
łoie	case	ed fro	om 0.0	00-1	0.50m					Water Strike	Casing Depth	Sealed At		Time (min)	Con	nment	S	recorded
NST		ΔΤΙ	ON DI	FTA	II S					Date	Hole	Casing	Depth to	1		UNDW	/ATER	DETAILS
	Date 05-23	Ţ	ip De 10.50	pth		RZ Base 10.50		Typ 50mm		Date	Depth	Depth	Depth to Water	Comm	ien(S		*****	



IGSL RC FI 10M 24665 - MAIN SITE.GPJ IGSL.GDT 6/8/23

GEOTECHNICAL CORE LOG RECORD

V)ලිදු	رايَّ				GLO I	LUI	HAIC	JAL CO	KE LOG	RECO	עאי				2	2466	S5
CC	NTR	ACT	٨	1ona(ghan Acti	ve Travel	- Mai	n Site						ILLHOLE EET	NO	RC	03 et 1 of	2
		DINA		(mO	D)				RIG TYPE FLUSH			Beretta T	DA	TE DRILL TE LOGG		26/0	95/2023 95/2023	3
	IENT GINE			fonaç ORA	han Co.	Co.			INCLINAT	ION (deg) METER (mi	m)	-90 78	1	ILLED B			SSL - J .O'She	
Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	Spa L (n	cture acing og am) 0 500	Non-intact Zone	Legend			Descrip	ition			Depth (m)	Elevation	Standpipe Details	SPT (N Value)
0 1 2 3 4 5 5 6 7 8 8 PE	8.10 9.10	100 71 KS ed fro	0 0	0					Returns of gravelly Ci	stiff to very LAY, with oc to subround estone.	stiff, dark	brown, sligi	ntly sandy,	Gravel	8.10 WAT Co.	TER ST	RIKE S	N = 57 (9, 13, 17, 11, 15, 14) N = 50 (5, 11, 10, 17, 11, 12) N = 47 (4, 7, 9, 9, 14, 15) N = 53 (4, 11, 11, 13, 13, 16) N = 50 (8, 11, 12, 13, 10, 15)
INS	TALI	LATIO	ם מכ	ETAI	LS	***************************************				Date	Hole	Casing	Depth t	o Com	GRO		VATER	DETAILS
	Date					RZ Base		Тур	e		Depth	Depth	vvater	. 50/11	nc			
		- 1					I			i	1	1	1	1				



GEOTECHNICAL CORE LOG RECORD

Λĺ	ලියි	<u>.</u>							JAL CO		J INEGO					2	2466	55
CC	NTR	ACT	. V	/lona	ghan Acti	ive Travel	- Mai	n Site					DRI SHE	LLHOLE	NO	RC:	03 et 2 of :	?
GR		D LI	TES EVEL	(mC	DD)				RIG TYPE			Beretta T4 Air/Mist	DAT	E DRILLE		26/0	5/2023 5/2023	J
	ENT GINE			Mona CORA	ghan Co.	Co.			INCLINAT	ION (deg) METER (m	ım)	-90 78		LLED BY GED BY			SSL - Ji .O'She	
⇒ Downhole Depth (m)	L	T.C.R.%	S.C.R.%	R.Q.D.%	(n	cture acing og nm)	Non-intact Zone	Legend			Descript	tion			Depth (π)	Elevation	Standpipe Details	SPT (N Value)
:	10.50							- 0	<u> </u>	of Borehole	at 10.50 m				10.50			
12				***************************************				THE PROPERTY OF THE PROPERTY O						3				
14			- Anna															
15																		
17						i deline e de											7,7,7	
REN Hole					The state of the s											THE PROPERTY OF THE PROPERTY O		
												rawe a second	*****					
Hole	ARM case		om 0	3-00.	3.00m					Water Strike	Casing Depth	Sealed At	Rise To	Time (min)		ER ST		DETAILS
															No	water	strike	recorded
INS.	TALL	.ATI	ON D	ETA	ILS					Date	Hole	Casing	Depth to Water	Oppr	GRO nents	WONU	VATER	DETAILS
	Date				RZ Top	RZ Base		Туг	be	29-05-23	10.50	Depth 8.00	10.40		levels re	ecorded	5 mins a	ifter end of



GEOTECHNICAL CORE LOG RECORD

REPORT NUMBER

24665

1110	၁၁.	ار دا														_	. 100	,,
CON	TRA	CT	N	Iona	ghan Acti	ve Travel	- Mai	n Site					1	ILLHOL	E NO	RC		
CO-C	ORD	INA:	TES				•		DIO TARR				DA	EET TE DRIL	LED		et 1 of 05/022:	
GRO		LE							RIG TYPE FLUSH	i		Beretta Ta Air/Mist	DA	TE LOG	3ED	23/0	5/202	3
CLIE: ENGI		R		tona ORA	ghan Co.0	Co.			INCLINAT	iON (deg) VMETER (m	3171)	-90 78	•	ILLED E GGED E			SSL - J .O'She	
Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	Spa L (m	cture icing og im)	Non-intact Zone	Legend			Descrip	tion			Depth (m)	Elevation	Standpipe Details	SPT (N Value)
ŏ (<u>ა</u>				0 25	0 500	ž	٤	CVMMET	יו אם איי	NO. No.			he	å	ᆲ	ţ	Q.
1	.50	0	0	0					returns of	CLAY.	ING: No rec				1.50			
2 2.	.60	73	0	0				0 0	gravelly C is angular are of lime	LAY, with one to subrount estone.	y stiff, dark occasional co ded fine to d	obbles. San- coarse of lin	d is fine. (nestone. (Gravel Cobbles	2.60			
3	.00	0	0	0	***************************************			0 0	returns of	gravelly CL	NG: No rec AY with occ	overy, obsei casional cob	ved by dr bles	aller as				N = 57 (13, 12, 27, 11, 9, 10)
4.	50	100	0	0	Automotive and a second													N = 55 (6, 17, 18, 11 12, 14)
6	00	0	0	0	Regularisas managaman da	and the development of the second		0										N = 44 (5, 7, 10, 11, 10, 13)
8.8.99 100 REMA	50	0	0	0				0										N = 45 (8, 9, 8, 14, 13, 11)
9 10	.00	0	0	0														N = 10/75 mm (7, 14, 10)
REM/	٩RK			00.4	E 00-	<u> </u>				Wester	Casina	Code	Dia-	T		ER ST	rike.	DETAILS
Hole (case	ed tre	om U.	.00-1	5.00m					Water Strike	Casing Depth	Sealed At	Rise To	Time (min)		mment	s	***************************************
														-	N	o wate	r strike	recorded
															GBC	אכוואן זכ	VATE	DETAILS
NST/	ALL	ATI	ON D	ETA	ILS	•		••••	•	Date	Hole Depth	Casing Depth	Depth Wate	to Cor	nments			· ····································
Da	ate	7	lip De	epth	RZ Top	RZ Base		Tyr	e									



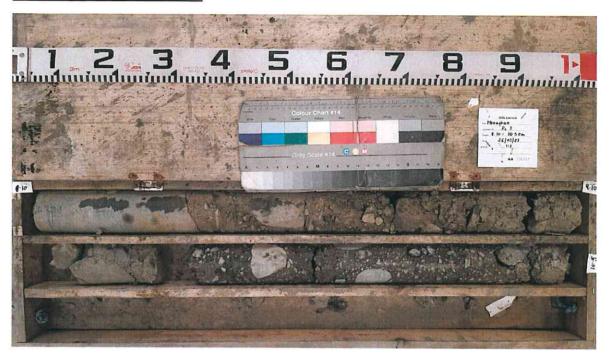
GEOTECHNICAL CORE LOG RECORD

REPORT NUMBER

24665

1	9 3	<u>.</u>														Æ.	:400	
COI	VTR.	ACT	٨	1ona	ghan Acti	ve Travel	- Mai	n Site				·······	DRI	LLHOLI	E NO	RC	06	
CO-	ORI	ANIC	TES			<u></u>							SHE				et 2 of	
GRO	DUN	D LE	VEL	(mC	D)				RIG TYPE			Beretta T	ea I	E DRILI E LOGO			5/0223 5/2023	
CLI	ENT			lona ORA	ghan Co.(Co.			INCLINAT	ION (deg) METER (m	m)	-90 78	1	LLED B			SSL - J .O'She	
Downhole Depth (m)	Core Run Depth (m)	T.C.R.%	S.C.R.%	R.Q.D.%	Spa Li (m	cture icing og im) o sod	Non-intact Zone	Legend			Descrip				Depth (m)	Elevation	Standpipe Details	SPT (N Value)
10	11.50	0	0	0				- O	SYMMET returns of	RIX DRILLI gravelly Cl.	NG: No rec AY with occ	overy, obse casional cob	rved by dri Ibles <i>(cont</i>	ller as inued)				N = 46 (4, 11, 12, 9, 11, 14)
12	3.00	0	0	0		ş		- 0 - 0										N = 55 (15, 16, 19, 11, 12, 13)
13	3.50	0	0	0	-										13.50			
13		100	0	0				6.16.19	gravelly C	f stiff to ven LAY, with o to subround estone.	ccasional co	obbles. San	d is fine. C	ravel				N = 50/32 mm (25, 50)
- 15	5.00							<u></u>	Ead	of Borehole	at 15 00 m				15.00			
17 18	TO THE PARTY OF TH														Marganian and Andrews and Andr			
REN			om n	00-1	15.00m					Water	Casing	Sealed	Rise	Time				DETAILS
REN Hole	: UdS	eu If	он O	.00-	IJUU.CI					Strike	Depth	At	To	(min)	N		r strike	recorded
INST	[ALI	ATI	ON D	ETA	ILS					Date	Hole	Casing	Depth to	O Con	nments		ᄣ	COETAILS
)ate		Tip D	epth	RZ Top	RZ Base		Туг	ЭЕ	23-05-23	Depth 15.00	15.00	13.40			recorded	l 5 mins	after end of

RC03 - Box 1 of 1 - 8.10-10.50m



RC06 - Box 1 of 1 - 1.50-15.00m



Appendix III Trial Pit Records Photographs



CONTR										24	000	
	RACT	Monaghan Active Travel						TRIAL P	IT NO.	TP0		
LOGGI	ED BY	I.Reder	CO-ORDINAT		833,8	06.85 E 17.07 N		DATE ST		28/0-	et 1 of 1 4/2023 4/2023	
CLIENT		Monaghan Co.Co. DBFL/Cora	GROUND LE	VEL (m)	72.34			EXCAVA METHO		3T T macl	racked nine	
									Sample	5	² a)	meter
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0 -	TOPSOI	L		1 11 1 11 11								
	coarse, c	eyish brown to brown, slightly sand th medium cobbles content. Sand i gravel is fine to coarse subangular ded, cobbles are small to coarse si ded	to	0 0 0 0	0.25	72.09		AA200193	В	0.70		
1.0 (f	CLAY with	tiff, grey, slightly sandy gravelly slig th high cobbles and boulders conte parse, gravel is fine to coarse suba ded, cobbles and boulders are sub	ent. Sand is noular to		0.90	71.44						
i	TP termi	nated at 1.8m due to many boulder rial Pit at 1.80m	rs	0-7	1.80	70.54		AA200194	В	1.60		
 -4.0												
Ground TP dry	dwater C	onditions						 		.1		,
Che Late		numeros and a second										
Stability TP stab	y ole											
Genera TP don	Il Remar ne for civi	ks ic offices project									and the second s	



J	53L	Т	RIAL PIT	RECO	RD					24	665	
CON	ITRACT	Monaghan Active Travel						TRIAL P	IT NO.	TP()2 et 1 of 1	
LOG	GED BY	l.Reder	CO-ORDINAT		833,7	17.94 E 82.52 N		DATE S		27/0	4/2023 4/2023	
CLIE	INEER	Monaghan Co.Co. DBFL/Cora	GROUND LE	/EL (m)	69.34			EXCAVA METHOI		3T T mac	racked hine	
									Sample	S	'a)	neter
	***************************************	Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0	TOPSOI		aralla Of AV	7 W V	0.25	69.09						
<u> </u> - -	with low fine to co	rm, brown, slightly sandy slightly gra cobbles content. Sand is fine to coa parse subangular to subrounded, co bangular to subrounded.	rse, gravel is		0.55	68.79						
1.0	Dense, t	original to subrounded. brownish grey, very clayey very sand ubrounded to subangular GRAVEL, ilar to angular cobbles and boulders	y fine to with high content.	00000000000000000000000000000000000000				AA200181	В	0.80		
-	TP termi End of T	nated at 1.4m due to many boulders rial Pit at 1.40m	;	90°0	1.40	67.94						
2.0												
3.0												
4.0				4								
-												
TP d	ry	onditions										
Stabi TP st	ility ightly unst	able from 0.55m										
Gene TP de	eral Remar one for civ	ks ic offices project									·	

IGSL TP LOG 24665.GPJ IGSL.GDT 10/5/23



ಕರ್ಷ/				עאי				***************************************	24	665	
TRACT	Monaghan Active Travel							T NO.			
GED BY	I.Reder			833,7	66.18 N		DATE ST		27/0	4/2023	
NT NEER	Monaghan Co.Co. DBFL/Cora	GROUND LE	VEL (m)	72.15							
								Sample	5	oa)	meter
	Geotechnical Descrip	otion	Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KF	Hand Penetrometer (KPa)
Soft, bro content. subangu	own, sandy slighlty gravelly CL Sand is fine to coarse, gravel Jlar to subrounded, cobbles a	is fine to coarse		0.20	71.95						
silty CLA is fine to subroun	AY with high cobbles and boul coarse, gravel is fine to coars ded, cobbles and boulders are	ders content. Sand	5 80 80 80 8 8 8 8 8	0.80	71.35		AA200179	B	0.60		
TP termi End of T	inated at 1.7m due to many bo rial Pit at 1.70m	oulders	XO S	1.70	70.45		AA200180	В	1.50		
			The control of the co								
	Conditions		<u> </u>			<u> </u>					
•											
lity able							V-8-4-4				

	TOPSO Soft, brocontent, subangular, subangular, angular. TP term End of T	TRACT Monaghan Active Travel GED BY I.Reder NT Monaghan Co.Co. NEER DBFL/Cora Geotechnical Descrip TOPSOIL Soft, brown, sandy slightly gravelly CL content. Sand is fine to coarse, grave subangular to subrounded, cobbles a to subrounded. Firm to stiff, greyish brown, slightly sa silty CLAY with high cobbles and bould is fine to coarse, gravel is fine to coar subrounded, cobbles and boulders ar angular. TP terminated at 1.7m due to many be End of Trial Pit at 1.70m	TRACT Monaghan Active Travel GED BY I.Reder NT Monaghan Co.Co. NEER DBFL/Cora Geotechnical Description TOPSOIL Soft, brown, sandy slighlty gravelly CLAY with low cobbles content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles are small subangular to subrounded. Firm to stiff, greyish brown, slightly sandy gravelly slightly silty CLAY with high cobbles and boulders content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles and boulders are subangular to angular. TP terminated at 1.7m due to many boulders End of Trial Pit at 1.70m	TRACT Monaghan Active Travel GED BY I.Reder NT Monaghan Co.Co. NEER DBFL/Cora Geotechnical Description Geotechnical Description TOPSOIL Soft, brown, sandy slightly gravelly CLAY with low cobbles content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded. Firm to stiff, greyish brown, slightly sandy gravelly slightly slift CLAY with high cobbles and boulders content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded. The terminated at 1.7m due to many boulders are subangular to angular. The terminated at 1.7m due to many boulders are subangular to family and the subrounded. The terminated at 1.7m due to many boulders are subangular to angular. The terminated at 1.7m due to many boulders are subangular to subrounded.	TRACT Monaghan Active Travel GED BY I.Reder GROUND LEVEL (m) 72.15 TOPSOIL Soft, brown, sandy slightly gravelly CLAY with low cobbles content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles and boulders content. Sand is fine to coarse, gravel is fine to coarse, gravel is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles and boulders content. Sand is fine to coarse, gravel is fine to coarse subangular to angular. TP terminated at 1.7m due to many boulders End of Trial Pit at 1.70m Individual to many boulders content. Sand is fine to coarse, gravel is fine to coarse. gravel is fine to coarse subangular to angular.	TRACT Monaghan Active Travel GED BY I.Reder S33,766.18 N TOPSOIL Geotechnical Description Firm to stiff, greyish brown, slightly gravelly CIAY with low cobbles content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles are small subangular to subrounded, cobbles are subangular to subrounded, cobbles and boulders content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles and boulders are subangular to angular. TP terminated at 1.7m due to many boulders TP terminated at 1.7m due to many boulders End of Trial Pit at 1.70m The terminated at 1.7m due to many boulders TP terminated at 1.7m due to many boulders	GED BY I.Reder Geotechnical Description TOPSOIL Soft, brown, sandy slightly gravelly CLAY with low cobbles content. Sand is fine to coarse subangular to subrounded. Firm to stiff, greyish brown, slightly sandy gravelly slightly slib CLAY with high cobbles are small subangular to subrounded, cobbles are subangular to subrounded, cobbles and boulders content. Sand is fine to coarse, gravel is fine to co	TRACT Monaghan Active Travel GED BY I.Reder TO-ORDINATES 667,451.08 E 833,766.18 N BATES OR STAND TRACE OR ST	TRIACT Monaghan Active Travel GED BY LiReder THAL PIT NO. SHEET GED BY LiReder THAL PIT NO. SHEET DATE STARTED DATE COMPLET EXCAVATION METHOD TOPSOIL Soft brown, sandy slightly gravelly CLAY with low cobbler content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles and boulders are subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse, gravel is fine to coarse. The coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse subangular to approach to coarse, gravel is fine to coarse gravel is fine to coarse. The coarse gravel is fine to coarse gravel is fine to coarse gravel is fine to coarse. The coarse gravel is fine to coarse gravel is fine to coarse gravel is fine to coarse. The coarse gravel is fine to coarse gravel is fine to coarse. The coarse gravel is fine to coarse gravel is fine to coarse. The co	TRACT Monaghan Active Travel GED BY LReder CO-ORDINATES 667.451.08 E 833.766.18 N GROUND LEVEL (m) 72.15 TO BET DEPL'Cara Geotechnical Description Geotechnical Description Geotechnical Description Geotechnical Description Geotechnical Description Geotechnical Description TOPSOIL Soft, brown, sandy slightly gravely CLAY with low obbiles content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbiles are subangular to subrounded, cobbiles are boulders content. Sand is fine to coarse subangular to subrounded, cobbiles and boulders are subangular to subrounded. TIP terminated at 1.7m due to many boulders TOPSOIL A2200179 B 0.60 A2200179 B 0.60 A2200180 B 1.50 TOPSOIL A2200180 B 1.50	TRIAL PI NO. TP03 SheET Shout I of 1 Secretary I Reder CO-ORDINATES 657-451.08 E 833,766.19 N ORDINATE Monaghan Co Co. NEER DBFL/Cora Geolechnical Description Geolechnical



J.	ار باندن	•	TRIAL PIT I	RECO	RD					24	665	
CON	TRACT	Monaghan Active Travel						TRIAL P	IT NO.	TPO)4 et 1 of 1	
LOG	GED BY	l.Reder	CO-ORDINAT		833,7	81.57 E 81.44 N		DATE S'		28/0	4/2023 4/2023	
CLIE ENGI	NT NEER	Monaghan Co.Co. DBFL/Cora	GROUND LEV	/EL (m)	73.74			EXCAVA METHO		3T T mac	racked hine	
									Sample	s	a)	neter
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Type	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
1.0	cobbles gravel is cobbles (POSSII Firm to s CLAY w	own, slightly sandy very gravelly CL and boulders content. Sand is fine in fine to coarse subangular to subro and boulders are subangular to an BLE FILL) stiff, greyish brown, sandy gravelly s ith high cobbles and boulders conte oarse, gravel is fine to coarse suba ded, cobbles and boulders are sub	to coarse, nunded, gular. slightly silty ent. Sand is		0.10	73.64		AA200184	В	0.50		
2.0	TP termi	inated at 1.8m due to many boulde rial Pit at 1.80m	rs		1.80	71,94		AA200185	В	1.30		
3.0												
~4.0												
Grou	ndwater (Conditions										
TP dr												
Stabi TP st	lity able						***************************************					
	ral Remar	rks vic offices project							·····			
11" U(one for CIV	ne omess project										



10			TRIAL PIT	RECO	RD				,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	24	665	
CON	TRACT	Monaghan Active Travel		······································				TRIAL P	IT NO.	TP()5 et 1 of 1	
LOG	GED BY	I.Reder	CO-ORDINA		833,7	07.95 E 82.70 N		DATE S		28/0	4/2023 4/2023	
CLIE	NT NEER	Monaghan Co.Co. DBFL/Cora	GROUND LE	VEL (m)	69.54	,		EXCAVA METHOI		3T T mac	racked hine	
									Sample	s	a)	neter
		Geotechnical Descriptio	n	Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0	TOPSO Soft, bro	own, slightly sandy slighlty gravel	ly CLAY. Sand is	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	0.20	69.34						
	fine to consult subroun Firm to some silty CLA Sand is	oarse, gravel is fine to coarse suided. stiff, brownish grey, slightly sand AY with high cobbles and low bot fine to coarse, gravel is fine to co	bangular to / gravelly slightly ilders content. parse subangular		0.50	69.04		AA200182	: В	0.70		
1.0	to subro subroun	unded, cobbles and boulders are ded.	e subangular to	X 8 X 4 X 5 X 5						4.70		
2.0	TP termi End of T	inated at 2.1m due to many bould Trial Pit at 2.10m	ders	X A	2.10	67.44		AA200183	В	1.70		
3.0												
4.0												
Grou		Conditions									<u> </u>	;
., uí	,											
Stabi TP st	lity able											
Gene	ral Remar	rks ric offices project		· · · · · · · · · · · · · · · · · · ·								

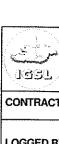


TRIAL PIT RECORD

REPORT NUMBER

24665

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CON	TRACT	Monaghan Active Travel						TRIAL P	IT NO.	TP(06 et 1 of 1	
LOG	GED BY	I.Reder	CO-ORDINA		833,8	74.33 E 10.79 N		DATE S		28/0	14/2023 14/2023	
CLIE	NT NEER	Monaghan Co.Co. DBFL/Cora	GROUND LE	VEL (m)	74.34	4		EXCAV/ METHOI	ATION D		racked hine	
		Geotechnical Descript	tion		The second secon	To a control of the day of the da	Ke Ke		Sample	G	(KPa)	etrometer
				Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0	cobbles coarse s	il. bwn, slightly sandy slightly grave content. Sand is fine to coarse subangular to subrounded, cobular to subrounded.	e, gravel is fine to	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.25	74.09						
1.0	Firm to s silty CLA	stiff, greyish brown, slightly san AY with high cobbles and bould coarse, gravel is fine to coars ded, cobbles and boulders are	ders content. Sand e subangular to		0.80	73.54	(Seepage)	AA200186	В	0.70		1 1111111111111111111111111111111111111
	TP termi End of T	nated at 1.5m due to many bo rial Pit at 1.50m	ulders	2 2-3	1.50	72.84		AA200187	В	1.40		
2.0				The state of the s								
3.0								THE PARTY OF THE P				
4.0												
	ndwater C age flow a	conditions at 1.0m								<u> </u>	L	
Stabil P sta												
	al Remar ne for civ	ks ic offices project		<u> </u>	****							
					····							



TRIAL PIT RECORD

REPORT NUMBER

/ 1/3	33L								***************************************	24	665	
CON	TRACT	Monaghan Active Travel	CO-ORDINAT	TES	667.4	58.22 E		TRIAL PI			et 1 of 1	
LOG	GED BY	I.Reder			833,8	33.25 N		DATE ST			4/2023 4/2023	
CLIE ENGI	NT NEER	Monaghan Co.Co. DBFL/Cora	GROUND LE	VEL (m)	75.79			EXCAVA METHOD		3T T mac	racked hine	
									Samples		(a)	neter
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0	TOPSOI	L wn, slightly sandy slighlty gravelly	/ CLAV with	<u> </u>	0.25	75.54						
	some ha	iir roots. Sand is fine to coarse, gr ubangular to subrounded. rm, greyish brown, very sandy gra	ravel is fine to	<u> </u>	0.60	75.19		AA200188	В	0.50		
1.0	medium fine to co	cobbles content. Sand is fine to c parse subangular to subrounded, bangular to angular. (Possible ve	coarse, gravel is cobbles are									
	Firm to s	tiff, brown, slightly sandy gravelly and low boulders content. Sand is	CLAY with high s fine to coarse.		1.50	74.29		AA200189	В	1.30		
	gravel is	fine to coarse subangular to sub- and boulders are subangular to a	rounded,		1.90	73.89		AA200190	В	1.80		
3.0												
4.0												
Grou i TP dr		conditions										
Stabil TP sta			The second secon									
Gene TP do	ral Remar ne for civ	ks ic offices project		·								
	-	AND		· · · · · · · · · · · · · · · · · · ·								



J.S.S.T.	Т	RIAL PIT I	RECO	RD					24	665	
CONTRACT	Monaghan Active Travel					***	TRIAL P	IT NO.	TPO		
LOGGED BY	I.Reder	CO-ORDINATI		833,8	26.80 E 58.25 N		DATE ST		28/0	et 1 of 1 4/2023 4/2023	
	Monaghan Co.Co. DBFL/Cora	GROUND LEV	/EL (m)	79.90			EXCAVA METHO		3T T mac	racked hine	
								Sample	s)a)	meter
	Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0 TOPSOIL			7.37.3 7.8.77	0.25	79.65						
k gravel is f	m, brown, slightly sandy gravelly Cl and hair roots content. Sand is fine fine to coarse subangular to subrou	ınded	10 19 10 1	0.50	79.40		7				
Firm to st with high to coarse	tre small subangular to subrounded iff, greyish brown, slightly sandy gra cobbles and low boulders content. , gravel is fine to coarse subangula ed, cobbles and boulders are suba	avelly CLAY Sand is fine				,	AA200195	В	0.80		
							AA200196	В	1.80		
TP termin End of Tri	ated at 2.2m due to many boulders ial Pit at 2.20m	3	O Do	2.20	77.70						
3.0											
4.0											
Groundwater Co	nditions										
ΓP dry											
Stability P stable											
General Remark	S coffices project			 		··· · · · · · · · · · · · · · · · · ·					
T GOLIE TOT CIVIC	, omoes project										



U.S.	155L		TRIAL PIT	RECO	RD					24	665	
CON	TRACT	Monaghan Active Travel						TRIAL P	IT NO.	TPO		
LOG	LOGGED BY I.Reder				667,477.14 E 833,842.01 N			DATE S'				
CLIE	NT NEER	Monaghan Co.Co. DBFL/Cora	GROUND LE		75.17	F		EXCAVA METHO		3T Tracked machine		
								Sampl		s	a)	meter
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Type	Depth	Vane Test (KPa)	Hand Penetrometer
0.0	TOPSOI	L	7 77 7 77 77	2.05	74.00							
	roots cor	wn, slightly sandy slightly gravelly ntent. Sand is fine to coarse, grave	0 0	0.25 0.50	74.92 74.67							
1.0	Coarse subangular to subrounded. Firm to stiff, greyish brown, slightly sandy gravelly slightly silty CLAY with high cobbles and boulders content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles and boulders are subangular to angular.				0.30	74.07		AA200191	В	0.70		
	TP termi End of T	nated at 1.5m due to many boulde rial Pit at 1.50m	at 1.5m due to many boulders at 1.50m			73.67		AA200192	В	1.50		
3.0												
4.0												
		conditions										
ΓP dr	у											
Stabi P st	lity able		······································									
iene	ral Remar	ks ic offices project	······································				**************************************				 	•
ut	101 019											



	33L	Т	RIAL PIT	RECO	RD					24	665	
CON	TRACT	Monaghan Active Travel						TRIAL P	IT NO.	TP10 Sheet 1 of 1		
LOGGED BY i.Reder		CO-ORDINATES 667,449.28 833,875.03			75.03 N		DATE ST		D 28/04/2023		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
CLIE ENGI	NT NEER	Monaghan Co.Co. DBFL/Cora	GROUND LEVEL (m		81.69			EXCAVA METHO		3T Tracked machine		
									Sample		(a)	meter
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
1.0	roots co- coarse s Stiff to v gravelly Sand is to subro angular.	own, slightly sandy slightly gravelly Content. Sand is fine to coarse, gravel subangular to subrounded. ery stiff, greyish brown to brown, slightly with high cobbles and boulde fine to coarse, gravel is fine to coarse, gravel is sone to coarse, unded, cobbles and boulders are su	is fine to ghtly sandy ers content		0.30 0.50 2.50	81.39 81.19		AA200197 AA200198 AA200199	В	0.60 1.60 2.50		
TP di	У	Conditions										
Stabi TP st	lity able	***************************************						······································				
	ral Remar	rks ric offices project	***************************************									
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TRIAL PIT RECORD

)](g	ಕರ೭ 📗									2.4	665	
CON	TRACT	Monaghan Active Travel						TRIAL PI SHEET	T NO.	TP1	1 et 1 of 1	***************************************
LOG	GED BY	I.Reder	CO-ORDINA	CO-ORDINATES		667,482.17 E 833,886.75 N			ARTED			
CLIE	NT NEER	Monaghan Co.Co. DBFL/Cora	GROUND LE	GROUND LEVEL (m) 76.84				EXCAVA METHOE			3T Tracked machine	
									Sample	s	Vane Test (KPa)	meter
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth		Hand Penetrometer
0.0	TOPSO Soft to f	OIL ofirm, brown/grey mottled, slightly sandy slightly by CLAY with low cobbles content. Sand is fine to		9 4	0.20	76.64						
1.0	coarse,	gravel is fine to coarse subangula ded, cobbles are small subangula	content. Sand is fine to subangular to					AA205152	В	0.70		
	with med	stiff, greyish brown, slightly sandy dium cobbles and low boulders co parse, gravel is fine to coarse sub ded, cobbles and boulders are su	ntent. Sand is angular to		1.10	75.74	·	AA205153	В	1.50		
2.0	TP terminated at 2.3m due to many boulders End of Trial Pit at 2.30m		ers		2.30	74.54		AA205154	В	2.20		
3.0												
4.0												
Groui P dr		Conditions										-
Stabi P stabi												
	ral Remai one for civ	rks ric offices project	AP-A-NAVIIIVIII III III III III III III III II									***************************************



TRIAL PIT RECORD

REPORT NUMBER

24665

CONTRACT Monaghan Active Travel CO-GRDNATES 687, 491.71 E SAME TO JOHN COMMITTED CONTROL OF STREET COMMITTED DATE STARTED DATE STARTED DATE STARTED DATE STARTED DATE STARTED DATE STARTED DATE COMMITTED CONSIDERS COUNTY Monaghan Co.Co ENGINEER DESCRIPTION 37 T-46 COUNTY MONAGHAN CO.Co ENGINEER DESCRIPTION 37 T-3 ENCONTROL OF	Ų.	331									۷4	.000	
CO-ORDINATES 667.491.71 E SASJ.909.43 N Monaghan Co.Co. DOFI/Cora Tracked METHOD METHOD MATE COMPLETED 040552023 EXCAVATION ST Tracked METHOD METHOD MATE COMPLETED 040552023 EXCAVATION ST Tracked METHOD M	CON	TRACT	Monaghan Active Travel						1	IT NO.			
Geotechnical Description Geotechnical Descr	LOG	GED BY	I.Reder		833,909.43 N				DATE ST		ED 04/05/2023		
TOPSOIL Soft, brown, slightly sandy slightly gravelly CLAY with hair roots content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded. Soft to firm, greysh brown, slightly sandy slightly gravelly CLAY with how cobbles content. Sand is fine to coarse subangular to subrounded. Soft to firm, greysh brown, slightly sandy very gravelly CLAY with high cobbles and boulders content. Sand is fine to coarse, gravel y clay with high cobbles and boulders are subangular to angular. (possible very clayery angular gravel and cobbles) Telephone Tel				GROUND LE	VEL (m)	77.46		·					
TOPSOIL Soft, brown, slightly sandy slightly gravelly CLAY with hair roots content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded. Soft to firm, greysh brown, slightly sandy slightly gravelly CLAY with how cobbles content. Sand is fine to coarse subangular to subrounded. Soft to firm, greysh brown, slightly sandy very gravelly CLAY with high cobbles and boulders content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded. Soft to firm, greysh brown, slightly sandy very gravelly CLAY with high cobbles and boulders content. Sand is fine to coarse, gravel y clay with high cobbles and boulders are subangular to angular, cobbles and boulders are subangular to angular, cobbles and boulders are subangular to angular. (possible very clayery angular gravel and cobbles) Telephore to a gravel is fine to coarse, gravelly clayers and gravelly clayers and gravelly clayers and gravelly clayers. The first to coarse, gravelly clayers and gravelly clayers and gravelly clayers. Soft to firm, gravelly clayers and gravelly clayers and gravelly clayers. The first coarse gravelly clayers and gravelly clayers. The first coarse gravelly clayers and gravelly clayers. The first coarse gravelly clayers and gravelly clayers and gravelly clayers. The first coarse gravelly clayers and gravelly clayers and gravelly clayers. The first coarse gravelly clayers and gravelly clayers and gravelly clayers. The first coarse gravelly clayers and gravelly clayers and gravelly clayers. The first coarse gravely clayers and gravelly clayers and gravelly clayers and grav									,	Samples)a)	neter
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with hair roots content. Sand is fine to coarse, gravel is line to coarse subangular to subrounded. Soft to firm, greyish brown, slightly sandy slightly gravelly CLAY with low cobbles content. Sand is fine to coarse subangular to subrounded. Soft to firm, greyish brown, slightly sandy slightly gravelly CLAY with log cobbles and boulders content. Sand is fine to coarse subangular to subrounded. Soft to firm, greyish brown, slightly sandy very gravelly CLAY with log cobbles and boulders content. Sand is angular, cobbles and boulders are subangular to angular, (possible very clayey angular gravel and cobbles) TP terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m A205179 B 1.70 A205179 B 1.70 Stability P stable Seneral Remarks	0.0				<u> </u>		77 16			,			
CLAY with tou coarse subangular to subrounded, cobbles are small subangular to subrounded. Soft to firm, greyish brown, slightly sandy very gravelly CLAY with high cobbles and boulders content. Sand is fine to coarse subangular to angular, cobbles and boulders are subangular to angular. (possible very clayery angular gravel and cobbles) TP terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m 1.80 To terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m Simulation of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m		with hair fine to c	roots content. Sand is fine to oarse subangular to subround	coarse, gravel is led.	0								
ine to coarse, gravel is fine to coarse subangular to angular, cobbles and boulders are subangular to angular, (possible very clayey angular gravel and cobbles) TP terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m To undwater Conditions P dry Tability P stable eneral Remarks	1.0	CLAY with low cobbles content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles are small subangular to subrounded. Soft to firm, grevish brown, slightly sandy very gravelly				1.00	76.46		AA205178	В	0.80		
The terminated at 1.8m due to boulders or rock End of Trial Pit at 1.80m 1.00		fine to co angular,	carse, gravel is fine to coarse cobbles and boulders are su	subangular to bangular to angular.		1.90	75.66		AA205179	B	1 70	·	
4.0 Groundwater Conditions P dry Stability P stable General Remarks	2.0					1.50	75.00						
4.0 Groundwater Conditions TP dry Stability TP stable General Remarks						Walter Works							
Groundwater Conditions TP dry Stability TP stable General Remarks	3.0												
P dry Stability P stable General Remarks	4.0				PROPERTY OF THE PARTY AND ADDRESS OF THE PARTY OF THE PAR								
Stability P stable General Remarks													
P stable General Remarks			Conditions		:						:		
TP stable General Remarks	Stabil	itv											
	P sta	able											



رز	55L	1	TRIAL PIT	RECO	RD					24	665	
CON	TRACT	Monaghan Active Travel				7/17/1/11		TRIAL P	PIT NO.	TP1		APPR
LOG	GED BY	I.Reder		CO-ORDINATES 667,464.88 E 833,929.00 N GROUND LEVEL (m) 83.28				DATE S		- · · · ·		
CLIE	NT INEER	Monaghan Co.Co. DBFL/Cora	GROUND LEV				83.28		EXCAVATION METHOD		3T Tracked machine	
								Sample		s	(a)	meter
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0	TOPSOIL Soft brown clightly conduction by clightly group its CLAY			77 74	0.20	83.08						
	Soft, brown, slightly sandy slightly slightly gravelly CLAY with hair roots content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded. Firm to stiff, greyish brown, slightly sandy slightly gravelly CLAY with low cobbles and boulders content. Sand is fine to coarse, gravel is fine to coarse subangular to subrounded, cobbles and boulders are subangular to subrounded.				0.50	82.78			-			
1.0					0.50	J. T.		AA205173	В	0.60		
- - -	TP termi End of T	terminated at 1.4m due to many boulders d of Trial Pit at 1.40m		<u> </u>	1.40	81.88		AA205174	В	1.40	THE PROPERTY OF THE PROPERTY O	
2.0												
3.0												
4.0						e e e e e e e e e e e e e e e e e e e						
- -						петима стано вымамы.						
Grou	ndwater C	Conditions							······································			 -
31	•											
Stabi TP st	lity able			····								
Gene	ral Remar	ks ic offices project										

IGSL TP LOG 24665.GPJ IGSL.GDT 10/5/23



TRIAL PIT RECORD

REPORT NUMBER

24665

J@	93T /									24	000	
CON	TRACT	Monaghan Active Travel	•					TRIAL P	IT NO.	TP1	4 et 1 of 1	
LOG	GED BY	I.Reder	CO-ORDINAT	667,490.90 E 833,949.34 N				TARTED OMPLET	D 04/05/2023			
CLIE	NT INEER	GROUND LEVEL (m)		80.90	80.90		EXCAVATION METHOD			3T Tracked machine		
								Samples	S	a)	meter	
		Geotechnical Description		Legend	Depth (m)	Elevation	Water Strike	Sample Ref	Туре	Depth	Vane Test (KPa)	Hand Penetrometer (KPa)
0.0	TOPSOI			14.34.34 12.312.31	0.20	80.70						
-	CLAY wi	rm, brown, slightly sandy slightly slig th hair roots content. Sand is fine to fine to coarse subangular to subrou	coarse,		0.50	80.40		4.4				
0	Firm to s with low coarse.	tiff, greyish brown, slightly sandy gra cobbles and low boulders content. S gravel is fine to coarse subangular to ded, cobbles and boulders are suba	avelly CLAY Sand is fine to		0.50	80.40		AA205175	В	0.70		
								AA205176	В	1.50		
2.0	TP termi End of T	nated at 2.1m due to many boulders rial Pit at 2.10m	5		2.10	78.80		AA105177	В	2.10		
3.0												
4.0												
Grou TP di	ndwater C	conditions						4			<u> </u>	
Stabi TP st	ility able							***************************************				

IGSL TP LOG 24665.GPJ IGSLGDT 10/5/23

General Remarks
TP done for civic offices project

Site: Monaghan Active Travel Project Engineer: DBFL/CORA





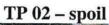
TP 01 – spoil



Site: Monaghan Active Travel Project Engineer: DBFL/CORA









Site: Monaghan Active Travel Project Engineer: DBFL/CORA







Site: Monaghan Active Travel Project Engineer: DBFL/CORA





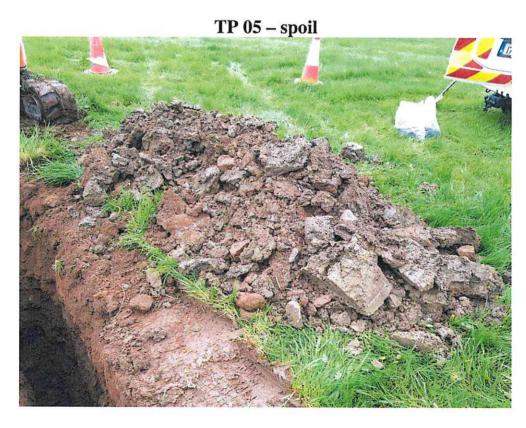
TP 04 – spoil



Site: Monaghan Active Travel Project Engineer: DBFL/CORA



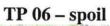




Site: Monaghan Active Travel
Project Engineer: DBFL/CORA





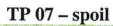




Site: Monaghan Active Travel Project Engineer: DBFL/CORA









Site: Monaghan Active Travel Project Engineer: DBFL/CORA









Site: Monaghan Active Travel Project Engineer: DBFL/CORA

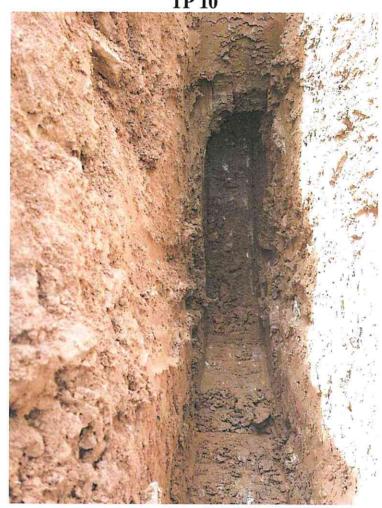






Site: Monaghan Active Travel Project Engineer: DBFL/CORA





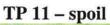




Site: Monaghan Active Travel Project Engineer: DBFL/CORA









Site: Monaghan Active Travel Project Engineer: DBFL/CORA







Site: Monaghan Active Travel Project Engineer: DBFL/CORA





TP 13 – spoil



Site: Monaghan Active Travel Project Engineer: DBFL/CORA





TP 14 – spoil





f -value from field tests Soakaway Design IGSI Contract: Monaghan, Active Travel 24665 Test No. SA01 **Engineer CORA** Date: 04/05/2023 Summary of ground conditions Description from to Ground water 0.00 0.25 TOPSOIL 0.25 0.50 Soft, brown, slightly sandy slightly gravelly CLAY with low hair roots content 0.50 DRY 1.30 Soft to firm, brown/grey mottled, slightly sandy gravelly slightly silty CLAY with high subangular to angular cobbles and boulders content 1.30 Obstruction - boulders Location: E:667491.477; N:833784.047; G.L. 71.944mOD Notes: SA01 done for Civic Offices project Field Data Field Test Depth to Elapsed Depth of Pit (D) 1.30 m Water 0.50 Time Width of Pit (B) m 2.00 (m) (min) Length of Pit (L) m 0.500 0.00 Initial depth to Water = 0.50 m 0.510 Final depth to water = 1.00 0.73 m 0.530 2.00 30.00 Elapsed time (mins)= 0.560 3.00 0.580 4.00 Top of permeable soil m 0.590 5.00 Base of permeable soil 0.600 6.00 0.605 7.00 0.610 8.00 0.615 9.00 0.620 10.00 Base area= 1 m2 0.640 12.00 *Av. side area of permeable stratum over test period 3.425 m2 0.660 14.00 Total Exposed area = 4.425 m2 16.00 0.670 18.00 0.680 20.00 0.690 Infiltration rate (f) = Volume of water used/unit exposed area / unit time 0.710 25.00 0.730 30.00 f= 0.00173 m/min or 2.88763E-05 m/sec Depth of water vs Elapsed Time (mins) 35.00 30.00 Time(mins) 25.00 20.00 15.00 10.00 5.00 0.00 0.000 0.100 0.200 0.300 0.400 0.700 0.500 0.600 0.800 Depth to Water (m)

f -value from field tests Soakaway Design **IGSI** Contract: Monaghan, Active Travel 24665 Test No. SA02 Engineer CORA Date: 04/05/2023 Summary of ground conditions from Description Ground water to 0.00 0.20 TOPSOIL 0.20 0.70 Soft to firm, brown, slightly sandy slightly gravelly CLAY with medium cobbles 0.70 1.60 Firm to stiff, greyish brown, slightly sandy gravelly slightly silty CLAY with low DRY subangular to angular cobbles and boulders content Location: E:667480.695; N:833861.983; G.L. 75.647mOD Notes: SA02 done for Civic Offices project Field Data Field Test Depth to Elapsed Depth of Pit (D) 1.60 m Water Time 0.50 Width of Pit (B) m 2.00 (m) (min) Length of Pit (L) m 0.600 0.00 Initial depth to Water = 0.60 m 0.610 1.00 Final depth to water = 0.68 m 0.620 2.00 Elapsed time (mins)= 60.00 0.630 3.00 0.630 4.00 Top of permeable soil m 0.640 5.00 Base of permeable soil 0.640 6.00 0.640 7.00 0.640 8.00 0.640 9.00 0.640 10.00 Base area= m2 0.640 12.00 *Av. side area of permeable stratum over test period 4.8 m2 14.00 0.640 Total Exposed area = 5.8 m2 0.640 16.00 0.650 18.00 0.660 20.00 Infiltration rate (f) = Volume of water used/unit exposed area / unit time 0.660 25.00 f= 0.00023 m/min 0.670 30.00 or 3.83142E-06 m/sec 0.670 40.00 0.680 50.00 0.680 60.00 Depth of water vs Elapsed Time (mins) 70.00 60.00 Time(mins) 50.00 40.00 30.00 20.00 10.00 \$ 0.00 0.580 0.600 0.620 0.640 0.660 0.680 0.700

Depth to Water (m)

f -value from field tests Soakaway Design IGS Contract: Monaghan, Active Travel 24665 Test No. SA03 **Engineer CORA** Date: 04/05/2023 Summary of ground conditions from to Description Ground water TOPSOIL 0.00 0.30 0.30 1.60 Firm to stiff, greyish brown, slightly sandy gravelly slightly silty CLAY with high cobbles and low boulders content DRY Location: E:667448.448; N:833888.586; G.L. 83.582mOD Notes: SA03 done for Civic Offices project Field Data Field Test Depth to Elapsed Depth of Pit (D) 1.60 m Water Time Width of Pit (B) 0.50 m (m) (min) Length of Pit (L) 2.00 0.540 0.00 Initial depth to Water = 0.54 m 0.540 1.00 Final depth to water = 0.55 m 0.540 2.00 Elapsed time (mins)= 30.00 0.540 3.00 0.550 4.00 Top of permeable soil m 0.550 5.00 Base of permeable soil 0.550 6.00 Water movement stoped at 0.55m 0.550 7.00 0.550 8.00 0.550 9.00 0.550 10.00 Base area= 1 m2 0.550 12.00 *Av. side area of permeable stratum over test period 5.275 m2 14.00 0.550 Total Exposed area = 6.275 m2 0.550 16.00 0.550 18.00 0.550 20.00 Infiltration rate (f) = Volume of water used/unit exposed area / unit time 0.550 25.00 0.550 30.00 f= 5.3E-05 m/min or 8.85347E-07 m/sec Depth of water vs Elapsed Time (mins) 35.00 30.00 Time(mins) 25.00 20.00 15.00 10.00 5.00 0.00 0.540 0.542 0.538 0.544 0.546 0.548 0.550 0.552 Depth to Water (m)

f -value from field tests Soakaway Design IGS Contract: Monaghan, Active Travel 24665 Test No. SA04 **Engineer CORA** Date: 04/05/2023 Summary of ground conditions from Description to Ground water 0.00 TOPSOIL 0.25 0.25 0.50 Soft, brown, sl. sandy sl. gravelly CLAY with low cobbles and hair rrots content 0.50 1.30 Firm to stiff, brown, slightly sandy gravelly CLAY with high subangular to subrour DRY cobbles and low boulders content 1.30 Obstruction - boulders Location: E:667494.53; N:833936.177; G.L. 79.506mOD Notes: SA04 done for Civic Offices project Field Data Field Test Depth to Elapsed Depth of Pit (D) 1.30 m Water Time Width of Pit (B) 0.50 m (m) (min) Length of Pit (L) 1.50 0.480 0.00 Initial depth to Water = 0.48 m 0.480 1.00 Final depth to water = 0.48 m 0.480 2.00 Elapsed time (mins)= 30.00 0.480 3.00 0.480 4.00 Top of permeable soil m 0.480 5.00 Base of permeable soil 0.480 6.00 No Water Movement 0.480 7.00 0.480 8.00 0.480 9.00 0.480 10.00 Base area= 0.75 m2 0.480 12.00 *Av. side area of permeable stratum over test period 3.28 m2 14.00 0.480 Total Exposed area = 4.03 m2 0.480 16.00 0.480 18.00 0.480 20.00 Infiltration rate (f) = Volume of water used/unit exposed area / unit time 0.480 25.00 0.480 30.00 f= 0 m/min or 0 m/sec Depth of water vs Elapsed Time (mins) 35.00 30.00 Time(mins) 25.00 20.00 15.00 10.00 5.00 0.00 0.000 0.100 0.200 0.300 0.400 0.500 0.600 Depth to Water (m)

Site: Monaghan Active Travel Project Engineer: DBFL/CORA



TRIAL PIT PHOTOGRAPHY RECORD SA 01



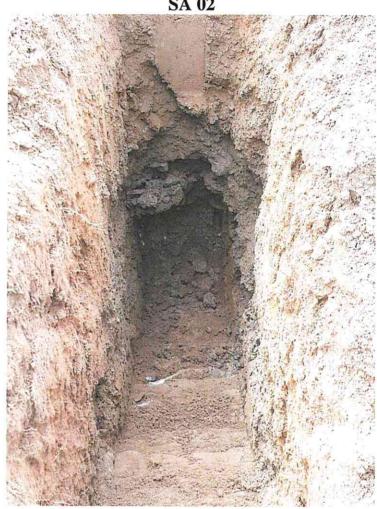




Site: Monaghan Active Travel Project Engineer: DBFL/CORA



TRIAL PIT PHOTOGRAPHY RECORD SA 02

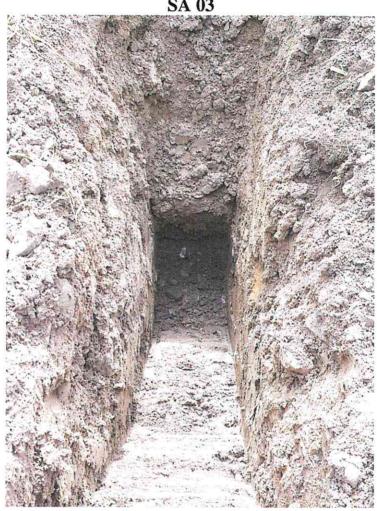




Site: Monaghan Active Travel Project Engineer: DBFL/CORA



TRIAL PIT PHOTOGRAPHY RECORD SA 03





Site: Monaghan Active Travel Project Engineer: DBFL/CORA

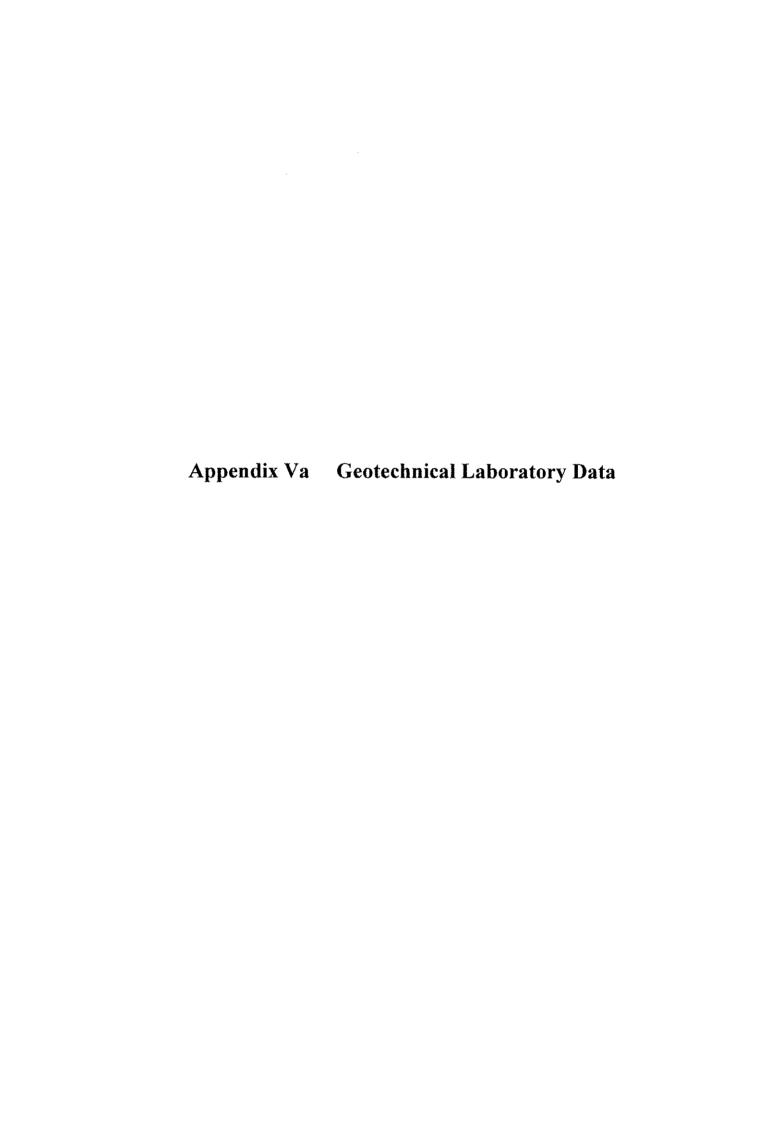


TRIAL PIT PHOTOGRAPHY RECORD SA 04

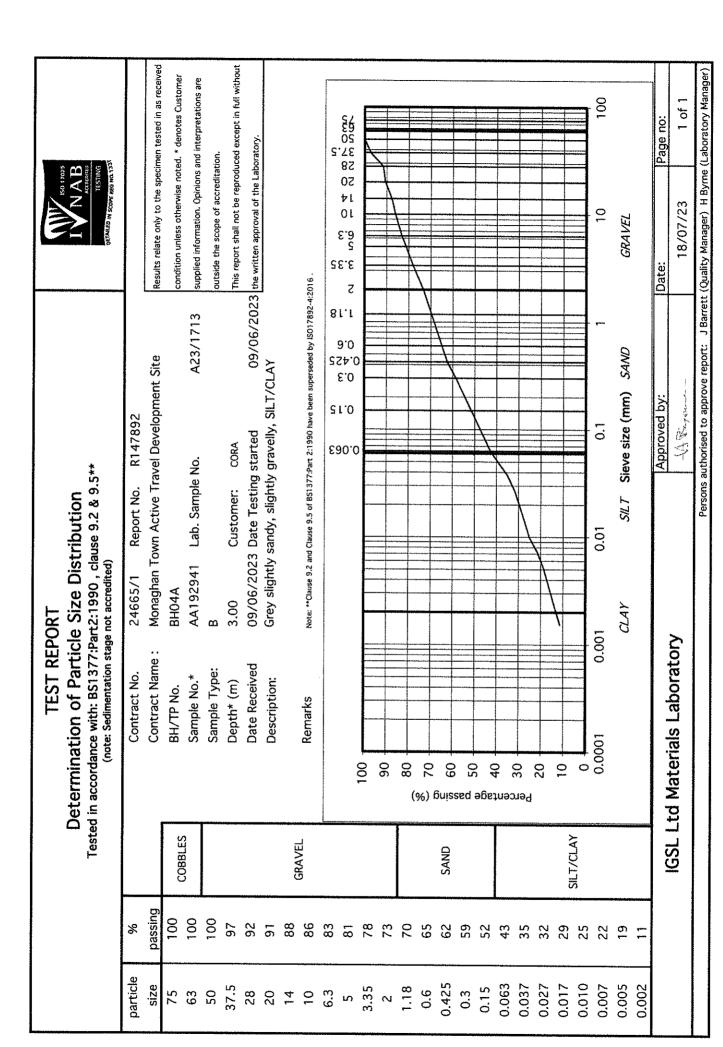


SA 04 – spoil





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			Monaghan Town Active Travel Development Site			Description	Brown sandy gravelly CLAY	Brown sandy gravelly CLAY	Brown sandy gravelly CLAY	Brown sandy gravelly SILT	Grey brown sandy gravelly CLAY	Grey brown sandy gravelly CLAY							Results relate only to the specimen tested, in as received condition unless otherwise noted.	892-12.	Opinions and interpretations are outside the scope of accreditation." denotes Customer supplied information.	This report shalf not be reproduced except in fullwithout written approval from the Laboratory.		18/07/23
		3**	iive Travel			Classification (BS5930)	70	70	- 0		CL	70							adition unless	92-1 and EN17	ditation. * deno	ten approval fr		
	lic Limits	3, 4.4 & 5.	Town Act			Preparation Liquid Limit	4.4	4.4	4.4	4.4	4.4	4.4							as received co	ded by EN 178	scope of accre	fullwithout writ	ру	一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一一
	d & Plast	ises 3.2, 4.	Monaghar			ļ	WS	WS	SM	MS	WS	WS							cimen tested,in	NOTE: "These clauses have been superceded by EN 17892-1 and EN17892-12.	are outside the	fuced except in	Approved by	多字
oort	ent, Liqui	:1990, clau	Vame:			% <425µm	67	99	71	85	61	72							only to the sper	e clauses have	nterpretations	all not be reproc		
Test Report	ure Conte	377:Part 2	Contract Name:			Plasticity Index	14	16	17	М	14	14						Remarks:	Results relate	NOTE: "Thes	Opinions and i	This report sha		Aanager)
 	of Moist	e with BS1				Plastic Limit %	14	17	19	ď	16	19						Irbed	P 3				ve reports	aboratory A
	Determination of Moisture Content, Liquid & Plastic Limits	in accordance with BS1377:Part 2:1990, clauses 3.2, 4.3, 4.4 & 5.3**	24665/1		09/06/23	Liquid Limit %	28	33	36	32	30	33						B - Bulk Distu	U - Undisturbed				ized to appro	H Byrne (Laboratory Manager)
	Deter	Tested in	No.		sted:	Moisture Content %	18	14	16	19	14	14						Sample Type: B - Bulk Disturbed					Persons authorized to approve reports	
			Contract No.		Date Tested:	Sample Type*	В	В	В	В	В	В									method			
					09/06/23	Lab. Ref	A23/1710	A23/1711	A23/1712	A23/1714	A23/1716	A23/1717									meter definitive	meter one poin		boratory
			R147891	Cora	eived:	epth* (m)	2.0	1.0	1.0	1.0	0.7	9.0						WS - Wet sieved	AR - As received	NP - Non plastic	4.3 Cone Penetrometer definitive method	4.4 Cone Penetrometer one point method	-	teriais La
ratory	usiness Park		Report No.	Customer	Samples Received:	Sample No. Depth* (m)	AA192933	AA197802	AA192935	AA192947	AA200193	AA200179	************			*****		Preparation: W	∢		Ξ	Clause: 4.		IGSL Ltd Materials Laboratory
IGSL Ltd Materials Laboratory	Unit J5, M7 Business Park Newhall, Naas	Co. Kildare 045 846176				ВН/ТР*	BH01	BH02	ВН03	BH05	TP01	TP03										_	<u>`</u>	<u>5</u>



TEST REPORT Determination of Particle Size Distribution

Tested in accordance with: BS1377:Part2:1990 , clause 9.2 & 9.5** (note: Sedimentation stage not accredited)



particle	%			Contract No.	24665/1	Report No.	R147893			
size	passing			Contract Name:	Monaghan T	own Active Tra	Monaghan Town Active Travel Development Site	t Site	Results relate only to the specimen tested in as received	nen tested in as received
75	100	CORRIEC		BH/TP No.	BH08		-		condition unless otherwise noted. * denotes Customer	d. * denotes Customer
63	100	CORRECT		Sample No.*	AA192947	Lab. Sample No.	No.	A23/1715	supplied information. Opinions and interpretations are	nd interpretations are
20	94			Sample Type:	В				outside the scope of accreditation.	on.
37.5	94			Depth* (m)	2.00	Customer:	CORA		This report shall not be reproduced except in full without	ced except in full without
28	89			Date Received	09/06/202	09/06/2023 Date Testing started	started	09/06/2023	09/06/2023 the written approval of the Laboratory.	oratory.
20	88			Description:	Grey sandy,	Grey sandy, slightly gravelly, SILT/CLAY	, SILT/CLAY			
4	98	GRAVFI								
10	85	77 A LAID		Remarks	Note: **Clause 9.2 a	ind Clause 9.5 of BS1377	Part 2:1990 have been s	Note: **Clause 9.2 and Clause 9.5 of BS1377:Part 2:1990 have been superseded by ISO17892-4:2016	2016.	
6.3	82							\$2		Ş.
S	80						0.0 r.0	E.O S4.O ∋.O	3.3 5.6.3 10 10 14 20 20 20 20 20 3.3	28 20 50 53 54
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0.002	10			The state of the s						
		1001	A 1 1 1 2 2 2 2	neter to the train			Approved by:		Date:	Page no:
		IGSL L	.ta Mater	IGSL Ltd Materiais Laboratory	>		一个多一个		18/07/23	1 of 1
						Persons a	Persons authorised to approve report:		J Barrett (Quality Manager) H Byrne (Laboratory Manager)	(Laboratory Manager)

Determination of Particle Size Distribution Tested in accordance with: BS1377:Part2:1990, clause 9.2 & 9.5** (note: Sedimentation stage not accredited) **TEST REPORT**



particle	%			Contract No.	24665/1 Report No.	R147894		
size	passing			Contract Name:	Monaghan Town Active Travel Development Site	vel Development Site	Results relate only to the specimen tested in as received	en tested in as received
75	100	CORRIEC		BH/TP No.	TP05		condition unless otherwise noted. * denotes Customer	* denotes Customer
63	100	CORRECT		Sample No.*	AA200182 Lab. Sample No.	No. A23/1718	supplied information. Opinions and interpretations are	interpretations are
20	100			Sample Type:	В		outside the scope of accreditation.	<u>م</u>
37.5	100			Depth [⋆] (m)	0.70 Customer:	CORA	This report shall not be reproduced except in full without	d except in full without
28	66			Date Received	09/06/2023 Date Testing started		09/06/2023 the written approval of the Laboratory.	atory.
20	86			Description:	Brown slightly sandy, slightly gravelly, SILT/CLAY	ly gravelly, SILT/CLAY		
14	96	CDAVC						
10	95	GRAVEL		Remarks	Note: **Clause 9.2 and Clause 9.5 of 8S1377	Note: **Clause 9.2 and Clause 9.5 of BS1377-Part 2:1990 have been superseded by ISO17892-4:2016 .	2016,	
6.3	93			***************************************		S		S
Ŋ	92					90.0 1.0 5.0 3.0 1.1	3.3 56.3 10 14 20 28	28 28 37 50 83 50 50 50
3.35	96		100					
2	87		06					
1.18	84		80					
9.0	83		2 %) (%					
0.425	79	SAND	gnie					
0.3	22							
0.15	99							
0.063	55							
0.037	46							
0.027	41		50					
0.017	35	CII T/CI AV	10					
0.010	29		0					
0.007	56		0.0	0.0001 0.001	0.01	0.1	10	100
0.005	21				CLAY SILT SI	Sieve size (mm) SAND	GRAVEL	
0.002	14			ORGANIA III III III III III III III III III				
		1001	1 1 1 1 2 2 2	1 -1 -1 -1		Approved by:	Date:	Page no:
		IGSE E	to mate	IGSL Ltd Materials Laboratory	>	- V.J. B. y Excession	18/07/23	1 of 1

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IGSL Ltd Materials Laboratory

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Date:

Determination of Particle Size Distribution Tested in accordance with: BS1377:Part2:1990, clause 9.2 & 9.5** (note: Sedimentation stage not accredited) **TEST REPORT**



particle	%		_	Contract No.	24665/1 Report No. R147895	
size	passing			Contract Name:	Monaghan Town Active Travel Development Site	Results relate only to the specimen tested in as received
22	100	CORRIEC		3H/TP No.	TP09	condition unless otherwise noted. * denotes Customer
63	100	67766		Sample No.*	AA200191 Lab. Sample No. A2	A23/1719 supplied information. Opinions and interpretations are
20	100		~ /	Sample Type:	8	outside the scope of accreditation.
37.5	96		ا ست	Depth* (m)	0.70 Customer: CORA	This report shall not be reproduced except in full without
28	95			Date Received	09/06/2023 Date Testing started 0'	09/06/2023 the written approval of the Laboratory.
20	91			Description:	Grey brown sandy, slightly gravelly, SILT/CLAY	
4	88	13/1/05				
10	85	GNAVEL		Remarks	Note: **Clause 9.2 and Clause 9.5 of BS1377.Part 2:1990 have been superseded by ISO17892-4:2016	by iSO17892-4:2016 ,
6.3	8				S	CT 88
ιΩ	78		1		90.0 11.0 5.0	0.6 3.3 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5 5.5
3.35	74		100			
2	69		- 06			
1.18	65		+ 08 °			
9.0	09		2 %) f			
0.425	56	SAND	gnie:			
0.3	51					
0.15	39					
0.063	28					
0.038	23					
0.027	21		- 707			
0.017	8	CII T/CI AV	10 +			
0.010	16		<u> </u>			
0.007	15		0.0001	0.001	0.01 0.01	10 100
0.005	12				CLAY SILT Sieve size (mm) SAND	C GRAVEL
0.002	6					

TEST REPORT Determination of Particle Size Distribution Tested in accordance with: BS1377:Part2:1990, clause 9.2 & 9.5** (note: Sedimentation stage not accredited)



L_									A STANKE REG MO. 13.4.		
	particle	%			Contract No.	24665/1 Report No.	R147896	•			-
	size	passing		#	Contract Name:	Monaghan Town Active Travel Development Site	avel Development Site		Results relate only to the specimen tested in as received	en tested in as received	
	75	100	CORRIES		BH/TP No.	TP12			condition unless otherwise noted. * denotes Customer	. * denotes Customer	
	63	100	CORRECT		Sample No.*	AA205178 Lab. Sample No.	No. A23/1720	1720	supplied information. Opinions and interpretations are	d interpretations are	
	20	06			Sample Type:	œ			outside the scope of accreditation.	Ġ.	
	37.5	82			Depth* (m)	0.80 Customer:	CORA		This report shall not be reproduced except in full without	ed except in full without	
•••••	28	80			Date Received	09/06/2023 Date Testing started)6/2023	09/06/2023 the written approval of the Laboratory.	atory.	_
	20	75			Description:	Brown slightly sandy, gravelly, SILT/CLAY		•			_
	4	7.1	CDAVE								
	10	89			Remarks	Note: **Clause 9.2 and Clause 9.5 of BS1377;Part 2:1990 have been superseded by ISO17892-4:2016 .	77:Part 2:1990 have been superseded by	15017892-4:20	16.		
****	6.3	63				AND ADDRESS OF THE PERSON NAMED IN THE PERSON	S		The state of the s	S	
	52	09					90.0 1.0 €.0 S₱.0	ויו	3.8 8.8 8.9 14 82 82	28 28. 37. 503 503 503 503	
*****	3.35	57		100							
	2	52		06							
	1.18	48		80							
	9.0	42		2 1 (%							
	0.425	39	SAND	gnie							
	0.3	34							\ \		
·	0.15	53						1			
	0.063	20									
<u>-</u>	0.038	17					\				
	0.027	15		20							
	0.017	13	CH T/CI AV	10							
	0.010	12	S-1-15	0							
	0.007	11		õ	0.0001 0.001	0.01	0.1		10	100	
	0.005	თ				CLAY SILT	Sieve size (mm) SAND		GRAVEL		
	0.002	7			THE PARTY STORMS STORMS STORMS STORMS	A THE					
			- 1001	44 1424			Approved by:		Date:	Page no:	
			192F L	to Mate	IGSL Ltd Materials Laboratory	λ	(18 x xxxxxxx		18/07/23	1 of 1	
-					THE REAL PROPERTY OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COL					Barner 1	

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Date:

		>	DETAILED IN SCO
TEST REPORT	Determination of Particle Size Distribution	Tested in accordance with: BS1377:Part2:1990, clause 9.2 & 9.5**	(note: Sedimentation stage not accredited)



10:11:00	6							
particle	8			Contract No.	24665/1 Report No.	40. R147897		
size	passing			Contract Name:	Monaghan Town Active	Monaghan Town Active Travel Development Site	Results relate only to the	Results relate only to the specimen tested in as received
22	90	CORRIFS		BH/TP No.	TP14		condition unless otherwise	condition unless otherwise noted. * denotes Customer
63	100			Sample No.*	AA205176 Lab. Sample No.		A23/1721 supplied information. Opin	supplied information. Opinions and interpretations are
20	100			Sample Type:	മ		outside the scope of accreditation.	ditation,
37.5	93			Depth* (m)	1.50 Customer:	er: CORA	This report shall not be re	This report shall not be reproduced except in full without
28	89			Date Received	09/06/2023 Date Testing started		09/06/2023 the written approval of the Laboratory.	Laboratory.
20	98			Description:	Grey brown slightly sar	SILT/CLA		
4	82	CDAVE						
10	2.2	פֿעאַ	-t-Walla tama	Remarks	Note: **Clause 9.2 and Clause 9.5 of	Note: **Clause 9.2 and Clause 9.5 of BS1377;Part 2:1990 have been superseded by ISO17892-4:2016	y ISO1 7892-4:2016 .	
6.3	72				A. C.	S	8	S
Ŋ	69					90.0 1.0 8.0	0.6 3.3 3.3 5.6.3 10 10	28 28. 37. 58. 58. 58. 58.
3.35	09		100					
2	53		06					X
1.18	49		80					
9.0	45		2 %) f					
0.425	43	SAND	Suiz:					
0.3	40							
0.15	34							
0.063	56							
0.038	23							
0.027	21		50					
0.017	19	CII T/CI AV	10					
0.010	17	5777	Ó					
0.007	15		ე. ე.	0.0001 0.00	0.01	0.1	1 10	100
0.005	13				CLAY SILT	7 Sieve size (mm) SAND	GRAVEL	
0.002	10							

IGSL Ltd Materials Laboratory Unit J5,M7 Business Park

Naas

Co. Kildare

Test Report

Determination of Moisture Condition Value at Natural Moisture Content



Tested in accordance with BS1377:Part 4:1990, clause 5.4

045 899324	100.00 11 00001001100	7 Will D07077.1 art 4.1990, clause 3.4
	Report No.	R147898
	Contract No.	24665/1
	Contract Name:	Monaghan Town Active Travel Development Site
	Customer:	CORA
	BH/TP*	TP01
	Sample No.*	AA200193
	Depth* (m)	0.70
	Sample Type:	В
	Lab Sample No.	A23/1716
	Source* (if applicable)	N/A
	Material Type* (if applicable):	В
	Sample Received:	09/06/23
	Date Tested:	09/06/23
	Sample Cert:	Not Provided
	Moisture Content (%):	15
	% Particles > 20mm (By dry mass):	16
	MCV:	6.6
	Interpretation of Plot:	Steepest Straight Line
	Description of Soil:	Grey brown sandy gravelly CLAY

Results relate only to the specimen tested, in as received condition unless other	rwise noted.	Persons authorised to	approve reports
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File: R147898 TP01 @ 0.7m MCV

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Materials Laboratory Unit J5,M7 Business Park

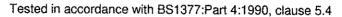
Naas

Co. Kildare 045 899324

Test Report

Determination of Moisture Condition Value at Natural Moisture Content







Report No. R147899 Contract No. 24665/1

Contract Name: Monaghan Town Active Travel Development Site

Customer: CORA

BH/TP* TP03

Sample No.* AA200179

Depth* (m) 0.60

Sample Type:

Lab Sample No. A23/1717

Source* (if applicable) N/A

Material Type* (if applicable): В

Sample Received: 09/06/23

Date Tested: 09/06/23

Sample Cert: Not Provided

Moisture Content (%): 13

% Particles > 20mm 15

(By dry mass):

MCV: 7.3

Interpretation of Plot: Steepest Straight Line

Description of Soil: Grey brown sandy gravelly CLAY

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IGSL Ltd Materials Laboratory Unit J5,M7 Business Park

Naas

Test Report

Determination of Moisture Condition Value at Natural Moisture Content



			Outlon	TESTI
Co. Kildare 045 899324	***************************************	Tested in accord	dance with BS1377:Part 4:1990, clause 5.4	DETAILED IN SCOPE AEG NO
	Report No		R147900	
	Contract N	o.	24665/1	
	Contract N	ame:	Monaghan Town Active Travel Developm	nent Site
	Customer:		CORA	
	BH/TP*		TP05	
	Sample No).*	AA200182	
	Depth* (m)		0.70	
	Sample Ty	pe:	В	
	Lab Sampl	e No.	A23/1718	
	Source* (if	applicable)	N/A	
	Material Ty	pe* (if applicable):	В	
	Sample Re	ceived:	09/06/23	
	Date Teste	d:	09/06/23	
	Sample Ce	ert:	Not Provided	
	Moisture C	ontent (%):	13	
	% Particles (By dry ma		11	
	MCV:		6.8	
	Interpretation	on of Plot:	Steepest Straight Line	
	Description	of Soil:	Brown slightly sandy, slightly gravelly, SIL	_T/CLAY

Results relate only to the specimen tested, in as received condition unless othe	wise noted.	Persons authorised to	approve reports
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Unit J5,M7 Business Park

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Test Report

Determination of Moisture Condition Value at Natural Moisture Content



Co. Kildare 045 899324		Tested in accordance with BS1377:Part 4:1990, clause 5.4		
	Report No	í.	R147901	
	Contract N	о.	24665/1	
	Contract N	ame:	Monaghan Town Active Travel Developme	ent Site
	Customer:		CORA	
	BH/TP*		TP09	
	Sample No).*	AA200191	
	Depth* (m)		0.70	
	Sample Ty	pe:	В	
	Lab Sampl	e No.	A23/1719	
	Source* (if	applicable)	N/A	
	Material Ty	rpe* (if applicable):	В	
	Sample Re	ceived:	09/06/23	
	Date Teste	d:	09/06/23	
	Sample Ce	ert:	Not Provided	
	Moisture C	ontent (%):	13	
	% Particles (By dry ma		11	
	MCV:		6.8	
	Interpretation	on of Plot:	Steepest Straight Line	
	Description	of Soil:	Grey brown sandy, slightly gravelly, SILT/0	CLAY

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IGSL Ltd Materials Laboratory Unit J5,M7 Business Park

Naas Co. Kildare

Test Report

Determination of Moisture Condition Value at Natural Moisture Content



Co. Kildare 045 899324		Tested in accordance	e with BS1377:Part 4:1990, clause 5.4	DELIVATED M. SCO.
	Report No		R147902	
	Contract N	0.	24665/1	
	Contract N	ame:	Monaghan Town Active Travel Developme	nt Site
	Customer:		CORA	
	BH/TP*		TP12	
	Sample No	.*	AA205178	
	Depth* (m)		0.80	
	Sample Typ	pe:	В	
	Lab Sample	e No.	A23/1720	
	Source* (if	applicable)	N/A	
	Material Ty	pe* (if applicable):	В	
	Sample Re	ceived:	09/06/23	
	Date Teste	d:	09/06/23	
	Sample Ce	rt:	Not Provided	
	Moisture Co	ontent (%):	10	
	% Particles (By dry mas		21	
	MCV:		6.7	
	Interpretation	on of Plot:	Steepest Straight Line	
	Description	of Soil:	Brown slightly sandy, gravelly, SILT/CLAY	

Results relate only to the specimen tested, in as received condition unless other	rwise noted.	Persons authorised to	approve reports
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Co. Kildare 045 899324

Test Report

Determination of Moisture Condition Value at Natural Moisture Content



Tested in accordance with BS1377:Part 4:1990, clause 5.4

Report No.

R147903

Contract No.

24665/1

Contract Name:

Monaghan Town Active Travel Development Site

Customer:

CORA

BH/TP*

TP14

Sample No.*

AA205176

Depth* (m)

1.50

Sample Type:

В

Lab Sample No.

A23/1721

Source* (if applicable)

N/A

Material Type* (if applicable):

В

Sample Received:

09/06/23

Date Tested:

09/06/23

Sample Cert:

Not Provided

Moisture Content (%):

14

% Particles > 20mm

15

(By dry mass):

MCV:

7.8

Interpretation of Plot:

Steepest Straight Line

Description of Soil:

Grey brown slightly sandy, gravelly, SILT/CLAY

Results relate only to the specimen tested, in as received condition unless otherwise noted. Opinions and interpretations are outside the scope of accreditation. Persons authorised to approve reports

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TEST REPORT Determination of California Bearing Ratio (CBR)



Tested in accordance with BS1377:Part 4:1990, clause 7

199324			l resten	iii accoluai	ice with BS137	7.Fan 4.199	o, clause /	<u> </u>
	Report No.		R147904	į.	Contract	Monaghan Tov	vn Active Travel	Development Si
	Contract No.		24665/1	24665/1		Customer		
	Date received		d 09/06/2	3	Date Tested	15/06/23	Coi	a
	BH/TP No.*		TP01		Sample No.*	AA200193	Туре:	В
	Depth*	(m)	0.70		Lab sample N	lo.	A23/1716	
	0							
	2 -							
	1.8 -							
	1.6 -							
	1.4 -							
	1.2 -							
Ŝ								
Force (kN)	1 -							
For	0.8 -							
	0.6 -							
	0.4 -		-1					
	0.2	تو						
	0 -	0.5					<u> </u>	
	C	0.5	5 1 1.5	2 2.5	3 3.5 4 Penetration (n		5.5 6 6.5	7 7.5
				_				
_	Key:			- Тор		Base		
[Descrip	otion:	Grey brow	n sandy gra	velly CLAY	*****		
Initial Condition:			on:	Unsoaked				
			tent (%):	14	Bulk Density (- '	2.01	
	Surcha % Mate			4 13	Dry Density (N	/lg/m³):	1.76	
			mpaction:		paction Method	12		
F	Test Re	esult	Тор	Base	7			
-		R %	5.7	5.6				
l		sture	14	14				
L	Conte	ent %	ł	1	1			

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045 899324

TEST REPORT Determination of California Bearing Ratio (CBR)



Tested in accordance with BS1377:Part 4:1990, clause 7

Report No. R147905 Contract Monaghan Town Active Travel Development Site Contract No. 24665/1 Customer Cora Date received 09/06/23 **Date Tested** 15/06/23 BH/TP No.* **TP03** Sample No.* AA200179 Type: В Depth* (m) 0.60 Lab sample No. A23/1717 1.2 1 8.0 Force (kN) 0.6 0.4 0.2 0 1.5 2 2.5 3 3.5 4 4.5 5.5 6 6.5 7.5 Penetration (mm) Key: Top ----- Base

Description: Grey brov	vn sandy gra	velly CLAY	
Initial Condition:	Unsoaked		
Moisture Content (%):	12	Bulk Density (Mg/m ³):	2.03
Surcharge (kg):	4	Dry Density (Mg/m ³):	1.82
% Material >20mm:	10		
Method of compaction:	Static Com	paction Method 2	

Test Result	Тор	Base
CBR %	4.5	4.8
Moisture Content %	12	11

Results relate only to the specimen tested, in as received condition unless otherwise noted

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Unit J5,M7 Business Park Naas Co.Kildare

TEST REPORT Determination of California Bearing Ratio (CBR)



045 899324 Tested in accordance with BS1377:Part 4:1990, clause 7 Report No. R147906 Contract Monaghan Town Active Travel Development Site Contract No. 24665/1 Customer Cora Date received 09/06/23 **Date Tested** 15/06/23 BH/TP No.* **TP05** Sample No.* AA200182 Type: В Depth* (m) 0.70 Lab sample No. A23/1718 1.4 1.2 1 0.8 Force (kN) 0.6 0.4 0.2 1.5 2 2.5 3.5 3 4 4.5 5 5.5 6 6.5 7.5 Penetration (mm) Key: Top ----- Base Description: Brown slightly sandy, slightly gravelly, SILT/CLAY Initial Condition: Unsoaked Moisture Content (%): 13 Bulk Density (Mg/m³): 2.08 Surcharge (kg): Dry Density (Mg/m³): 1.83 % Material >20mm: 10 Method of compaction: Static Compaction Method 2

Test Result	Тор	Base
CBR %	3.7	4.5
Moisture Content %	14	13

Results relate only to the specimen tested, in as received condition unless otherwise noted

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045 899324

TEST REPORT Determination of California Bearing Ratio (CBR)



Tested in accordance with BS1377:Part 4:1990, clause 7

Report I	No.	R147907	•	Contract	Monaghan Tov	vn Active Travel	Development
Contrac	Contract No. 24665/1			Customer		Coi	·a
Date red	ceived	09/06/2	3	Date Tested	15/06/23	Col	a
BH/TP N	No.*	TP09		Sample No.*	AA200191	Туре:	В
Depth* ((m)	0.70		Lab sample N	lo.	A23/1719	
				,			
0.8 T							
ļ							
0.6							
L							
⊋							
- 0.4							
20							
*			100				
0.2			سننتلط				
-		/					
م ا	250						
0	0.5	1 1.5	2 2.5	3 3.5 4	4.5 5 5	5.5 6 6.5	7 7.5
				Penetration (n	nm)		
Key:			- Тор		Rase		
Descript	ion	Crow brown	-				
<u> </u>			n sandy, się	htly gravelly, S	IL1/GLAY		
Initial Co			Unsoaked		3		
Moisture Surcharg			14 4	Bulk Density (Dry Density (N		2.11	
% Mater			13	DI Y DELISITY (N	ngati):	1.85	
Method			Static Con	paction Method	d 2	··········	
Test Res	sult	Тор	Base	7			
CBR	***************************************	1.8	2.1	1			

Test Result	Тор	Base		
CBR %	1.8	2.1		
Moisture Content %	14	14		

Results relate only to the specimen tested, in as received condition unless otherwise noted

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J Barrett (Quality Manager)

H Byrne (Laboratory Manager)

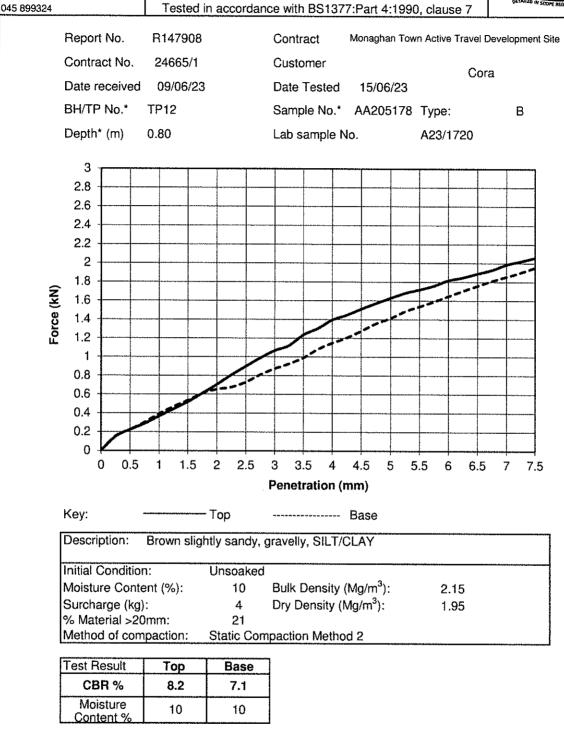
IGSL Ltd Materials Laboratory

Approved by	Date	Page No.
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TEST REPORT Determination of California Bearing Ratio (CBR)



Tested in accordance with BS1377:Part 4:1990, clause 7



Results relate only to the specimen tested, in as received condition unless otherwise noted

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Approved by Page No. Date M. Broggian 18/07/23 1 of 1

TEST REPORT Determination of California Bearing Ratio (CBR)



5 899324	Tested in accord	dance with BS1377:Part 4:1990, clause 7	£B IN SCOPE
Report No.	R147909	Contract Monaghan Town Active Travel Developm	ent Site
Contract No.	24665/1	Customer	
Date received	09/06/23	Cora Date Tested 15/06/23	
BH/TP No.*	TP14	Sample No.* AA205176 Type: B	
Depth* (m)	1.50	Lab sample No. A23/1721	
1.2			7
1			_
0.8			
Force (kN)			
0.4			
0.2			
0 0.5	1 1.5 2 2.5		
0 0.5	1 1.5 2 2.5	5 3 3.5 4 4.5 5 5.5 6 6.5 7 7 Penetration (mm)	7.5
Key:	Тор	Base	
Description:	Grey brown slightly	sandy, gravelly, SILT/CLAY	
Initial Conditio		2	_
Surcharge (kg): 4	Bulk Density (Mg/m³): 2.04 Dry Density (Mg/m³): 1.79	

Test Result	Тор	Base
CBR %	2.7	3.3
Moisture Content %	14	13

15

Static Compaction Method 2

Results relate only to the specimen tested, in as received condition unless otherwise noted

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% Material >20mm:

Method of compaction:

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Approved by	Date	Page No.
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Test Report

Dry Density/Moisture Content Relationship



Tested in accordance with BS1377:Part 4:1990

Report No.

R147910

Contract No. 24665/1

Contract Name:

Monaghan Town Active Travel Development Site

Location*:

TPOI

Sample No*.

AA200193

Depth* (m)

1.79

0.7

Material Type

В

Lab sample no.

A23/1716

Customer: CORA

2.5 Kg Rammer

Date Received:

Dry Density (Mg/m³)

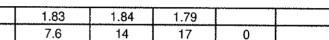
09/06/2023

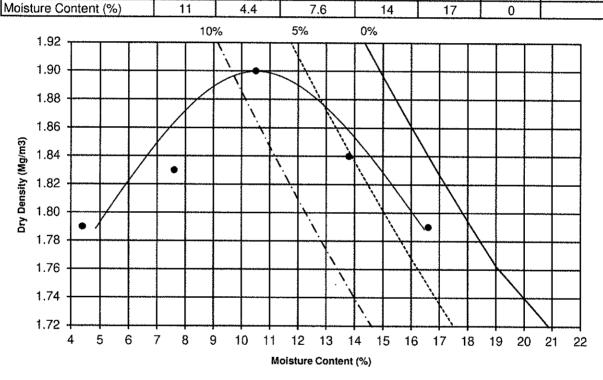
1.90

Test Method:

3.3

Date Tested: 03/07/2023 BS1377:Part 4:1990





Maximum Dry Density (Mg/m3):

1.90

Optimum Moisture Content (%):

11

Description:

Brown sandy gravelly SILT/CLAY

Sample Preparation:

Material passing 20mm

Single / Separate samples used

Particle Density (Mg/m³):

2.65

Particle Density:

Assumed

% retained on 20/37.5mm sieve:

13

R147910 TP01 0mc

Results relate only to the specimen tested, in as received condition unless otherwise noted.

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Approved by Date Page 11 Verge 6 18/07/23 1 of 1 IGSL Ltd Materials Laboratory M7 Business Park Naas

Test Report

Dry Density/Moisture Content Relationship



Tested in accordance with BS1377:Part 4:1990

Report No.

Co. Kildare

R147911

Contract No. 24665/1

Contract Name:

Monaghan Town Active Travel Development Site

Location*:

TDAS

Sample No*.

AA200179

Depth* (m)

Material Type

В

Lab sample no.

Date Received:

A23/1717 09/06/2023

Customer: CORA

Test Method:

0.7

2.5 Kg Rammer

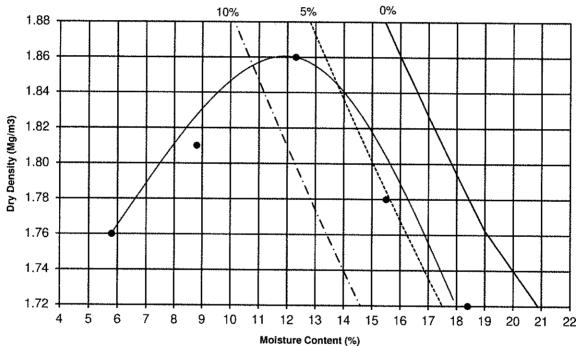
Date Tested:

03/07/2023

BS1377:Part 4:1990

3.3

Dry Density (Mg/m³)	1.86	1.76	1.81	1.78	1.72		
Moisture Content (%)	12	5.8	8.8	16	18	0	



Maximum Dry Density (Mg/m3):

1.86

Optimum Moisture Content (%):

12

Description:

Brown sandy gravelly SILT/CLAY

Sample Preparation:

Material passing 20mm

Single / Separate samples used

Particle Density (Mg/m³):

2.65

Particle Density:

Assumed

% retained on 20/37.5mm sieve:

10

R147911 TP03 @ 0.6 0mc

Results relate only to the specimen tested, in as received condition unless otherwise noted. Opinions and interpretations are outside the scope of accreditation.

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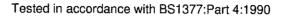
Approved by	Date	Page
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IGSL Ltd Materials Laboratory M7 Business Park

Naas Co. Kildare

Test Report

Dry Density/Moisture Content Relationship





Report No.

R147912

Contract No. 24665/1

Contract Name:

Monaghan Town Active Travel Development Site

Location*:

Sample No*.

AA200182

Depth* (m)

0.7

Test Method:

Material Type

В

Lab sample no. Date Received:

A23/1718 09/06/2023 Customer: CORA

2.5 Kg Rammer

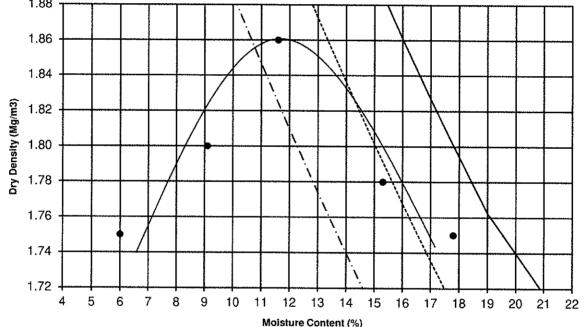
Date Tested:

03/07/2023

BS1377:Part 4:1990

3.3

Dry Density (Mg/m²)	1.86	1.75	1.80	1.78	1.75		
Moisture Content (%)	12	6.0	9.1	15	18	0	
1.88		10%	5%	0%			
1.00		l i	\ \				1
1,86		لذلل					
.,,,,	1 1	1 1 1/2	-1-11	1 1 1		1 1	



Maximum Dry Density (Mg/m3):

1.86

Optimum Moisture Content (%):

12

Description:

Brown slightly sandy, slightly gravelly, SILT/CLAY

Sample Preparation:

Material passing 20mm

Single / Separate samples used

Particle Density (Mg/m³):

2.65

Particle Density:

Assumed

% retained on 20/37.5mm sieve:

10

Results relate only to the specimen tested, in as received condition unless otherwise noted.

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IGSL Ltd Materials Laboratory M7 Business Park

Naas Co. Kildare

Test Report

Dry Density/Moisture Content Relationship



Tested in accordance with BS1377:Part 4:1990

Report No.

R147913

Contract No. 24665/1

Contract Name:

Monaghan Town Active Travel Development Site

Location*:

Sample No*.

AA200191

Depth* (m)

0.7 Material Type В

Lab sample no.

A23/1719

Customer: CORA

Test Method:

2.5 Kg Rammer

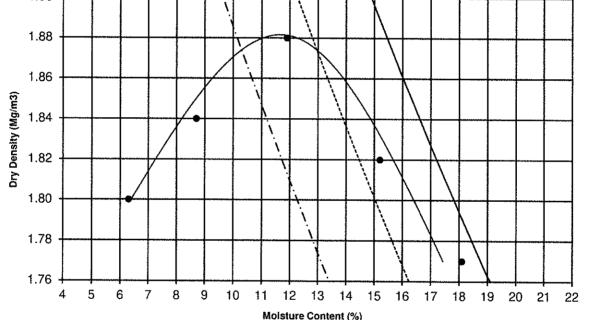
Date Received: Date Tested:

09/06/2023 03/07/2023

BS1377:Part 4:1990

3.3

Dry Density (Mg/m ³)	1.88	1.80	1.84	1.82	1.77		
Moisture Content (%)	12	6.3	8.7	15	18	0	
1.90		10%	5%	0%			
1.90		, i		$\Box \setminus \Box$			
1.88							



Maximum Dry Density (Mg/m³):

1.88

Optimum Moisture Content (%):

12

Description:

Grey brown sandy, slightly gravelly, SILT/CLAY

Sample Preparation:

Material passing 20mm

Single / Separate samples used

Particle Density (Mg/m3):

2.65

Particle Density:

Assumed

% retained on 20/37.5mm sieve:

13

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Test Report

Dry Density/Moisture Content Relationship



Tested in accordance with BS1377:Part 4:1990

Report No.

R147914

Contract No. 24665/1

Contract Name:

Monaghan Town Active Travel Development Site

Location*:

TO12

Sample No*.

AA205179

Depth* (m)

0.8 Material Type

В

Lab sample no.

A23/1720

Customer: CORA

Date Received:

09/06/2023

Test Method:

2.5 Kg Rammer

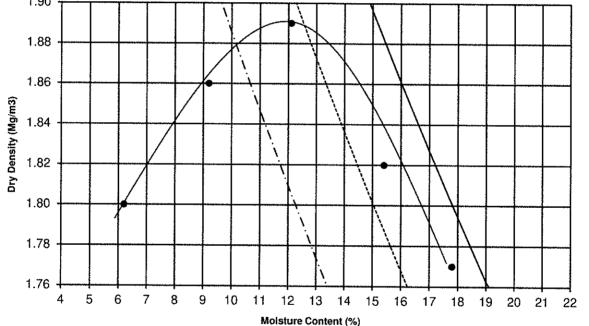
Date Tested:

03/07/2023

BS1377:Part 4:1990

3.3

Dry Density (Mg/m°)	1.89	1.80	1.86	1.82	1.77		1
Moisture Content (%)	12	6.2	9.2	15	18	0	
1.90		10%	5%	0%			
1.00				\prod			
1.88							***
	1 1	ואו	1 M X	1 1 7 7			



Maximum Dry Density (Mg/m3):

1.89

Optimum Moisture Content (%):

12

Description:

Brown slightly sandy, gravelly, SILT/CLAY

Sample Preparation:

Material passing 20mm

Single / Separate samples used

Particle Density (Mg/m³):

2.65

Particle Density:

Assumed

% retained on 20/37.5mm sieve:

19

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IGSL Ltd Materials Laboratory M7 Business Park Naas

Co. Kildare

Test Report

Dry Density/Moisture Content Relationship



Tested in accordance with BS1377:Part 4:1990

Report No.

R147915

Contract No. 24665/1

Contract Name:

Monaghan Town Active Travel Development Site

Location*:

Sample No*.

AA205176

Depth* (m)

1.5 Material Type В

Lab sample no. Date Received:

A23/1721

Customer: CORA

09/06/2023

Test Method:

2.5 Kg Rammer

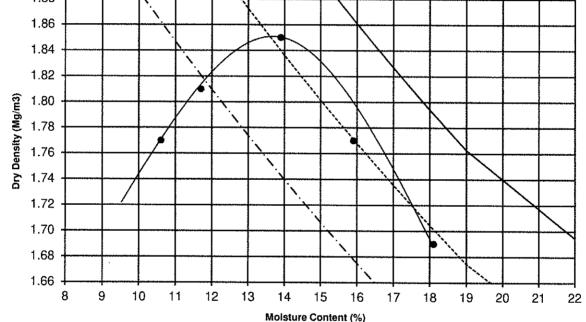
Date Tested:

03/07/2023

BS1377:Part 4:1990

3.3

Dry Density (Mg/m	า")	1.77	1.69	1.85	1.81	1.77		
Moisture Content	(%)	11	18	14	12	16	0	***************************************
1.00	10	1%	5%	0%	•			
1.88 T		·. T	N N			T T		
1.86	 	` .	 	ļ	+			
1.84								
1 92				NN				



Maximum Dry Density (Mg/m³):

1.85

Optimum Moisture Content (%):

14

Description:

Grey brown slightly sandy, gravelly, SILT/CLAY

Sample Preparation:

Material passing 20mm

Single / Separate samples used

Particle Density (Mg/m³):

2.65

Particle Density:

Assumed

% retained on 20/37.5mm sieve:

14

Results relate only to the specimen tested, in as received condition unless otherwise noted.

Persons authorised to approve reports

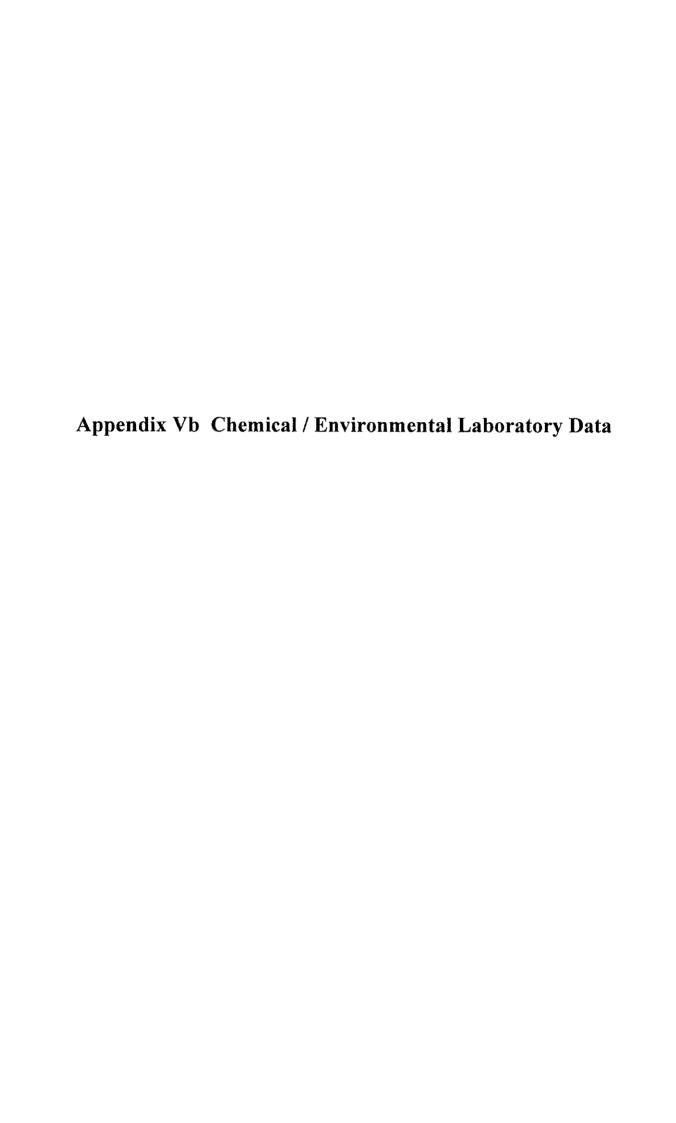
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🗱 eurofins

Chemtest

Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL

Tel: 01638 606070 Email: info@chemtest.com

Final Report

Report No.:

23-19446-1

Initial Date of Issue:

19-Jun-2023

Re-Issue Details:

Client

IGSL

Client Address:

M7 Business Park

Naas

County Kildare

ireland

Contact(s):

Darren Keogh

Project

24665 / 1 Monaghan Town Active

Travel Development Site(CORA)

Quotation No.:

Q20-19951

Date Received:

08-Jun-2023

Order No.:

Date Instructed:

08-Jun-2023

No. of Samples:

18

7

Turnaround (Wkdays):

Results Due:

16-Jun-2023

Date Approved:

19-Jun-2023

Approved By:

Details:

Stuart Henderson, Technical

Manager

Results - Leachate

Site(CORA)											
Client: IGSL			Che	mtest Jc	b No.:	Chemtest Job No.: 23-19446	23-19446	23-19446	23-19446	23-19446	23-19446
Quotation No.: Q20-19951		Ĭ	Shemte	Chemtest Sample ID.:	ole ID.:	1653387	1653389	1653392	1653395	1653398	1653402
Order No.:			Clie	Client Sample Ref.:	le Ref.:	AA192931	AA192934	AA171710	AA200184	AA200195	AA205173
			Se	Sample Location:	cation:	BH01	BH03	8H07	TP04	TP08	TP13
				Sample Type:	3 Type:	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
				Top Depth (m):	ith (m):	0.50	0.50	0.80	0.50	08.0	0.60
Determinand	Accred.		Type	SOP Type Units	GOT						
pH	n	1010	10:1		N/A	8.4	8.1	8.8	6.8	8.9	8.2
Ammonium	n	1220	10:1	l/6m	0.050	0.22	0.13	0.11	0.11	0.12	0.15
Ammonium	Z	1220	10:1	mg/kg	0.10	2.5	1,4	1.5	1.6	1.7	1.6
Boron (Dissolved)	n	1455	10:1	mg/kg	0.01	< 0.01	< 0.01	< 0.01	< 0.01	< 0.01	< 0.01
Benzoliffuoranthene	z	1800	10.1	l/bn	ug/ 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010

Project: 24665 / 1 Monaghan Town Active Travel Development

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Client: IGSI		HU.	Chemiest Joh No.	Ich Na	SANOTO	27 40446	SALOF CO	2×10× cc	SERVE CO	OFFICE CO.	0770700	20000	SHOW AND COMPANY OF THE PARTY O
Quotation No.: Q20-19951		Chem	Chemtest Sample ID.	nole ID.:	1653387	1653388	1653389	1653390	1653301	1653302	1653303	1653304	23-19446
Order No.:		ð	Client Sample Ref	ple Ref.	AA192931	AA197802	AA192934	AA192939	AA192947	AA171710	A 200193	AA200179	1033333
		0.	Sample Location	ocation.	BH01	RHO	BHO3	BHOAA	BELOG	2000	1504	77000	10070
			Same	Sample Type		7015	200	100	200	1010	100	5011	100
			Too D.	Ton Denth (m)		100	0 50	100		200	SOIL	301	301.
			o do C	(pri) (my)	0.30	20.1	0.00	20.7	00.1	0.00	0.70	na:n	0.50
			L	- 1	DURHAM		DUKHAM			NEW-ASB			NEW-ASB
Determinand	Accred.	SOP	Units	20		100							0.450
ACM Type	5	2192		ΥN	-		,			-			1
Asbestos Identification	⊃	2192		N/A	No Asbestos Detected		No Asbestos	-		No Asbestos			No Asbestos
Moisture	z	2030	%	0.020	25	7.0	Delected	-	**	Delected	Ç	4,	Defected
CH (25:1)	Z	2010		4.0		14186		IAIOE	14195				2
Boron (Hot Water Soluble)	: -	2120	ma/ka	0.40	[A] < 0.40	0.0	IA1 < 0.40	(A) 6:3	(M)	[A] < 0.40			[A] < 0.40
Magnesium (Water Soluble)	Z	2120		0 0 10		[A] < 0.010		141 < 0.040	101 < 0.040	21.0			0.40
Sulphate (2:1 Water Soluble) as SO4	: 0	2120))	0.010		[A] < 0.010		[A] < 0.010	[A] < 0.010				
Total Sulphur)	2175	%	0.010		[A] 0.032		[A] 0.015	[A] 0.019				
Sulphur (Elemental)	ח	2180	Ε	1.0	[A] 2.7	,	[A] 3.2			[A] 2.3			[A] 2.2
Chloride (Water Soluble)	₽	2220		0.010		[A] 0.11		[A] < 0.010	(A) < 0.010				
Nitrate (Water Soluble)	z	2220	1/6	0.010		< 0.010		< 0.010	< 0.010				
Cyanide (Total)	٦	2300	mg/kg	0.50	[A] < 0.50		[A] < 0.50			[A] < 0.50			[A] < 0.50
Sulphide (Easily Liberatable)	z	2325	mg/kg	0.50	[A] 14		[A] 4.7			[A12.5			[A] 4.6
Ammonium (Water Soluble)	n	2220	1/6	0.01		< 0.01		< 0.01	< 0.01				
Sulphate (Acid Soluble)	n	2430	%	0.010	[A] 0.024	[A] 0.057	[A] 0.018	[A] 0.029	[A] 0.031	[A] 0.014			[A] 0.053
Arsenic	n	2455	mg/kg	0.5	3.3		3.6			3.3			5.1
Barium	⊃	2455	mg/kg	0	28		39			88			34
Cadmium	n	2455	mg/kg	0.10	< 0.10		< 0.10			< 0.10			< 0.10
Chromium	n	2455	mg/kg	0.5	15		16			15			18
Molybdenum	n	2455	mg/kg	0.5	< 0.5		< 0.5			< 0.5			< 0.5
Antimony	z	2455	ga/kgm	2.0	< 2.0		< 2.0			< 2.0			< 2.0
Copper	n	2455	mg/kg	0.50	10		13			11			13
Mercury	ב		mg/kg	0.05	< 0.05		< 0.05			0.05			0.07
Nickel	ר		mg/kg	0.50	23		31			21			28
Lead	ם		mg/kg	0.50	8.1		12			14			20
Selenium	ב		mg/kg	0.25	< 0.25		< 0.25			< 0.25			< 0.25
Zinc	Ð	-	mg/kg	0.50	29		88			37			62
Chromium (Trivalent)	z	-	mg/kg	1.0	15		16			15			18
Chromium (Hexavalent)	z	2490	mg/kg	0.50	< 0.50		< 0.50			< 0.50			< 0.50
Organic Matter	ח	2625	%	0.40							[A] 2.5	[A] 1.8	
Mineral Oil (TPH Calculation)	Z	2670	mg/kg	10	< 10		< 10			< 10			< 10
Aliphatic TPH > C5-C6	Z	2680	mg/kg	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Aliphatic TPH >C6-C8	z	2680	mg/kg	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Aliphatic TPH >C8-C10	z		mg/kg	1:0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Aliphatic TPH >C10-C12	z	2680	mg/kg	0.1	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Aliphatic TPH >C12-C16	z	2680	mg/kg	0.	[A] < 1.0		[A] < 1.0			[A] < 1.0			(A) < 1.0
Aliphatic TPH >C16-C21	z	2680	mg/kg ∣	0.	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0

Project: 24665 / 1 Monaghan Town Active Travel Development

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Client: IGSL		3	1emtes	Chemtest Job No.:	.: 23-19446	23-19446	23-19446	23-19446	23-19446	23-19446	23-1944F	22.10446	22.10/46
Quotation No.: Q20-19951		Chen	ntest Sa	Chemtest Sample ID.:		1653388	1653389	1653390	1653391	1653392	1653393	1653394	1653395
Order No.:	-	Ō	lient Sa	Client Sample Ref.:	AA192931	AA197802	AA192934	AA192939	AA192947	AA171710	AA200193	AA200179	AA200184
			Sample	Sample Location:	1: 8H01	BH02	BH03	BH04A	BH05	BH07	TP01	TP03	TPDA
			San	Sample Type:	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOII	ilos
			Top	Top Depth (m):	0.50	1.00	0.50	1.00	1.00	0.80	0.70	0,60	0.50
			Asb	Asbestos Lab:	DURHAM		DURHAM			NEW-ASB			NFW-ASB
Determinand	Accred.	SOP		100 S									200
Aliphatic TPH >C21-C35	N	2680	mg/kg	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Aliphatic TPH >C35-C44	z	2680	_	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Total Aliphatic Hydrocarbons	z	2680	mg/kg	3 5.0	[A] < 5.0		[A] < 5.0			[A] < 5.0			[A] < 5.0
Aromatic TPH >C5-C7	z	2680	mg/kg		[A] < 1.0		[A] < 1.0			A] < 1.0			[A] < 1.0
Aromatic TPH >C7-C8	z	2680		L	[A] < 1.0		[A] < 1.0			[A] < 1.0			141<10
Aromatic TPH >C8-C10	z	2680			[A] < 1.0		[A] < 1.0			A < 1.0			[A] < 1.0
Aromatic TPH >C10-C12	z	2680	mg/kg	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Aromatic TPH >C12-C16	z	2680	mg/kg	1.0	[A] < 1.0		A < 1.0			[A] < 1.0			[A] < 1.0
Aromatic TPH >C16-C21	z	2680	mg/kg	1.0	[A] < 1.0		[A] < 1.0			IA] < 1.0			[A] < 1.0
Aromatic TPH >C21-C35	Z	2680		1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Aromatic TPH >C35-C44	z	2680	mg/kg	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Total Aromatic Hydrocarbons	N	2680		5.0	[A] < 5.0		[A] < 5.0			[A] < 5.0			[A] < 5.0
Total Petroleum Hydrocarbons	z	2680	mg/kg	10.0	[A] < 10		[A] < 10			[A] < 10			[A] < 10
Велгепе	Λ	2760	µg/kg	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Toluene	n	2760	µg/kg	1.0	[A] < 1.0		(A) < 1.0			A] < 1.0			[A] < 1.0
Ethylbenzene	n	2760	µg/kg	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
m & p-Xylene	n	2760	µg/kg	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
o-Xylene	ב	2760	µg/kg	1.0	[A] < 1.0		[A] < 1.0			[A] < 1.0			[A] < 1.0
Methyl Tert-Butyl Ether	⊃	2760			_		[A] < 1.0			[A] < 1.0			[A] < 1.0
Naphthalene	z	2800	mg/kg		\Box		[A] < 0.010			[A] < 0.010			[A] < 0.010
Acenaphthylene	z	2800	mg/kg				[A] < 0.010			[A] < 0.010			[A] < 0.010
Acenaphthene	z	2800	mg/kg				[A] < 0.010			[A] < 0.010			[A] < 0.010
Fluorene	z	2800	mg/kg	_	[A] < 0.010		[A] < 0.010			[A] < 0.010			[A] < 0.010
Phenanthrene	z	2800	mg/kg	0.010	[A] < 0.010		[A] < 0.010			[A] < 0.010			[A] < 0.010
Anthracene	z	2800	mg/kg		₹.		[A] < 0.010			[A] < 0.010			[A] < 0.010
Fluoranthene	z	2800	mg/kg		_		[A] < 0.010			[A] < 0.010			[A] < 0.010
Pyrene	z	2800	mg/kg	_	[A] 0.18		[A] < 0.010			[A] < 0.010			[A] < 0.010
Benzo[a]anthracene	z	2800	mg/kg	-	_		[A] < 0.010			[A] < 0.010			[A] < 0.010
Chrysene	z	2800	mg/kg	-			[A] < 0.010			[A] < 0.010			[A] < 0.010
Benzo[b]fluoranthene	z	2800	mg/kg		_		[A] < 0.010			[A] < 0.010			[A] < 0.010
Benzo(k)fluoranthene	z	2800			\neg		[A] < 0.010			[A] < 0.010			[A] < 0.010
Benzo(a)pyrene	z	2800	mg/kg				[A] < 0.010			[A] < 0.010			[A] < 0.010
Indeno(1,2,3-c,d)Pyrene	z	2800	mg/kg	_	_		[A] < 0.010			[A] < 0.010			[A] < 0.010
Dibenz(a,h)Anthracene	z	2800	mg/kg	0.010			[A] < 0.010			[A] < 0.010			[A] < 0.010
Benzo[g,h,i]perylene	Z	2800			[A] < 0.010		[A] < 0.010			[A] < 0.010			[A] < 0.010
Coronene	z	2800	mg/kg	1	[A] < 0.010		[A] < 0.010			[A] < 0.010			[A] < 0.010
Total Of 17 PAH's	z	2800	mg/kg				[A] < 0.20			[A] < 0.20			[A] < 0.20
PCB 28	2	2815	mg/kg	0.0010	[A] < 0.0010		[A] < 0.0010			[A] < 0.0010			[A] < 0.0010

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Client: IGSL		Chemi	Chemtest Job No.:	: 23-19446	23-19446	23-19446	23-19446	23-19446	23-19446	23-19446	23-19446	22-19446
Quotation No.: Q20-19951		Chemtest	Chemtest Sample ID.:	: 1653387	1653388	1653389	1653390	1653391	Т	1653393		1653395
Order No.:		Client	Client Sample Ref :	: AA192931	AA197802	AA192934	AA192939	AA192947	AA171710	AA200193	AA200179	AA200184
		Sam	Sample Location:	: BH01	BH02	BH03	BH04A	BH05	BH07	TP01	1	TP04
		נט	Sample Type:	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
		7	Top Depth (m):	0.50	1.00	0.50	1.00	1.00	0.80	0.70	0.60	0.50
		∢	Asbestos Lab:	: DURHAM		DURHAM			NEW-ASB			NEW-ASB
Determinand	Accred.	SOP	Accred. SOP Units LOD									
PCB 52	z	2815 mc	2815 mg/kg 0.0010	[A] < 0.0010		[A] < 0.0010			[A] < 0.0010			[A] < 0.0010
PCB 90+101	z	2815 mg	mg/kg 0.0010	[A] < 0.0010		[A] < 0.0010			[A] < 0.0010			[A] < 0.0010
PCB 118	2	2815 mg	2815 mg/kg 0.0010	[A] < 0.0010		[A] < 0.0010			[A] < 0.0010			[A] < 0.0010
PCB 153	z	2815 mg	2815 mg/kg 0.0010	[A] < 0.0010		[A] < 0.0010			[A] < 0.0010			[A] < 0.0010
PCB 138	z	2815 mg	2815 mg/kg 0.0010	[A] < 0.0010		[A] < 0.0010			[A] < 0.0010			[A] < 0.0010
PCB 180	z	2815 mg	mg/kg 0.0010	[A] < 0.0010		[A] < 0,0010			[A] < 0.0010			[A] < 0.0010
Total PCBs (7 congeners)	z	2815 mg	2815 mg/kg 0.0010	[A] < 0.0010		[A] < 0.0010			[A] < 0.0010			[A] < 0.0010
Total Phenols	⊃	2920 ma/ka	1/kg 0.10	< 0.10		< 0.10			< 0.10			V 0 10

Project: 24665 / 1 Monaghan Town Active Travel Development Site(CORA)

Client Sample ID.: 1653396 1653397		Chemtest Job	st.Job No:	24-1044R	32.104AR	22,104.18	72.10AAR	2410 - CC	STRUK CO 1	OFFUR CO	OF FUE CO	2770700
Cilent Sample Ref. AA200182 AA200182 Sample Location: TP05 TP07 Sample Location: TP05 TP07 Sample Location: TP05 TP07 Accred. SOP Units LOD U	້ວ	mtest :	Sample ID	91	1653397	1653398	1653399	1653400	1653401	1653402	1653403	1653404
Sample Location: TP05 TP07		Client S	ample Ref		AA200188	AA200195	AA200196	AA200191	AA205178	AA205173	AA205175	AA205176
Accrect Sample Type: SOIL SOIL		Samp	le Location		TP07	TP08	TP08	TP09	TP12	TP13	TP 14	TP14
Accred SOP Units LOD C.70 C.50		Ϊ́	Imple Type	l	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
Accred SOP Units LOD U 2192 N/A 12 U 2192 N/A 12 N 2020 % 0.020 13 12 I 2120 m/A 0.020 13 12 Ie) N 2120 m/A (A) 0.010 [A] 0.010 U 2120 m/A 0.010 [A] 0.010 U 2120 m/A 0.010 [A] 0.010 U 2120 m/A 0.010 [A] 0.034 U 2220 m/A 0.010 [A] 0.034 U 2456 m/A 0.01 [A] 0.064 U 2456 m/A 0.0 0.0		 ਹੁ	Depth (m)		0.50	0.80	1.80	0.70	0.80	09'0	0.70	1.50
Accred. SDP Units LOD U 2192 N/A 12 U 2192 N/A 12 N 2030 % 0.020 13 12 N 2010 % 0.020 13 12 (a) 2120 mg/kg 0.40 [A] 8.0 (A] 8.0 (b) 2120 mg/kg 0.010 [A] 0.034 (A] 0.034 (b) 2120 g/l 0.010 [A] 0.024 (A] 0.034 (c) 1 2175 g/l 0.010 [A] 0.034 (c) 0 2120 g/l 0.010 [A] 0.034 (c) 0 2120 g/l 0.010 [A] 0.034 (c) 0 2120 g/l 0.010 [A] 0.034 (d) 0 2120 g/l 0.010 [A] 0.034 (d) 0 2220 g/l 0.010 [A] 0.041 (d) 0 <		As	bestos Lat	5:		COVENTRY				NEW-ASB		
U 2192 N/A NA I 2192 N/A 12 I 2030 % 0.020 13 12 I 2030 % 0.020 13 12 I 2120 g/I 0.010 I/A 8.0 I/A 8.0 I 2120 g/I 0.010 I/A 8.0 I/A 8.0 U 2120 g/I 0.010 I/A 8.0 I/A 8.0 U 2120 g/I 0.010 I/A 8.0 I/A 8.0 U 2120 g/I 0.010 I/A 8.0 I/A 8.0 D 2120 g/I 0.010 I/A 8.0 I/A 8.0 D 2120 g/I 0.010 I/A 8.0 I/A 8.0 D 2220 g/I 0.010 I/A 9.0 I/A 9.0 D 2455 mg/Kg 0.50 I/A 9.0 I/A 9.0 D 0 2455 mg/Kg 0.5 I/A 9.0 I/A 9.0 <th>70.</th> <th>CEES</th> <th>ts LOD</th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th></th> <th>9511107 WW. 10010</th>	70.	CEES	ts LOD									9511107 WW. 10010
N 2192 N/A		92	N/A			ť				-		
N 2030 % 0.020 13 12		32	Υ/N			No Asbestos Detected				No Asbestos Detected		
Each Both Color Each		L	t	_	12	10	7.8	11	11	19	12	13
1	Г	9	4.0		[A] 8.0		[A] 8.6				[A] 7.8	
ble) as SO4 U 2120 g/l 0.010 [A] < 0.010 [A] < 0.010 buble) as SO4 U 2120 g/l 0.010 [A] < 0.010 clip as SO4 U 2120 g/l 0.010 [A] < 0.010 clip as SO4 U 2120 g/l 0.010 [A] < 0.017 clip able) N 2220 g/l 0.010 clip able) N 2230 mg/kg 0.50 clip able) U 2455 mg/kg 0.50 clip able) N 2450 mg/kg 1.0 clip able) N 2450 mg/kg 1.0 clip able) N 2650 mg/kg 1.0 clip able) N 26	_		┿			[A] < 0,40				[A] 0.52	22.6	
Decision	2	L	t		[A] < 0.010		[A] < 0.010				[A] < 0.010	
U 2175 % 0.010 [A] 0.034 U 2180 mg/kg 1.0 [A] 0.038 N 2220 g/l 0.010 [A] 0.017 able) N 2220 g/l 0.010 0.017 ble) U 2220 g/l 0.010 0.017 ble) U 2220 g/l 0.010 0.017 ble) U 2220 g/l 0.010 0.017 U 2425 mg/kg 0.50 0.01 0.010 U 2455 mg/kg 0.05 0.05 0.00 U 2455 mg/kg 0.50 0.00 0.00 0.00 U 2455 mg/kg 0.50 0.00	n	L	 		[A] < 0.010		[A] < 0.010				[A] 0.24	
U 2180 mg/kg 1.0 D 2220 g/I 0.010 [A] 0.017 Bole) N 2220 g/I 0.010 0.017 ble) N 2325 mg/kg 0.50 0.017 0.017 ble) N 2326 mg/kg 0.50 0.001 (A] 0.065 ble) U 2430 mg/kg 0.50 0.001 (A] 0.065 U 2455 mg/kg 0.10 (A] 0.065 0.00 <td>Г</td> <td></td> <td></td> <td></td> <td>[A] 0.034</td> <td></td> <td>[A] 0.027</td> <td></td> <td></td> <td></td> <td>IA10.077</td> <td></td>	Г				[A] 0.034		[A] 0.027				IA10.077	
(b) (b) (b) (b) (b) (b) (c) (c) <td>n</td> <td></td> <td>-</td> <td></td> <td></td> <td>[A] 2.6</td> <td></td> <td></td> <td></td> <td>[A] 3.8</td> <td></td> <td></td>	n		-			[A] 2.6				[A] 3.8		
N 2220 gfl 0.010 0.017	ח		_		[A] 0.028		[A] < 0.010				[A] < 0.010	
ble) N 2325 mg/kg 0.50		L	-		0.017		< 0.010				< 0.010	
ble) N 2325 mg/kg 0.50		_	0			[A] < 0.50				[A] < 0.50		
ble) U 2220 gyl 0.010 < 0.011 U 2430 % 0.010 [A] 0.065 U 2455 mg/kg 0.5 U 2455 mg/kg 0.10 U 2455 mg/kg 0.10 U 2455 mg/kg 0.5 U 2455 mg/kg 0.50 U 2450 U 2450 mg/kg 0.50 U 2450 U 24	N	_	0			[A] 3.3				[A] 3.3		:
U 2430 % 0.010 [A] 0.065 U 2455 mg/kg 0.5 (1)	n	_	_		< 0.01		< 0.01				< 0.01	
U 2455 mg/kg 0.5 U 2455 mg/kg 0.0 U 2455 mg/kg 0.10 U 2455 mg/kg 0.5 U 2455 mg/kg 0.0 U 2455 mg/kg 0.0 U 2455 mg/kg 0.50 N 2450 mg/kg 0.50 N 2680 mg/kg 0.0 N 2680 mg/kg 1.0 N 2680 mg/kg 1.0 N 2680 mg/kg 1.0 N 2680 mg/kg 1.0 N 2680 <					[A] 0.065	[A] 0.033	[A] 0.045			[A] 0.040	[A] 0.075	
U 2455 mg/kg 0 U 2455 mg/kg 0.10 U 2455 mg/kg 0.5 U 2455 mg/kg 0.0 N 2455 mg/kg 0.0 U 2455 mg/kg 0.50 N 2455 mg/kg 0.50 N 2455 mg/kg 0.50 N 2450 mg/kg 0.50 N 2680 mg/kg 1.0 N 2680 <t< td=""><td></td><td></td><td></td><td></td><td></td><td>4.5</td><td></td><td></td><td></td><td>5.3</td><td></td><td></td></t<>						4.5				5.3		
U 2455 mg/kg 0.10 U 2455 mg/kg 0.5 U 2455 mg/kg 0.0 N 2455 mg/kg 0.0 U 2455 mg/kg 0.0 U 2455 mg/kg 0.50 N 2455 mg/kg 0.50 N 2456 mg/kg 0.50 N 2650 mg/kg 1.0 N 2680 mg/kg 1.0 N <td></td> <td>_</td> <td></td> <td></td> <td></td> <td>48</td> <td></td> <td></td> <td></td> <td>43</td> <td></td> <td></td>		_				48				43		
U 2455 mg/kg 0.5 U 2455 mg/kg 0.0 N 2455 mg/kg 0.0 U 2455 mg/kg 0.0 U 2455 mg/kg 0.50 N 2490 mg/kg 0.50 N 260 mg/kg 1.0 N 2680 mg/kg 1.0 N		-				< 0.10				< 0.10		
U 2455 mg/kg 0.5 N 2455 mg/kg 2.0 U 2455 mg/kg 0.50 N 2450 mg/kg 0.50 N 2626 % 0.40 [A] 1.1 N 2680 mg/kg 1.0 N <td< td=""><td></td><td></td><td></td><td></td><td></td><td>27</td><td></td><td></td><td></td><td>28</td><td></td><td></td></td<>						27				28		
N 2455 mg/kg 2.0 U 2455 mg/kg 0.50 N 2490 mg/kg 1.0 N 2680 mg/kg 1.0 N 268			Щ			< 0.5				< 0.5		
U 2455 mg/kg 0.50 U 2455 mg/kg 0.05 U 2455 mg/kg 0.50 U 2455 mg/kg 0.50 U 2455 mg/kg 0.25 U 2455 mg/kg 0.50 N 2490 mg/kg 1.0 N 2626 % 0.40 [A] 1.1 N 2680 mg/kg 1.0 N N 2680 mg/kg			ᆫ			< 2.0				< 2.0		
U 2455 mg/kg 0.05 U 2455 mg/kg 0.50 U 2455 mg/kg 0.50 U 2455 mg/kg 0.50 U 2455 mg/kg 0.50 N 2490 mg/kg 1.0 N 2626 % 0.40 [A] 1.1 U 2626 % 0.40 [A] 1.1 N 2680 mg/kg 1.0 N			L.,			21				18		
U 2455 mg/kg 0.50 U 2455 mg/kg 0.50 U 2455 mg/kg 0.25 U 2455 mg/kg 0.50 N 2490 mg/kg 1.0 N 2490 mg/kg 1.0 U 2625 % 0.40 [A] 1.1 U 2626 mg/kg 1.0 1.0 N 2680 mg/kg 1.0 1.0						< 0.05				< 0.05		
titon) N 2680 mg/kg 1.0 mg						43				37		
U 2455 mg/kg 0.25 U 2455 mg/kg 0.50 N 2490 mg/kg 1.0 N 2490 mg/kg 1.0 U 2625 % 0.40 [A] 1.1 U 2626 mg/kg 1.0 N 2680 mg/kg 1.0						16				13		
U 2455 mg/kg 0.50			$\overline{}$			< 0.25				< 0.25		
N 2490 mg/kg 1.0						48				41		
N 2490 mg/kg 0.50 (tion) N 2625 % 0.40 [A] 1.1 N 2670 mg/kg 10 (A) 1.1 N 2680 mg/kg 1.0 (A) 1.0						27				28		
titon) N 2625 % 0.40 [A] 1.1 N 2680 mg/kg 1.0						< 0.50				< 0.50		
M 2670 mg/kg mg/kg 10 N 2680 mg/kg 1.0	⊃		0.40	Щ				[A] 1.1	[A] 1.0		-	A) 1.1
N 2680 mg/kg 1.0	Z					< 10				< 10		
N 2680 mg/kg 1.0			Ψ.			[A] < 1.0				[A] < 1.0		
N 2680 mg/kg 1.0 N 2680 mg/kg 1.0 N 2680 mg/kg 1.0			7			[A] < 1.0				[A] < 1.0		
N 2680 mg/kg 1.0 N 2680 mg/kg 1.0	Z		_			[A] < 1.0			••••	[A] < 1.0		
N 2680 mg/kg 1.0	Z					[A] < 1.0				[A] < 1.0		
	z	0 mg/				[A] < 1.0				[A] < 1.0		
2680 mg/kg 1.0	٦	0 mg/l				[A] < 1.0				[A] < 1.0		

Project: 24665 / 1 Monaghan Town Active Travel Development_ Site(CORA)

Client: IGSL		Ö	Chemtest Job	t Job No.:	23-19446	23-1944R	23-19446	23-1044R	23.1044E	23.10JAE	23.10448	22 404AR	29 40446
Quotation No.: Q20-19951		Chen	Chemtest Sampl	ample ID.:	17	1653397	1653398	1653399	1653400	1653401	1653402	1653403	1653404
Order No.:		Ö	lient Sa	Client Sample Ref.:	AA200182	AA200188	AA200195	AA200196	AA200191	AA205178	AA205173	AA205175	AA205176
			Sample	Sample Location:	TP05	TP07	TP08	TP08	TP09	TP12	TP13	TP14	TP14
			San	Sample Type:		SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
			Top	Top Depth (m):	0.70	0.50	08'0	1.80	0.70	0.80	09:0	0.70	1.50
			Asb	estos Lab:			COVENTRY				NEW-ASB		
Determinand	Accred.	SOP	3500	Units LOD									
Aliphatic TPH >C21-C35	z	2680	mg/kg	Ш			[A] < 1.0				[A] < 1.0		
Aliphatic TPH >C35-C44	z	2680	mg/kg	3 1.0			[A] < 1.0				[A] < 1.0		
Total Aliphatic Hydrocarbons	Z	2680	mg/kg	L			[A] < 5.0				[A] < 5.0		
Aromatic TPH >C5-C7	Z	2680	mg/kg	1.0			[A] < 1.0				[A] < 1.0		
Aromatic TPH >C7-C8	N	2680	mg/kg	1.0			[A] < 1.0				[A] < 1.0		
Aromatic TPH >C8-C10	z	2680	mg/kg	1.0			[A] < 1.0				[A] < 1.0		
Aromatic TPH >C10-C12	Z	2680	mg/kg	1.0			[A] < 1.0				[A] < 1.0		
Aromatic TPH >C12-C16	Z	2680	mg/kg				[A] < 1.0				[A] < 1.0		
Aromatic TPH >C16-C21	z	2680					[A] < 1.0				[A] < 1.0		
Aromatic TPH >C21-C35	z	2680		1.0			[A] < 1.0				[A] < 1.0		
Aromatic TPH >C35-C44	z	2680		L			[A] < 1.0				[A] < 1.0		
Total Aromatic Hydrocarbons	z	2680		L			[A] < 5.0				[A] < 5.0		
Total Petroleum Hydrocarbons	z	2680		Ľ			[A] < 10				[A] < 10		
Benzene	5	2760	µg/kg	_			[A] < 1.0				[A] < 1.0		
Toluene	Ð	2760					[A] < 1.0				[A] < 1.0		
Ethylbenzene	ם	2760		_			[A] < 1.0				[A] < 1.0		
m & p-Xylene	Þ	2760	µg/kg	1.0			[A] < 1.0				[A] < 1.0		
o-Xylene	Ω	2760	l µg/kg	1.0			[A] < 1.0				[A] < 1.0		
Methyl Tert-Butyl Ether	n	2760		_			[A] < 1.0				[A] < 1.0		
Naphthalene	z	2800					[A] < 0.010				[A] < 0.010		
Acenaphthylene	z	2800		0			[A] < 0.010				[A] < 0.010		
Acenaphthene	z	2800	mg/kg	_			[A] < 0.010				[A] < 0.010		
Fluorene	Z	2800	mg/kg	\vdash			[A] < 0.010				[A] < 0.010		
Phenanthrene	z	2800	mg/kg	-			[A] < 0.010				[A] < 0.010		
Anthracene	z	2800	mg/kg	0.010			[A] < 0.010				[A] < 0.010		
Fluoranthene	z	2800	mg/kg				[A] < 0.010				[A] < 0.010		
Pyrene	Z	2800	mg/kg	9			[A] < 0.010				[A] < 0.010		
Benzo[a]anthracene	z	2800	mg/kg				[A] < 0.010				[A] < 0.010		
Chrysene	z	2800					[A] < 0.010				[A] < 0.010		
Benzo[b]ffuoranthene	z	2800	mg/kg				[A] < 0.010				[A] < 0.010		
Benzo[k]fluoranthene	Z	2800	mg/kg	٥			[A] < 0.010				[A] < 0.010		
Benzo[a]pyrene	Z	2800	mg/kg	0.010			[A] < 0.010				[A] < 0.010		
Indeno(1,2,3-c,d)Pyrene	Z	2800	mg/kg	0			[A] < 0.010				[A] < 0.010		
Dibenz(a,h)Anthracene	Z	2800	mg/kg	0.010			[A] < 0.010				[A] < 0.010		
Benzo[g,h,i]perytene	z	2800	mg/kg	Ö			[A] < 0.010				[A] < 0.010		
Coronene	z	2800	mg/kg				[A] < 0.010				[A] < 0.010		
Total Of 17 PAH's	z	2800		_			[A] < 0.20				[A] < 0.20		
PCB 28	z	2815	mg/kg	0.0010		******	[A] < 0.0010				[A] < 0.0010		
				•				THE STATE OF THE S					Ĺ

Project: 24665 / 1 Monaghan Town Active Travel Development Site(CORA)

	THE REAL PROPERTY AND ADDRESS OF THE PERSON NAMED IN											
Client: IGSL		Chemi	Chemtest Job No	No.: 23-19446	23-19446	23-19446	23-19446	337	23-19446 23-19446	23-19446	23-19446 23-19446	23-19446
Quotation No.: Q20-19951	5	hemtest	Chemtest Sample ID.:	1653396	1653397	1653398	1653399	<u>L</u>	1653401	1653402	1653403	1653404
Order No.:		Client	Client Sample Ref.:	f.: AA200182	AA200188	AA200195	AA200196	AA200191	AA205178	AA205173	AA205175	AA205176
		Sam	Sample Location:	n: TP05	TP07	TP08	TP08	TP09	TP12	TP13	TP14	TP14
		رد	Sample Type:	e: SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
		Τc	ſop Depth (m):	0.70	0.50	08'0	1.80	0.70	08.0	09:0	0.70	1.50
		¥	Asbestos Lab:	.c		COVENTRY				NEW-ASB		
Determinand	Accred. SOP Units 1	SOP	nits LOD									1000
PCB 52	z	2815 mg	mg/kg 0.0010	0		[A] < 0.0010				[A] < 0.0010		
PCB 90+101	z	2815 mg/kg 0.0	3/kg 0.0010	C		[A] < 0.0010				[A] < 0.0010		
PCB 118	z	2815 mg/kg 0.(3/kg 0.0010	-		[A] < 0.0010				[A] < 0.0010		
PCB 153	Z	2815 mg	mg/kg 0.0010	-		[A] < 0.0010				[A] < 0.0010		
PCB 138	N	2815 mg	mg/kg 0.0010			[A] < 0.0010				[A] < 0.0010		
PCB 180	N N	:815 mg	2815 mg/kg 0.0010			[A] < 0.0010				[A] < 0.0010		
Total PCBs (7 congeners)	z	2815 mg	mg/kg 0.0010			[A] < 0.0010				[A] < 0.0010		
Total Phenois	0 	2920 mg/kg	1/kg 0.10			< 0.10				< 0.10		

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Limits Stable, Non-reactive Pazardous Pazardous	Chemtest Job No:	23-19446				Landfill	Landfill Waste Acceptance Criteria	e Criteria
National Carbon C	Chemtest Sample ID:	1653387					Limits	
Designation	Sample Ref:	AA192931					Stable, Non-	
Location: BH01 Decetion: Depth Decetion: Decetion: Decetion: Decetion: Depth Depth Decetion: D	Sample ID:						reactive	
Particle Depth(mi); Depth	Sample Location:	BH01			-		hazardous	Hazardous
Oppeth(m): SOP Accred. Units Landfill hazardous Ball SOP Accred. Units Landfill Landfill Landfill nand SOP Accred. Units SO Landfill	Top Depth(m):	0.50				Inert Waste	waste in non-	Waste
g Date: Accred. Units (A) 2.4 3 Canadial nand carbon 2825 U % [A) 2.4 3 5 guinticarbon 2810 U % 5.0 - - EX 2810 U mg/kg [A] < 0.010 6 - - EX 2810 U mg/kg [A] < 0.010 6 - - EX 2870 U mg/kg [A] < 0.010 6 - - 17 PAHS 2870 U mg/kg [A] < 0.010 - - - 17 PAHS 2010 U mg/kg [A] < 0.010 - - - 17 PAHS 2010 U mg/kg [A] < 0.010 -	Bottom Depth(m):					Landfill	hazardous	Landfill
nand SOP Acred Units M [A] 24 3 5 ganic Carbon 2625 U % [A] 24 3 5 ganic Carbon 2625 U % [A] 24 3 5 EX 2760 U mg/kg [A] 2010 6 — EX (2 congeners) 2815 U mg/kg [A] 240 1 — EX (2 congeners) 2815 U mg/kg [A] 240 1 — — EX (2 congeners) 2810 U mg/kg [A] 240 1 —	Sampling Date:						Landfill	
Parisic Carbon 2625 U P P P S D	Determinand	SOP	Accred.	Units				
Section Care Care	Total Organic Carbon	2625	n	%	[A] 2.4	3	5	9
EX 2760 U mg/kg [A] < 0.0010 6 — BS (7 congeners) 2815 N mg/kg [A] < 0.0010	Loss On Ignition	2610	n	%	5.0			10
Big (7 congeners) 2815 N mg/kg [A] < 0.0010 1 INVAC 2870 U mg/kg [A] < 10 INVAC 2800 N mg/kg [A] < 10 INVAC 2800 N mg/kg [A] < 100 INVAC 2800 N mg/kg [A] < 100 INVAC 2010 U mg/kg [A] < 100 Intelesation Capacity 2015 N mol/kg 0.0060 Intelesation Capacity 2015 N mol/kg 0.0060 0.0060 0.0060 0.0060 Intelesation Capacity 2015 N c.0.0050 0.006	Total BTEX	2760	n	mg/kg	[A] < 0.010	9	-	-
MAC 2670 U mg/kg [A] 0.500 U mg/kg [A] 0.55 1.00 U U T/8 T/8 U Sevalurate Sevalurate T/8 T/8 T/8 T/8 Sevalurate T/8	Total PCBs (7 congeners)	2815	Z	mg/kg	[A] < 0.0010	-	-	
Parks 2800	TPH Total WAC	2670	n	mg/kg	01 > [A]	200	-	
10 10 10 10 10 10 10 10	Total Of 17 PAH's	2800	Z	mg/kg	[A] 0.35	100	-	1
Intalisation Capacity 2015 N mol/kg 0.0060 — To evaluate nalysis Intalisation Capacity 2015 N 10:1 Eluate Limit values for compliance lear nalysis Interpretation Interpretation Interpretation Interpretation Interpretation 1455 U < 0.0005	Hd	2010	'n		7.8		9<	
nalysis 10:1 Eluate 10:1 Eluate Limit values for compliance lear mg/l mg/l mg/kg using BS EN 12457 at L/S 1 1455 U 0,0002 0,0523 0.5 2 n 1455 U < 0,0001	Acid Neutralisation Capacity	2015	Z	тоl/kg	0800'0	1	To evaluate	To evaluate
mg/l mg/l mg/kg using BS EN 12457 at L/S 10 to 1455 U 0,0002 0,0023 0,5 Z I	Eluate Analysis			10:1 Eluate	10:1 Eluate	Limit values	for compliance is	eaching test
1455 U 0,0002 0,005 0,5 2 1455 U < 0,005				mg/l	mg/kg	a Sing B	S EN 12457 at L/S	: 10 l/kg
1455 U < 0.005 0.050 20 100 In 1455 U < 0.0001	Arsenic	1455	n	0.0002	0.0023	0.5	2	25
Interview of the control of	Ваѓит	1455	ח	< 0.005	< 0.050	20	100	300
n 1455 U < 0,0005 < 0,0050 0.5 10	Cadmium	1455	n	< 0.00011	< 0.0011	0.04	*	c,
um 0.0011 0.011 2 50 um 455 U <0.0005	Chromium	1455	n	< 0.0005	< 0.0050	0.5	10	70
um 455 U <0.0006 <0.0056 0.01 0.00 <th< td=""><td>Copper</td><td>1455</td><td>n</td><td>0.0011</td><td>0.011</td><td>2</td><td>20</td><td>100</td></th<>	Copper	1455	n	0.0011	0.011	2	20	100
Jum 1455 U 0.0006 0.0058 0.5 10 1455 U 0.0007 0.0666 0.4 10 10 1455 U <0.0005	Mercury	1455	n	< 0.00005	< 0.00050	0.01	0.2	2
1455 U 0.0007 0.0666 0.4 10 1455 U <0.0050	Molybdenum	1455	n	0.0006	0.0058	0.5	10	30
(1455) U < 0.0005 < 0.0050 0.5 10 (1455) U < 0.0050	Nicke!	1455	∩	0.0007	0.0066	0.4	10	40
() 1455 U < 0.0050 0.06 0.7 () 1455 U < 0.005	Lead	1455	-	< 0.0005	< 0.0050	0.5	10	50
In the control of the contro	Antimony	1455	⊃	< 0.0005	< 0.0050	90:0	0.7	5
1455 U 0.005 0.046 4 50 FO FO FO FO FO FO FO F	Selenium	1455	D	< 0.0005	< 0.0050	0.1	0.5	7
1220 U <1.0 800 1500 1500	Zinc	1455	n	0.005	0.046	4	50	200
1220 U 0.15 1.5	Chloride	1220	¬	< 1.0	< 10	800	15000	25000
1220 U < 10 100 20000 Solved Solids 1020 N 45 450 6000 - dex 1920 U < 0.30	Fluoride	1220		0.15	1.5	10	150	500
1020 N 45 450 4060 60000 1020 U <0.030 1 1610 U 4.8 <50 500 800	Sulphate	1220	Ω	< 1,0	< 10	1000	20000	20000
1920 U < 0.30 1 - 1610 U 4.8 < 50 500 800	Total Dissolved Solids	1020	Z	45	450	4000	00009	100000
1610 U 4.8 <50 500 800	Phenol Index	1920	Ð	< 0.030	< 0.30	1	1	,
	Dissolved Organic Carbon	1610)	4.8	< 50	200	800	1000

Solid Information	
Dry mass of test portion/kg	060'0
Moisture (%)	25

Waste Acceptance Criteria

Project: 24665 / 1 Monaghan Town Active Travel Development Site(CORA)

Chemtest Job No:	23-19446				(interesting i	andell March Accombance Cuitation	Cattonia
Chemtest Sample ID:	1653389					Limits	POLICE
Sample Ref:	AA192934					Stable Non-	
Sample ID:						reactive	
Sample Location:	BH03					hazardous	Hazardous
Top Depth(m):	0.50				inert Waste	waste in non-	Waste
Bottom Depth(m):					Landfill	hazardous	Landfill
Sampling Date:						Landfill	
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	Ω	%	[A] 2,0	3	5	9
Loss On Ignition	2610	n	%	1.5			10
Total BTEX	2760	ñ	mg/kg	[A] < 0.010	9	-	
Total PCBs (7 congeners)	2815	Z	mg/kg	[A] < 0.0010	_		
TPH Total WAC	2670	n	mg/kg	[A] < 10	500		
Total Of 17 PAH's	2800	Z	mg/kg	[A] < 0.20	100		
Ha	2010	ח		8.0	7.0	9<	
Acid Neutralisation Capacity	2015	Z	mol/kg	0.012	3	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	10:1 Eluate	Limit values	Limit values for compliance leaching test	eaching test
			mg/i	mg/kg	using B	using BS EN 12457 at L/S 10 l/kg	10 l/kg
Arsenic	1455	ם	0.0003	0.0031	0.5	2	25
Barium	1455	n	< 0.005	< 0.050	20	100	300
Cadmium	1455	n	< 0.00011	< 0.0011	0.04	-	2
Chromium	1455	n	< 0.0005	< 0.0050	9.0	10	70
Copper	1455	n	0.0011	0.011	2	50	100
Mercury	1455	n	< 0.00005	< 0.00050	0.01	0.2	2
Mołybdenum	1455	Ð	0.0008	0.0079	0.5	10	30
Nickel	1455	n	0.0005	0.0050	0.4	10	40
Lead	1455	n	< 0.0005	< 0.0050	0.5	10	50
Antimony	1455	n	< 0.0005	< 0.0050	0.06	0.7	c
Seleníum	1455	n	< 0.0005	< 0.0050	0.1	0.5	7
Zinc	1455	U	0.003	0.033	4	20	200
Chloride	1220	n	< 1.0	< 10	800	15000	25000
Fluoride	1220	Ù	0.10	1.0	10	150	200
Sulphate	1220	D	< 1.0	< 10	1000	20000	50000
Total Dissolved Solids	1020	Z	47	470	4000	60000	100000
Phenol Index	1920	٦	< 0.030	< 0.30	-	-	-
Dissolved Organic Carbon	1610	U	4.1	< 50	200	800	1000

CORD MICHIGAN	
Dry mass of test portion/kg	060'0
(Moisture (%)	22

Waste Acceptance Criteria

Project; 24665 / 1 Monaghan Town Active Travel Development Site(CORA)

Chemtest Sample ID: Sample Ref: Sample ID: Sample Location:	C++0:-0.4				Landfill	Landfill Waste Acceptance Criteria	Criteria
Sample Ref: Sample ID: Sample Location: Ton Dooth(m):	1653392					Limits	
Sample ID: Sample Location:	AA171710					Stable, Non-	
Sample Location:						reactive	
Ton Donshim).	BH07					hazardous	Hazardous
Lob Deputation.	0.80				Inert Waste	waste in non-	Waste
Bottom Depth(m):					Landfill	hazardous	Landfill
Sampling Date:				*****		Landfill	
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	Ω	%	[A] 1.3	ဇ	5	9
Loss On Ignition	2610	n	%	10	1	l	10
Total BTEX	2760	n	mg/kg	[A] < 0.010	9	1	
Total PCBs (7 congeners)	2815	Z	mg/kg	[A] < 0.0010	-	-	ſ
TPH Total WAC	2670	ח	mg/kg	[A] < 10	500		1
Total Of 17 PAH's	2800	z	mg/kg	[A] < 0.20	100		1
Hd	2010	P		8.2	***	9<	ı
Acid Neutralisation Capacity	2015	Z	mol/kg	0.011	**	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	10:1 Eluate	Limit values	Limit values for compliance leaching test	aching test
			mg/l	mg/kg	using B	using BS EN 12457 at L/S 10 l/kg	10 l/kg
Arsenic	1455	Ŋ	0.0003	0:0030	0.5	2	25
Barium	1455	n	< 0.005	< 0.050	20	100	300
Cadmium	1455	ņ	< 0.00011	< 0.0011	0.04	1	5
Chromium	1455	n	< 0.0005	< 0.0050	0.5	10	70
Copper	1455	n	0.0016	0.016	2	50	100
Mercury	1455	D	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455)	0.0006	0.0062	0.5	10	30
Nickel	1455	n	0.0008	0.0077	0.4	10	40
Lead	1455	⊃	< 0.0005	< 0.0050	0.5	10	50
Antimony	1455	n	< 0.0005	< 0.0050	90:0	0.7	3
Selenium	1455	n	< 0.0005	< 0.0050	0.1	0.5	7
Zinc	1455	n	0.003	0.035	4	50	200
Chloride	1220	n	< 1.0	< 10	008	15000	25000
Fluoride	1220	n	0.14	1.4	10	150	200
Sulphate	1220	n	2.3	23	1000	20000	50000
Total Dissolved Solids	1020	z	40	400	4000	00009	100000
Phenol Index	1920	n	< 0.030	< 0.30		-	1
Dissolved Organic Carbon	1610	ח	4.4	> 50	200	800	1000

	0.090	6.8
Solid Information	Dry mass of test portion/kg	Moisture (%)

Waste Acceptance Criteria

Project: 24665 / 1 Monaghan Town Active Travel Development Site(CORA)

	0110				Time:	andfill Waste Accentance Criteria	Critoria
Chemtest Sample ID:	1653395			······································		Limits	
Sample Ref:	AA200184					Stable, Non-	
Sample ID:						reactive	
Sample Location:	TP04					hazardous	Hazardous
Top Depth(m):	0.50				Inert Waste	waste in non-	Waste
Bottom Depth(m):					Landfill	hazardous	Landfill
Sampling Date:						Landfill	
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	n	%	[A] 1.3	3	5	9
Loss On Ignition	2610	n	%	3.1	1	1	10
Total BTEX	2760	n	mg/kg	[A] < 0.010	9		
Total PCBs (7 congeners)	2815	Z	mg/kg	[A] < 0.0010	,		1
TPH Total WAC	2670	_	mg/kg	[A] < 10	500	-	
Total Of 17 PAH's	2800	z	mg/kg	[A] < 0.20	100	11.	ı
ЬН	2010)		8.2	1	9<	
Acid Neutralisation Capacity	2015	Z	mol/kg	0900'0	1	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	10:1 Eluate	Limit values	Limit values for compliance leaching test	aching test
			l∕g⁄⊓	mg/kg	using B	using BS EN 12457 at L/S 10 I/kg	10 l/kg
Arsenic	1455	ח	0.0003	0.0029	9.0	2	25
Barium	1455	⊃	< 0.005	< 0.050	20	100	300
Cadmium	1455	U	< 0.00011	< 0.0011	0.04	+	5
Chromium	1455	n	< 0.0005	< 0.0050	0.5	10	70
Copper	1455	n	0.0010	0.010	2	50	100
Mercury	1455	n	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	ח	0.0008	0.0081	0.5	10	30
Nickel	1455	⊃	0.0005	0.0053	0.4	10	40
Lead	1455	⊃	< 0.0005	< 0.0050	0.5	10	50
Antimony	1455	n	< 0.0005	< 0.0050	90.0	7.0	5
Selenium	1455	n	< 0.0005	< 0.0050	0.1	0.5	
Zinc	1455	ລ	0.005	0.055	4	20	200
Chloride	1220	U	< 1.0	< 10	800	15000	25000
Fluoride	1220	Ų	0.12	1.2	10	150	500
Sulphate	1220	ņ	< 1.0	< 10	1000	20000	20000
Total Dissolved Solids	1020	z	31	310	4000	00009	100000
Phenol index	1920	n	< 0.030	< 0.30	-		
Dissolved Organic Carbon	1610	Ú	3.5	< 50	200	800	1000

Solid Information	
Dry mass of test portion/kg	060'0
Moisture (%)	10

Waste Acceptance Criteria

Project: 24665 / 1 Monaghan Town Active Travel Development Site(CORA)

Chemtest Job No:	23-19446				Landfill V	Landfill Waste Acceptance Criteria	e Criferia
Chemtest Sample ID:	1653398					Limits	
Sample Ref:	AA200195					Stable, Non-	
Sample ID:						reactive	
Sample Location:	TP08					hazardous	Hazardous
Top Depth(m):	0.80				Inert Waste	waste in non-	Waste
Bottom Depth(m):					Landfill	hazardous	Landfill
Sampling Date:						Landfill	
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	n	%	[A] 0.77	3	S	9
Loss On Ignition	2610	Ŋ	%	2.8		ļ	10
Total BTEX	2760	n	mg/kg	[A] < 0.010	ගු	1	1
Total PCBs (7 congeners)	2815	N	mg/kg	[A] < 0.0010	Į		t
TPH Total WAC	2670	U	mg/kg	[A] < 10	200	ì	1
Total Of 17 PAH's	2800	N	mg/kg	[A] < 0.20	100	-	-
Hd	2010	U		8.4		9<	l
Acid Neutralisation Capacity	2015	Z	бұ/₃ош	0.017	1	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	10:1 Eluate	Limit values	Limit values for compliance leaching test	eaching test
			mg/i	mg/kg	using B	using BS EN 12457 at L/S 10 l/kg	10 l/kg
Arsenic	1455	n	9000.0	0.0061	9'0	2	25
Barium	1455	U	< 0.005	< 0.050	20	100	300
Cadmium	1455	O	< 0.00011	< 0.0011	0.04	-	5
Chromium	1455	, n	< 0.0005	< 0.0050	0.5	10	70
Copper	1455	U	0.0006	0.0060	2	90	100
Mercury	1455	U	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	n	0.0008	0.0076	0.5	10	30
Nickel	1455	U	< 0.0005	< 0.0050	0.4	10	40
Lead	1455	U	< 0.0005	< 0.0050	0.5	10	50
Antimony	1455	U	< 0.0005	< 0.0050	90.0	2.0	5
Selenium	1455	n	< 0.0005	< 0.0050	0.1	\$'0	7
Zinc	1455	n	0.003	0.034	4	50	200
Chloride	1220	n	< 1.0	< 10	800	15000	25000
Fluoride	1220	n	0.096	< 1.0	10	150	500
Sulphate	1220	n	< 1.0	< 10	1000	20000	50000
Total Dissolved Solids	1020	z	31	310	4000	00009	100000
Phenol index	1920	⊃	< 0.030	< 0.30	*-	ł	•
Dissolved Organic Carbon	1610	D	3.0	< 50	500	800	1000

Ë	
Dry mass of test portion/kg	0.090
Moisture (%)	10

Waste Acceptance Criteria

Project: 24665 / 1 Monaghan Town Active Travel Development Site(CORA)

Chemtest Job No:	23-19446				Landfill	Landfill Waste Acceptance Criteria	e Criteria
Chemtest Sample ID:	1653402					Limits	
Sample Ref:	AA205173					Stable, Non-	
Sample ID:						reactive	
Sample Location:	TP13					hazardous	Hazardous
Top Depth(m):	0.60				Inert Waste	waste in non-	Waste
Bottom Depth(m):					Landfill	hazardous	Landfill
Sampling Date:						Landfill	
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	n	%	[A] 0.34	9	5	9
Loss On Ignition	2610	Þ	%	4.7	ł	1	10
Total BTEX	2760	n	mg/kg	[A] < 0.010	9	444	
Total PCBs (7 congeners)	2815	Z	mg/kg	[A] < 0.0010	1	1	1
TPH Total WAC	2670	n	mg/kg	0t > [A]	500	11.11	1
Total Of 17 PAH's	2800	Z	mg/kg	[A] < 0.20	100	ŀ	1
Hd	2010	n		7.8		9,	1
Acid Neutralisation Capacity	2015	2	mol/kg	0600'0	*;	To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	10:1 Eluate	Limit values	Limit values for compliance leaching test	eaching test
			mg/l	mg/kg	using B	using BS EN 12457 at L/S 10 l/kg	i 10 l/kg
Arsenic	1455	n	0.0019	0.019	0.5	2	25
Barium	1455	Ω	< 0.005	< 0.050	20	100	300
Cadmium	1455	n	< 0.00011	< 0.0011	0.04	-	5
Chromium	1455	n	0.0036	960.0	0.5	10	70
Copper	1455	n	0.0058	0.058	2	20	100
Mercury	1455	n	< 0.00005	< 0.00050	0.01	0.2	2
Molybdenum	1455	n	0.0004	0.0043	0.5	10	30
Nickel	1455	U	0.0056	0.056	0.4	10	40
Lead	1455	⊃	0.0013	0.013	0.5	10	20
Antimony	1455	n	< 0.0005	< 0.0050	0.06	0.7	5
Sefenium	1455	n	< 0.0005	< 0.0050	0.1	0.5	7
Zinc	1455	U	0.010	0.10	4	50	200
Chloride	1220	n	1.1	11	008	15000	25000
Fluoride	1220	Ŋ	0.12	1.2	10	150	500
Sulphate	1220	n	2.0	20	1000	20000	50000
Total Dissolved Solids	1020	Z	13	130	4000	00009	100000
Phenol Index	1920	Ω	< 0.030	< 0.30	τ-	1	-
Dissolved Organic Carbon	1610	ņ	5.7	57	500	800	1000

Solid Information	
Dry mass of test portion/kg	0.090
Moisture (%)	19

Waste Acceptance Criteria

Deviations

In accordance with UKAS Policy on Deviating Samples TPS 63. Chemtest have a procedure to ensure 'upon receipt of each sample a competent laboratory shall assess whether the sample is suitable with regard to the requested test(s)'. This policy and the respective holding times applied, can be supplied upon request. The reason a sample is declared as deviating is detailed below. Where applicable the analysis remains UKAS/MCERTs accredited but the results may be compromised.

Sample:	Sample Ref:	Sample ID:	Sample Location:	Sampled Date:	Deviation Code(s):	Containers Received:
1653387	AA192931		BH01		A	Amber Glass 250ml
1653387	AA192931		BH01		А	Plastic Tub 500g
1653388	AA197802		BH02		A	Amber Glass 250ml
1653388	AA197802		BH02		А	Plastic Tub 500g
1653389	AA192934		BH03		A	Amber Glass 250ml
1653389	AA192934		BH03		А	Plastic Tub 500g
1653390	AA192939		ВН04А		A	Amber Glass 250ml
1653390	AA192939		BH04A		А	Plastic Tub 500g
1653391	AA192947		BH05		A	Amber Glass 250ml
1653391	AA192947		BH05		А	Plastic Tub 500g
1653392	AA171710		BH07		Α	Amber Glass 250ml
1653392	AA171710		BH07		А	Plastic Tub 500g
1653393	AA200193		TP01		А	Amber Glass 250ml
1653393	AA200193		TP01		А	Plastic Tub 500g
1653394	AA200179		TP03		А	Amber Glass 250ml
1653394	AA200179		TP03		А	Plastic Tub 500g
1653395	AA200184		TP04		Α	Amber Glass 250ml
1653395	AA200184		TP04		Α	Plastic Tub 500g
1653396	AA200182		TP05		Α	Amber Glass 250ml
1653396	AA200182		TP05		А	Plastic Tub 500g
1653397	AA200188		TP07		Α	Amber Glass 250ml
1653397	AA200188		TP07		А	Plastic Tub 500g

Deviations

In accordance with UKAS Policy on Deviating Samples TPS 63. Chemtest have a procedure to ensure 'upon receipt of each sample a competent laboratory shall assess whether the sample is suitable with regard to the requested test(s)'. This policy and the respective holding times applied, can be supplied upon request. The reason a sample is declared as deviating is detailed below. Where applicable the analysis remains UKAS/MCERTs accredited but the results may be compromised.

Sample:	Sample Ref:	Sample ID:	Sample Location:	Sampled Date:	Deviation Code(s):	Containers Received:
1653398	AA200195		TP08		A	Amber Glass 250ml
1653398	AA200195		TP08		А	Plastic Tub 500g
1653399	AA200196		TP08		А	Amber Glass 250ml
1653399	AA200196		TP08		А	Plastic Tub 500g
1653400	AA200191		TP09		А	Amber Glass 250ml
1653400	AA200191		TP09		А	Plastic Tub 500g
1653401	AA205178		TP12		А	Amber Glass 250ml
1653401	AA205178		TP12		А	Plastic Tub 500g
1653402	AA205173		TP13		А	Amber Glass 250ml
1653402	AA205173		TP13		A	Plastic Tub 500g
1653403	AA205175		TP14		A	Amber Glass 250ml
1653403	AA205175		TP14		А	Plastic Tub 500g
1653404	AA205176		TP14		А	Amber Glass 250ml
1653404	AA205176		TP14		Α	Plastic Tub 500g

Test Methods

SOP	Title	Parameters included	Method summary
1010	pH Value of Waters	Ηα	pH Meter
1020	Electrical Conductivity and Total Dissolved Solids (TDS) in Waters	Electrical Conductivity and Total Dissolved Solids (TDS) in Waters	Conductivity Meter
1220	Anions, Alkatinity & Ammonium in Waters	Fluoride; Chloride; Nitrite; Nitrate; Total; Oxidisable Nitrogen (TON); Sulfate; Phosphate; Alkalinity; Ammonium	Automated colorimetric analysis using 'Aquakem 600' Discrete Analyser.
1455	Metals in Waters by ICP-MS	Metals, including: Antimony; Arsenic; Barium; Beryllium; Boron; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Tin; Vanadium; Zinc	determination by inductively coupled plasma
1610	Total/Dissolved Organic Carbon in Waters	Organic Carbon	TOC Analyser using Catalytic Oxidation
1800	Speciated Polynuclear Aromatic Hydrocarbons (PAH) in Waters by GC-MS	Acenaphthene; Acenaphthylene; Anthracene; Benzo[a]Anthracene; Benzo[a]Pyrene; Benzo[b]Fluoranthene; Benzo[ghi]Perylene; Benzo[k]Fluoranthene; Chrysene; Dibenz[ah]Anthracene; Fluoranthene; Fluorene; Indeno[123cd]Pyrene; Naphthalene; Phenanthrene; Pyrene	Pentane extraction / GCMS detection
1920	Phenols in Waters by HPLC	Phenolic compounds including: Phenol, Cresols, Xylenols, Trimethylphenols Note: Chlorophenols are excluded.	Determination by High Performance Liquid Chromatography (HPLC) using electrochemical detection.
2010	pH Value of Soils	pH	pH Meter
2015	Acid Neutralisation Capacity	Acid Reserve	Titration
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2040	Soil Description(Requirement of MCERTS)	Soil description	As received soil is described based upon BS5930
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2175	Total Sulphur in Soils	Total Sulphur	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2180	Sulphur (Elemental) in Soils by HPLC	Sulphur	Dichloromethane extraction / HPLC with UV detection
2192	Asbestos	Asbestos	Polarised light microscopy / Gravimetry
2220	Water soluble Chloride in Soils	Chloride	Aqueous extraction and measuremernt by 'Aquakem 600' Discrete Analyser using ferric nitrate / mercuric thiocyanate.
2300	Cyanides & Thiocyanate in Soils	Free (or easy liberatable) Cyanide; total Cyanide; complex Cyanide; Thiocyanate	Allkaline extraction followed by colorimetric determination using Automated Flow Injection Analyser.
2325	Sulphide in Soils	Sulphide	Steam distillation with sulphuric acid / analysis by 'Aquakem 600' Discrete Analyser, using N,N-dimethyl-p-phenylenediamine.
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.
2455	Acid Soluble Metals in Soils	Metals, including: Arsenic; Barium; Beryllium; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Vanadium; Zinc	Acid digestion followed by determination of metals in extract by ICP-MS.
2490	Hexavalent Chromium in Soils	Chromium [VI]	Soil extracts are prepared by extracting dried and ground soil samples into boiling water. Chromium [VI] is determined by 'Aquakem 600' Discrete Analyser using 1,5-diphenylcarbazide.

Test Methods

SOP	Title	Parameters included	Method summary		
2610	Loss on Ignition	loss on ignition (LOI)	Determination of the proportion by mass that is lost from a soil by ignition at 550°C.		
2625	Total Organic Carbon in Soils	Total organic Carbon (TOC)	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.		
2670	Total Petroleum Hydrocarbons (TPH) in Soils by GC-FID	TPH (C6–C40); optional carbon banding, e.g. 3-band – GRO, DRO & LRO*TPH C8–C40	Dichloromethane extraction / GC-FID		
2680	TPH A/A Split	Aliphatics: >C5-C6, >C6-C8, >C8-C10, >C10-C12, >C12-C16, >C16-C21, >C21- C35, >C35- C44Aromatics: >C5-C7, >C7-C8, >C8-C10, >C10-C12, >C12-C16, >C16-C21, >C21-C35, >C35-C44	Dichloromethane extraction / GCxGC FID detection		
2760	Volatile Organic Compounds (VOCs) in Soils by Headspace GC-MS	Volatile organic compounds, including BTEX and halogenated Aliphatic/Aromatics.(cf. USEPA Method 8260)*please refer to UKAS schedule	Automated headspace gas chromatographic (GC) analysis of a soil sample, as received, with mass spectrometric (MS) detection of volatile organic compounds.		
2800	Speciated Polynuclear Aromatic Hydrocarbons (PAH) in Soil by GC-MS	Acenaphthene*; Acenaphthylene; Anthracene*; Benzo[a]Anthracene*; Benzo[a]Pyrene*; Benzo[b]Fluoranthene*; Benzo[ghi]Perylene*; Benzo[k]Fluoranthene; Chrysene*; Dibenz[ah]Anthracene; Fluoranthene*; Fluorene*; Indeno[123cd]Pyrene*; Naphthalene*; Phenanthrene*; Pyrene*	Dichloromethane extraction / GC-MS		
2815	Polychlorinated Biphenyls (PCB) ICES7Congeners in Soils by GC-MS	ICES7 PCB congeners	Acetone/Hexane extraction / GC-MS		
2920	Phenois in Soils by HPLC	Phenolic compounds including Resorcinol, Phenol, Methylphenols, Dimethylphenols, 1- Naphthol and TrimethylphenolsNote: chlorophenols are excluded.	60:40 methanol/water mixture extraction, followed by HPLC determination using electrochemical detection.		
640	Characterisation of Waste (Leaching C10)	Waste material including soil, sludges and granular waste	ComplianceTest for Leaching of Granular Waste Material and Sludge		

Report Information

Key	
U	UKAS accredited
М	MCERTS and UKAS accredited
Ν	Unaccredited
s	This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
SN	This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
Т	This analysis has been subcontracted to an unaccredited laboratory
I/S	Insufficient Sample
U/S	Unsuitable Sample
N/E	not evaluated
<	"less than"
>	"greater than"
SOP	Standard operating procedure
LOD	Limit of detection

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

Sample Deviation Codes

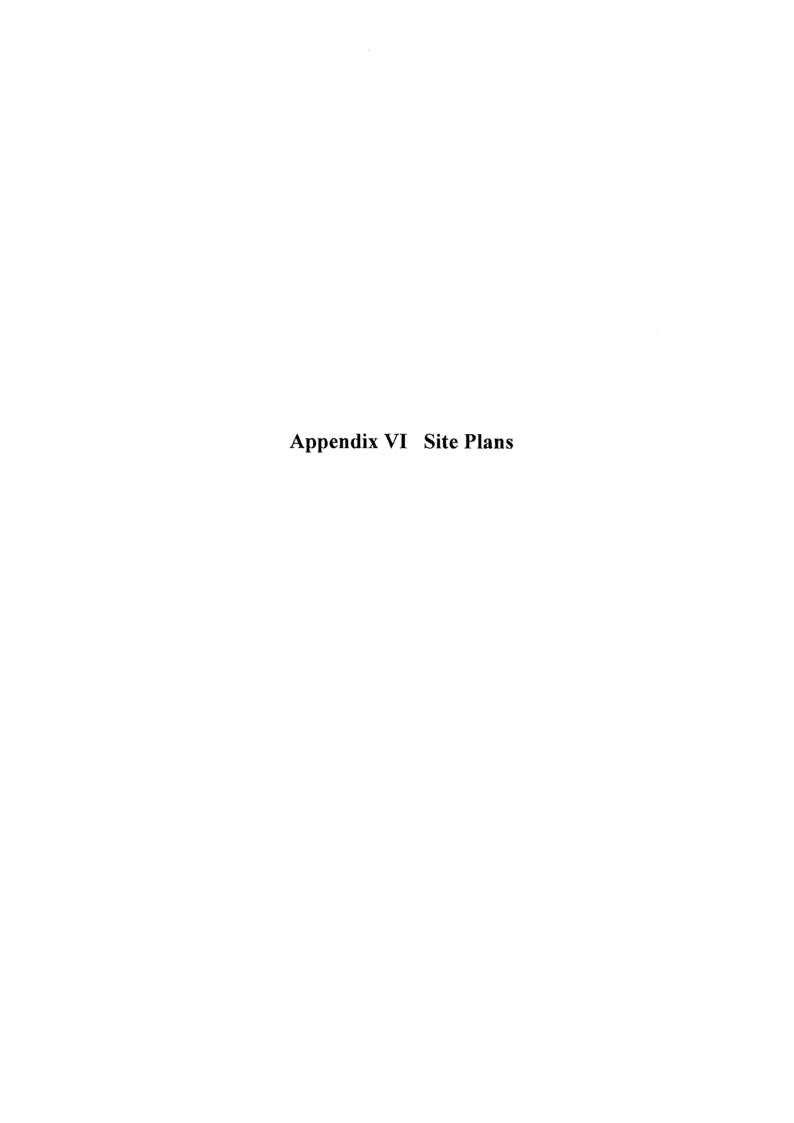
- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

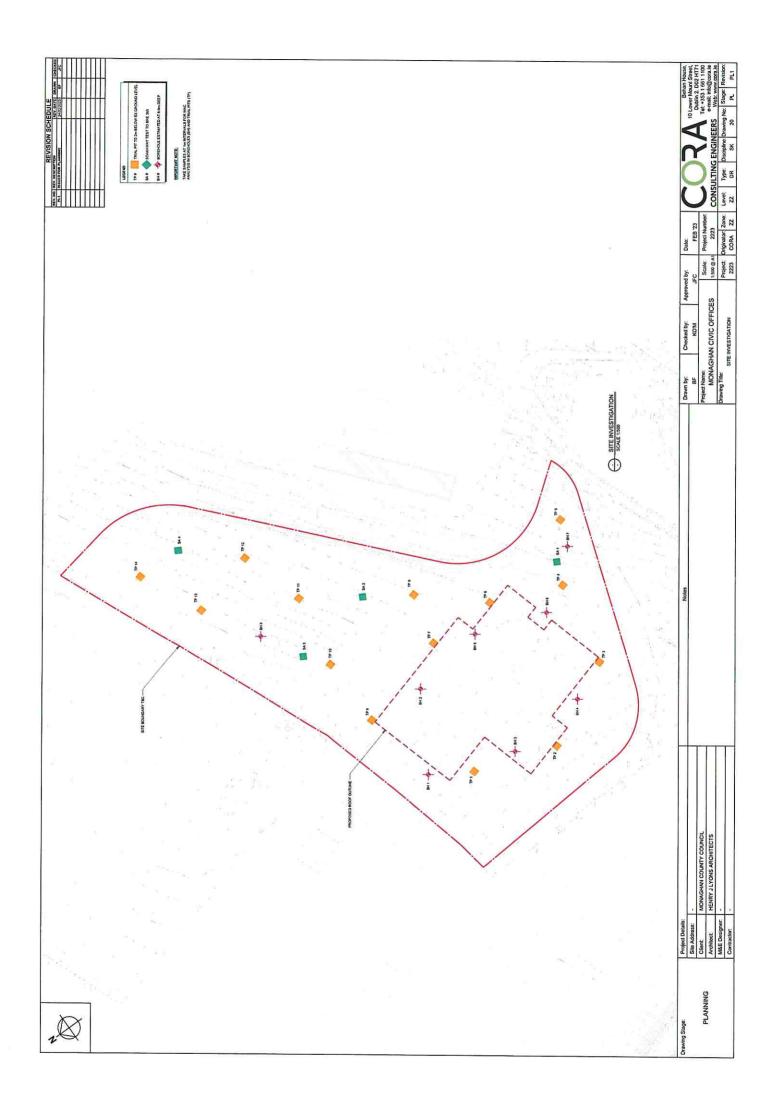
Sample Retention and Disposal

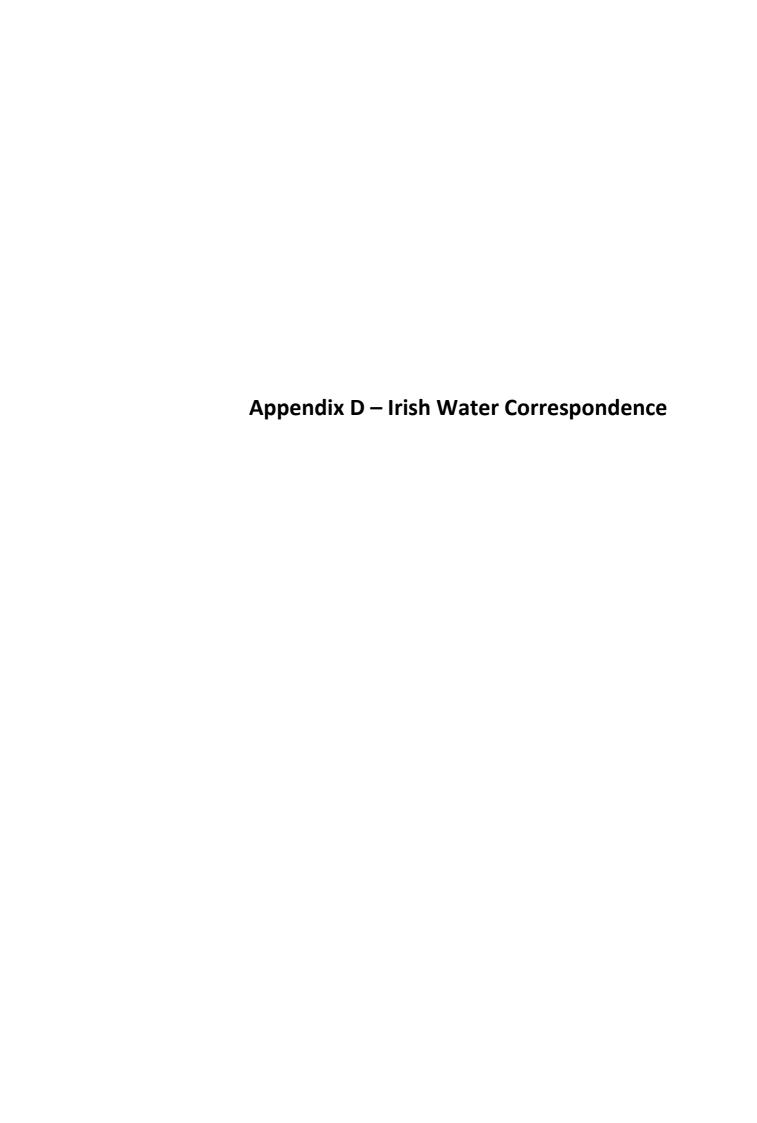
All soil samples will be retained for a period of 30 days from the date of receipt All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>









Uisce Éi reann

Bosca OP 448 Oifig Sheach adta na Cathrach Theas

Cathair Chorcaí

Irish Water

PO Box 448,

South City Delivery Office,

Cork City.

CONFIRMATION OF FEASIBILITY

John Callanan

CORA Consulting Engineers 10 Lower Mount Street Dublin 2 Co. Dublin D02HT71

7 June 2023

Our Ref: CDS23004020 Pre-Connection Enquiry Monaghan County Council Headquarters, Roosky, Monaghan Town, Monaghan

Dear Applicant/Agent,

We have completed the review of the Pre-Connection Enquiry.

Irish Water has reviewed the pre-connection enquiry in relation to a Water & Wastewater connection for a Business Connection of 1 unit(s) at Monaghan County Council Headquarters, Roosky, Monaghan Town, Monaghan, (the **Development**).

Based upon the details provided we can advise the following regarding connecting to the networks;

Water Connection

Feasible Subject to upgrades

Please note, while flows in excess of your required demand may be achieved in the Irish Water network and could be utilised in the event of a fire, Irish Water cannot guarantee a flow rate to meet your fire flow requirement. To guarantee a flow to meet the Fire Authority requirements you should provide adequate fire storage capacity within your development.

Please note that according to our records there is an existing 200mm dia water main running through this site (see drawing attached).

Any structures or works over or in close

Stiurthóirí / Directors: Tony Keohane (Chairman), Niall Gleeson (CEO), Christopher Banks, Fred Barry, Gerard Britchfield, Liz Joyce, Patricia King, Eileen Maher, Cathy Mannion, Michael Walsh

proximity to Irish Water infrastructure that will inhibit access for maintenance or endanger structural or functional integrity of the infrastructure are not allowed. The layout of the development must ensure that this pipe is protected and adequate separation distances are provided between Irish Water infrastructure and any structures on site. Alternatively you may enter into a diversion agreement with Irish Water and divert the pipe to accommodate your development. If you wish to proceed with this option please contact Irish Water at Diversions@water.ie and submit detailed design drawings before submitting your planning application. It will be necessary to provide a wayleave over this pipe to the benefit of Irish Water and ensure that it is accessible for maintenance. For more information, please see go to the link below: https://www.water.ie/connections/developerservices/diversions/

A connection off this diverted watermain for the proposed building is feasible.

Wastewater
 Connection

- Feasible Subject to upgrades
- A 130m network extension in the public domain (along Rooskey Vale Rd) will be required.

This letter does not constitute an offer, in whole or in part, to provide a connection to any Irish Water infrastructure. Before the Development can be connected to our network(s) you must submit a connection application and be granted and sign a connection agreement with Irish Water.

As the network capacity changes constantly, this review is only valid at the time of its completion. As soon as planning permission has been granted for the Development, a completed connection application should be submitted. The connection application is available at www.water.ie/connections/get-connected/

Where can you find more information?

- Section A What is important to know?
- Section B Details of Irish Water's Network(s)

This letter is issued to provide information about the current feasibility of the proposed connection(s) to Irish Water's network(s). This is not a connection offer and capacity in Irish Water's network(s) may only be secured by entering into a connection agreement with Irish Water.

For any further information, visit www.water.ie/connections, email newconnections@water.ie or contact 1800 278 278.

Yours sincerely,

Yvonne Harris

Head of Customer Operations

Section A - What is important to know?

What is important to know?	Why is this important?		
Do you need a contract to connect?	Yes, a contract is required to connect. This letter does not constitute a contract or an offer in whole or in part to provide a connection to Irish Water's network(s).		
	 Before the Development can connect to Irish Water's network(s), you must submit a connection application and be granted and sign a connection agreement with Irish Water. 		
When should I submit a Connection Application?	A connection application should only be submitted after planning permission has been granted.		
Where can I find information on connection charges?	Irish Water connection charges can be found at: https://www.water.ie/connections/information/charges/		
Who will carry out the connection work?	 All works to Irish Water's network(s), including works in the public space, must be carried out by Irish Water*. 		
	*Where a Developer has been granted specific permission and has been issued a connection offer for Self-Lay in the Public Road/Area, they may complete the relevant connection works		
Fire flow Requirements	The Confirmation of Feasibility does not extend to fire flow requirements for the Development. Fire flow requirements are a matter for the Developer to determine.		
	What to do? - Contact the relevant Local Fire Authority		
Plan for disposal of storm water	The Confirmation of Feasibility does not extend to the management or disposal of storm water or ground waters.		
	 What to do? - Contact the relevant Local Authority to discuss the management or disposal of proposed storm water or ground water discharges. 		
Where do I find details of Irish Water's network(s)?	Requests for maps showing Irish Water's network(s) can be submitted to: datarequests@water.ie		

What are the design requirements for the connection(s)?	•	The design and construction of the Water & Wastewater pipes and related infrastructure to be installed in this Development shall comply with <i>the Irish Water</i> Connections and Developer Services Standard Details and Codes of Practice, available at www.water.ie/connections
Trade Effluent Licensing	•	Any person discharging trade effluent** to a sewer, must have a Trade Effluent Licence issued pursuant to section 16 of the Local Government (Water Pollution) Act, 1977 (as amended).
	•	More information and an application form for a Trade Effluent License can be found at the following link: https://www.water.ie/business/trade-effluent/about/ **trade effluent is defined in the Local Government (Water Pollution) Act, 1977 (as amended)

Section B – Details of Irish Water's Network(s)

The map included below outlines the current Irish Water infrastructure adjacent the Development: To access Irish Water Maps email datarequests@water.ie

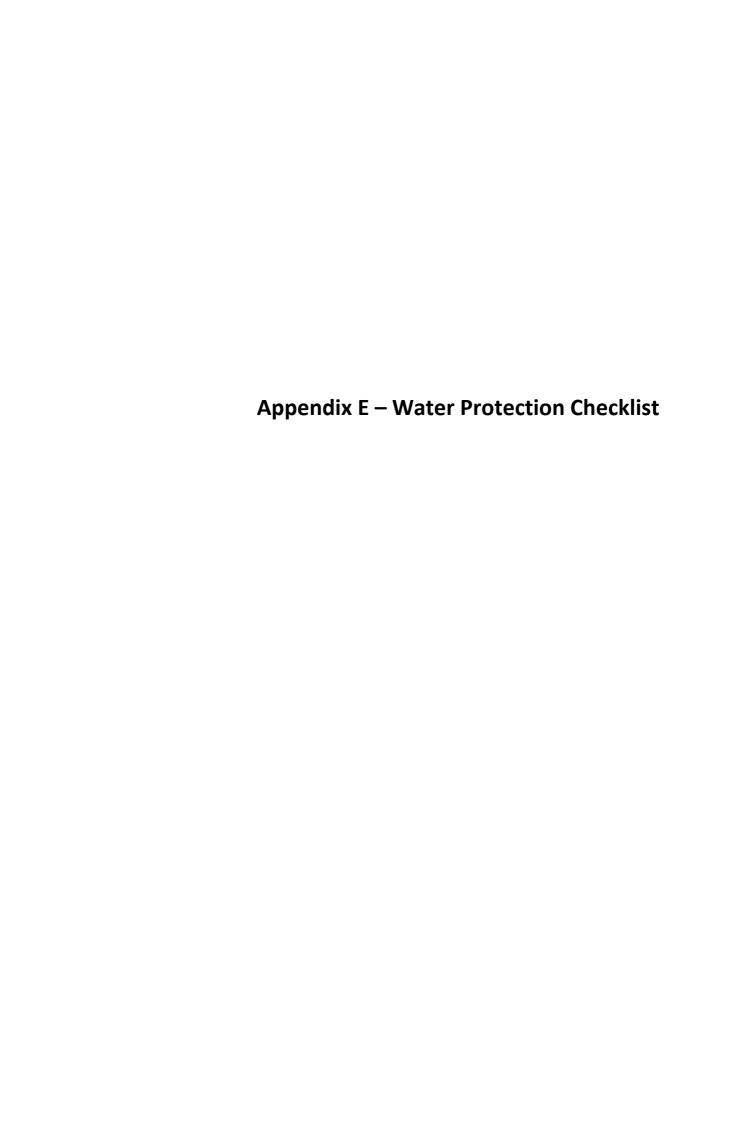


Reproduced from the Ordnance Survey of Ireland by Permission of the Government. License No. 3-3-34

Note: The information provided on the included maps as to the position of Irish Water's underground network(s) is provided as a general guide only. The information is based on the best available information provided by each Local Authority in Ireland to Irish Water.

Whilst every care has been taken in respect of the information on Irish Water's network(s), Irish Water assumes no responsibility for and gives no guarantees, undertakings or warranties concerning the accuracy, completeness or up to date nature of the information provided, nor does it accept any liability whatsoever arising from or out of any errors or omissions. This information should not be solely relied upon in the event of excavations or any other works being carried out in the vicinity of Irish Water's underground network(s). The onus is on the parties carrying out excavations or any other works to ensure the exact location of Irish Water's underground network(s) is identified prior to

excavations or any other works being carried out. Service connection pipes are not generally shown but their presence should be anticipated.

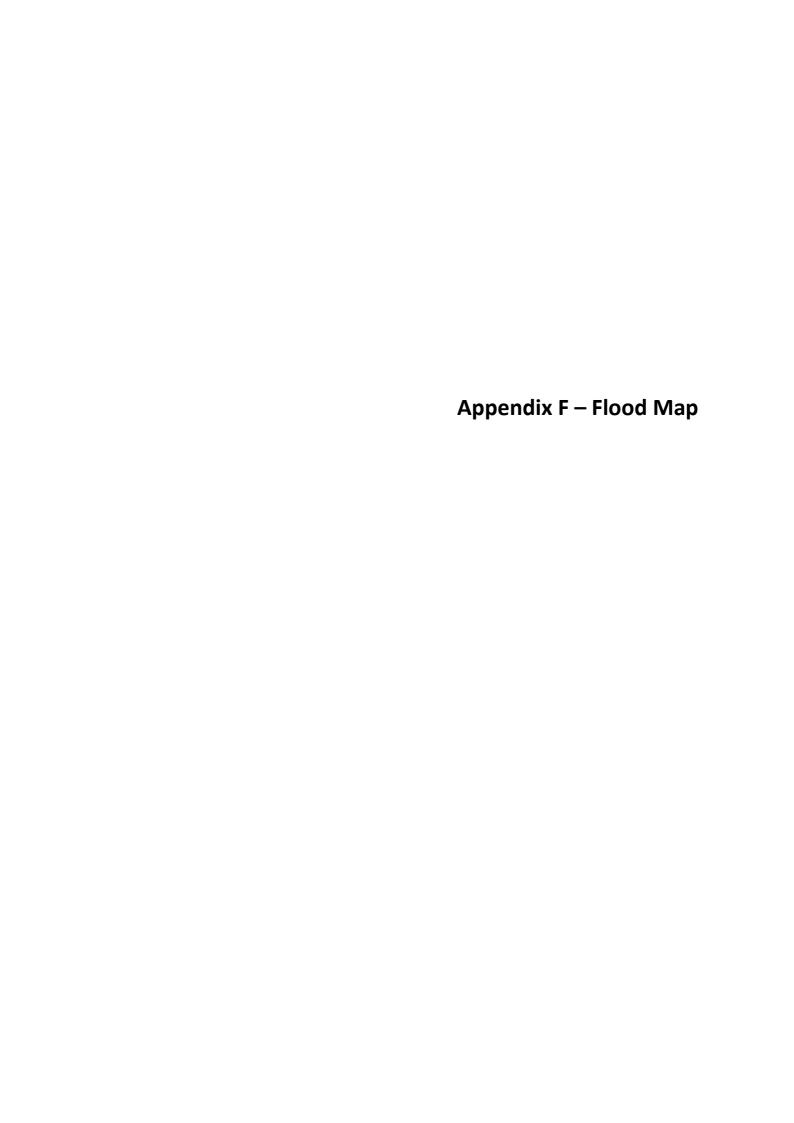


MONAGHAN LOCAL AUTHORITIES Water Protection Plan Checklist

(To be accompanied by a Site Drainage Plan - Refer to Chapter 4 of Monaghan County Development Plan 2013-2019)

General Site and Water Body Details								
Planning Ref. No. Not yet assigned			Applio			Monaghan Co.	Monaghan Co. Co.	
Townland	Roosky				y Source		Public Watermain	
X Co-ordinate 1	267521			Y Co-ordinate ¹		333834		
WMU ²				RWB ²		Shambles 010		
WB Status ²				tive ²				
Groundwater Low to Moderate Vulnerability ³					ortance ³	Regionally Imp Aquifer	Regionally Important Aquifer	
Proximity to nearest (meters)	Proximity to nearest watercourse (culverte			en), we	etland or lal	Patena Lake – 4 Ulster Canal – 6		
,		Propos	ed Devel	onmer	nt	Olotor Cariai	700111	
Is the d	leveloni	ment entirely		_		ing? (tick)		
Domestic dwelling	ic velopi	Agricultural (cattle/dairy)	or part o	l one c		ial-food related		
Public Works	/	Agricultural (mushrooms)		Industrial-n	on food		
Housing		Àgricultural (poultry)		Quarrying/e industries	extractive		
Institutional		Agricultural (Transport r			
Commercial/Retail		Other agricul specify below			Filling station	ng station/Fuel depot		
Mixed Use					Other			
Development								
		Water Produ						
Domestic type waste					Public Sewer			
Waste waters produced from any trade, food, preparation or business ⁴ Minor amounts produced from food preparation								
Wheel wash, vehicle			N/A					
Waste waters produce			N/A					
4		1 , 3						
Other waste waters 4			N/A					
Construction phase w								
Frequently asked que							refer	
						ss/faqforseptictanks/		
Number of fuel storag		Material Sto			None	eiopments		
Are fuel storage tanks	poseu on	N/A						
					IN/A			
•	Detail liquid / feedstuffs / organic / N/A / chemical / waste oil storage on							
outdoor sites								
Hard Surface and Open Yard Areas for Non Domestic Developments								
Footprint of proposed development including yard areas in m ²								
Is there potential for soiled yard areas from material, product waste N/A								
or manure handling, fuel dispensing, silt and soil, yard washing etc.								
If yes, are silt trap(s),	vater tank	<u> </u>						
control measures sho			pe	otextile deals with minor rol spills				
Has the use of SUDS (Sustainable Urban Drainage Systems – http://www.susdrain.org) been considered in the design of this								

Development History – All Developments Have previous pollution prevention planning conditions been complied with ⁶ ? Does existing development have an up to date (as constructed) site drainage plan? Is the existing/proposed development sewered or unsewered? If unsewered, is the existing wastewater treatment system fit for purpose ⁷ ?			
Does existing development have an up to date (as constructed) site drainage plan? Is the existing/proposed development sewered or unsewered? If unsewered, is the existing wastewater treatment system fit for purpose ⁷ ?			
plan? Is the existing/proposed development sewered or unsewered? If unsewered, is the existing wastewater treatment system fit for purpose ⁷ ?	N/A		
If unsewered, is the existing wastewater treatment system fit for purpose ⁷ ?	N/A		
	Sewere		
	N/A		
Has the storm water drainage system been examined and/or surveyed for	Prior to		
misconnections? (Information leaflet available from Environment Section)	works		
Checklist of items to be included on Site Drainage Plan			
Location of lakes, watercourse, wells used for water supply, or karst features on	N/A		
or within 25m of domestic or 100m of non domestic development site			
Location of all drainage outfall points	Yes		
Foul water drainage system (in Red)	Yes		
	(Brown)		
Storm water drainage system (in Blue)	Yes		
Soiled yard area, soiled water drainage and management system, including silt			
traps, oil interceptor(s) and any SUDS facilities			
Location of waste water treatment facilities	N/A		
Location of fuel storage tank(s)	N/A		
Stream/Lake/Wetland/Riparian Corridors			
Footnotes and Useful Information			
¹ Projection in the Irish Grid			
² WMU, RWB, WB status. Objectives available on water maps at <a href="https://www.wfdireland.id/www.</td><td></td></tr><tr><td><sup>3</sup> Available in the public mapping section at www.gsi.ie	<u>~</u>		
⁴ For information leaflets on Business Premises and Proper Use of Drains and info	rmation or		
discharge licensing see:	illiation of		
http://www.monaghan.ie/contentv3/services/environment/formsguidesdocumentsdownland	de/		
http://www.monaghan.ie/contentv3/services/environment/vater/waterawareness/leaftletsar			
⁵ Guidance: www.envirocentre.ie Best practice for Oil Storage (BPGCS05)			
⁶ Has certification of installation for previously granted wastewater treatment syste	m boon		
required and if so has it been submitted.	III Deeli		
⁷ Refer to EPA Guidance at:			
http://www.monaghan.ie/contentv3/services/environment/water/waterawareness/faqforsep	tiotonks/ an		
http://www.monaghan.ie/contentv3/setvices/environment/water/waterawareness/raqforsep	ar		
Pollution&DrainageSystems.pdf	<u>C1</u>		
⁸ Refer to Water Body, Sensitive Waters and Sensitive Land Maps in Chapter 4 of	tho		
Monaghan County Development Plan 2013-2019	ше		
Abbreviations			
WMU Water Management Unit			
WB Status Water Body Status			
IPPC Integrated Pollution Prevention Control Licence			
EIA Environmental Impact Assessment			
	Т		
For Office Use: Sensitivity of Location			
For Office Use: Sensitivity of Location Is the development located upstream of a high river quality site 8?	Is the development located within a good status waterbody 8?		
For Office Use: Sensitivity of Location Is the development located upstream of a high river quality site 8? Is the development located within a good status waterbody 8?			
For Office Use: Sensitivity of Location Is the development located upstream of a high river quality site 8? Is the development located within a good status waterbody 8? Is the development located in the catchment of a water supply source 8?			
For Office Use: Sensitivity of Location Is the development located upstream of a high river quality site 8? Is the development located within a good status waterbody 8? Is the development located in the catchment of a water supply source 8? Is the development located within the Source Protection Zone (SPZ) of a groundw	ater		
For Office Use: Sensitivity of Location Is the development located upstream of a high river quality site ⁸ ? Is the development located within a good status waterbody ⁸ ? Is the development located in the catchment of a water supply source ⁸ ? Is the development located within the Source Protection Zone (SPZ) of a groundw supply source ⁸ ?			
For Office Use: Sensitivity of Location Is the development located upstream of a high river quality site 8? Is the development located within a good status waterbody 8? Is the development located in the catchment of a water supply source 8? Is the development located within the Source Protection Zone (SPZ) of a groundw			
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For Office Use: Sensitivity of Location Is the development located upstream of a high river quality site ⁸ ? Is the development located within a good status waterbody ⁸ ? Is the development located in the catchment of a water supply source ⁸ ? Is the development located within the Source Protection Zone (SPZ) of a groundw supply source ⁸ ? Does the development require a discharge licence to surface or ground waters un	der the		



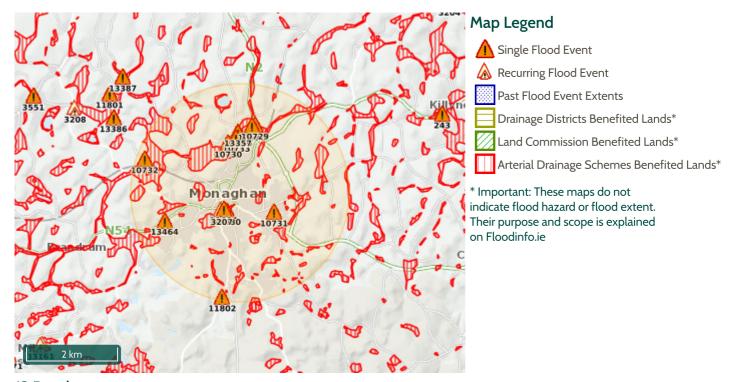
Past Flood Event Local Area Summary Report



Report Produced: 1/9/2023 9:33

This Past Flood Event Summary Report summarises all past flood events within 2.5 kilometres of the map centre.

This report has been downloaded from www.floodinfo.ie (the "Website"). The users should take account of the restrictions and limitations relating to the content and use of the Website that are explained in the Terms and Conditions. It is a condition of use of the Website that you agree to be bound by the disclaimer and other terms and conditions set out on the Website and to the privacy policy on the Website.



13 Results

Name (Flood_ID)	Start Date	Event Location
1. 🚹 Monaghan Town Cootehill Rd Monaghan 24th Oct 2011 (ID-11802)	23/10/2011	Approximate Point
Additional Information: Reports (1) Press Archive (0)		
2. 🛦 Monaghan Recurring (ID-3207)	n/a	Approximate Point
Additional Information: Reports (1) Press Archive (2)		
3. 🚹 Flooding at Ballyalbany on O5/12/2O15 (ID-13357)	05/12/2015	Approximate Point
Additional Information: Reports (O) Press Archive (O)		
4. 1 Flooding at Coolshannagh on 05/12/2015 (ID-13369)	05/12/2015	Approximate Point
Additional Information: Reports (O) Press Archive (O)		
5. 🚹 Flooding at Monaghan on 05/12/2015 (ID-13380)	05/12/2015	Approximate Point
Additional Information: Reports (O) Press Archive (O)		
6. 🚹 Flooding at Monaghan on 28/12/2015 (ID-13464)	28/12/2015	Approximate Point
Additional Information: Reports (0) Press Archive (0)		

Name (Flood_ID)	Start Date	Event Location
7. Monaghan C115 Ballyalbony 20th Nov 2009 (ID-10730)	19/11/2009	Approximate Point
Additional Information: <u>Reports (1) Press Archive (0)</u>		
8. A Shambles Monaghan Town 20th Nov 2009 (ID-10731)	19/11/2009	Approximate Point
Additional Information: Reports (1) Press Archive (0)		
9. Monaghan Crover 20th Nov 2009 (ID-10732)	19/11/2009	Approximate Point
Additional Information: Reports (1) Press Archive (0)		
10. 🚹 Monaghan C115 Coolshannagh 20th Nov 2009 (ID-10733)	19/11/2009	Approximate Point
Additional Information: Reports (1) Press Archive (0)		
11. Monaghan C115 Creamery 20th Nov 2009 (ID-10729)	19/11/2009	Approximate Point
Additional Information: <u>Reports (1) Press Archive (0)</u>		
12. Monaghan Blackwater Monaghan Town Creamery 24th October 2011 (ID-11691)	23/10/2011	Approximate Point
Additional Information: <u>Reports (1) Press Archive (0)</u>		
13. A Shambles River Monaghan Town 24th October 2011 (ID-11694)	23/10/2011	Approximate Point
Additional Information: Reports (1) Press Archive (0)		